## MEMORANDUM

TO:	Mr. Josh Sullivan Acquisitions & Due Diligence True Storage 670 North Commercial Street Manchester, NH 03101	FROM:	Mr. Jeffrey S. Dirk, P.E.,* PTOE, FITE Managing Partner and Mr. Thomas Hannon Transportation Engineer Vanasse & Associates, Inc. 35 New England Business Center Drive Suite 140 Andover, MA 01810-1066 (978) 269-6830 jdirk@rdva.com *Professional Engineer in CT, MA, ME, NH, RI and VA
DATE:	October 20, 2022	RE:	9481
SUBJECT:	Transportation Impact Assessment Proposed Self-Storage Facility – 24 Wareham, Massachusetts	00 and 2402	2 Cranberry Highway (Route 28)

Vanasse & Associates, Inc. (VAI) has conducted a Transportation Impact Assessment (TIA) in order to determine the potential impacts on the transportation infrastructure associated with the proposed construction of a self-storage facility to be located at 2400 and 2402 Cranberry Highway (Route 28) in Wareham, Massachusetts (hereafter referred to as the "Project"). This assessment evaluates the following specific areas as they relate to the Project: i) access requirements; ii) potential off-site improvements; and iii) safety considerations; and identifies and analyzes existing traffic conditions and future traffic conditions, both with and without the Project along Route 28.

Based on this assessment, we have concluded the following with respect to the Project:

- 1. Using trip-generation statistics published by the Institute of Transportation Engineers (ITE),<sup>1</sup> the Project is expected to generate approximately 88 vehicle trips on an average weekday and 106 vehicle trips on a Saturday (both two-way volumes over the operational day of the Project), with 5 vehicle trips expected during the weekday morning peak hour, 9 vehicle trips expected during the weekday evening peak hour and 10 vehicle trips expected during the Saturday midday peak hour;
- 2. No apparent safety deficiencies were noted with respect to the motor vehicle crash history along Route 28 in the vicinity of the Project site;
- 3. All movements exiting the Project site driveway to Route 28 are predicted to operate at a level-ofservice (LOS) D or better with negligible vehicle queuing, where an LOS of "D" or better is generally defined as "acceptable" operating conditions; and



<sup>&</sup>lt;sup>1</sup>*Trip Generation*, 11<sup>th</sup> Edition; Institute of Transportation Engineers; Washington, DC; 2021.

4. Lines of sight at the Route 28/Project site driveway intersection were found to exceed the recommended minimum distances for the intersections to operate in a safe and efficient manner based on the appropriate approach speed.

In consideration of the above, we have concluded that the Project can be accommodated within the confines of the existing transportation infrastructure in a safe and efficient manner with the implementation of the recommendations defined herein.

The following details our assessment of the Project.

#### **PROJECT DESCRIPTION**

As proposed, the Project will entail the construction of a two-story,  $60,000\pm$  square foot (sf) self-storage building to be located at 2400 and 2402 Cranberry Highway (Route 28) in Wareham, Massachusetts. The Project site encompasses  $3.76\pm$  acres of land that is bounded by Great Hill Drive to the north; a commercial property to the south; residential properties (Great Hill Estates mobile home park) to the east; and Route 28 to the west. The Project site is currently improved with a vacant commercial building and associated appurtenances that will be removed to accommodate the Project.



Imagery © 2022 Google

Access to the Project will be provided by a full access driveway that will intersect the east side of Route 28 at the location of the existing driveway in the southern portion of the Project site that is situated approximately 400 feet south of Great Hill Drive. The existing driveway in the northern portion of the Project site will be closed in conjunction with the Project, thereby reducing the number of conflict points for vehicles, pedestrians and bicyclists. The Project will require the issuance of a State Highway Access Permit from the Massachusetts Department of Transportation (MassDOT) for access to Route 28 as the section of roadway along which the Project site is located is a State highway under the jurisdiction of MassDOT.

Off-street parking will be provided for nine (9) vehicles, which is within the range of rates observed by ITE at similar land uses.<sup>2</sup>



<sup>&</sup>lt;sup>2</sup>*Parking Generation*, 5<sup>th</sup> Edition; Institute of Transportation Engineers; Washington, DC; 2019.

#### **STUDY METHODOLOGY**

This study, prepared in consultation with MassDOT and the Town of Wareham; was performed in accordance with MassDOT's *Transportation Impact Assessment (TIA) Guidelines* and the standards of the Traffic Engineering and Transportation Planning professions for the preparation of such reports; and was conducted in three distinct stages.

The first stage involved an assessment of existing conditions in the study area and included an inventory of roadway geometrics; pedestrian and bicycle facilities; on-street parking; public transportation services; observations of traffic flow; and collection of pedestrian, bicycle, and vehicle counts.

In the second stage of the study, future traffic conditions were projected and analyzed. Specific travel demand forecasts for the Project were assessed along with future traffic demands due to expected traffic growth independent of the Project. A seven-year time horizon was selected for analyses consistent with MassDOT guidelines. The analysis conducted in stage two identifies existing or projected future capacity, safety, and access issues, as these areas relate to the transportation infrastructure.

The third stage of the study presents and evaluates measures to address deficiencies in the transportation infrastructure, if any, identified in stage two of the study.

#### **EXISTING CONDITIONS**

A comprehensive field inventory of existing conditions within the study area was conducted in September 2022. The field investigation consisted of an inventory of existing roadway geometrics; pedestrian and bicycle facilities; public transportation services; traffic volumes; and operating characteristics; as well as posted speed limits and land use information within the study area. The study area that was assessed for the Project consisted of Route 28 in the vicinity of the Project site and the Project site driveway intersection with Route 28.

The following describes the study area roadway and intersection.

#### <u>Roadway</u>

#### Route 28

- > Two-lane urban minor arterial roadway under MassDOT jurisdiction;
- > Traverses the study area in a general northwest-southeast direction;
- Provides two 11- to 12-foot-wide travel lanes in the vicinity of the Project site that are separated by a painted median or double-yellow centerline with 4-foot-wide marked shoulders provided;
- > A sidewalk is provided along the east (Project) side of the roadway within the study area;
- > Illumination is not provided in the vicinity of the Project site;
- > The posted speed limit is 40 miles per hour (mph);
- > Land use within the study area consists of the Project site and commercial properties.



#### **Intersection**

Table 1 summarizes existing lane use, traffic control, and pedestrian and bicycle accommodations at the study area intersections as observed in September 2022.

Intersection	Traffic Control Type <sup>a</sup>	No. of Travel Lanes Provided	Shoulder Provided? (Yes/No/Width)	Pedestrian Accommodations? (Yes/No/Description)	Bicycle Accommodations? (Yes/No/Description)
Route 28/ Site Driveway	S	l general-purpose travel lane on all approaches.	Yes, 4-feet on Route 28	Yes, sidewalk along the east side of Route 28	Yes; shared traveled-way on Route 28 <sup>b</sup>

# Table 1STUDY AREA INTERSECTION DESCRIPTION

 $^{a}S = STOP$ -sign control.

<sup>b</sup>Combined shoulder and travel lane width equal to or exceeding 14 feet.

#### **Existing Traffic Volumes**

In order to determine existing traffic-volume demands and flow patterns within the study area, automatic traffic recorder (ATR) counts were completed in September 2022. The ATR counts were conducted on September 22<sup>nd</sup> through September 24<sup>th</sup>, 2022 (Thursday through Saturday, inclusive) on Route 28 in the vicinity of the Project site in order to record weekday and Saturday traffic conditions over an extended period.

In order to evaluate the potential for seasonal fluctuation of traffic volumes within the study area, traffic-volume data from MassDOT Continuous Count Station No. 7183 located on I-195 in Wareham was reviewed.<sup>3</sup> Based on a review of this data, it was determined that traffic volumes for the month of September are approximately 7.0 percent *above* average-month conditions. As such, no adjustment was made to the September traffic volumes as they are representative of above average-month conditions.

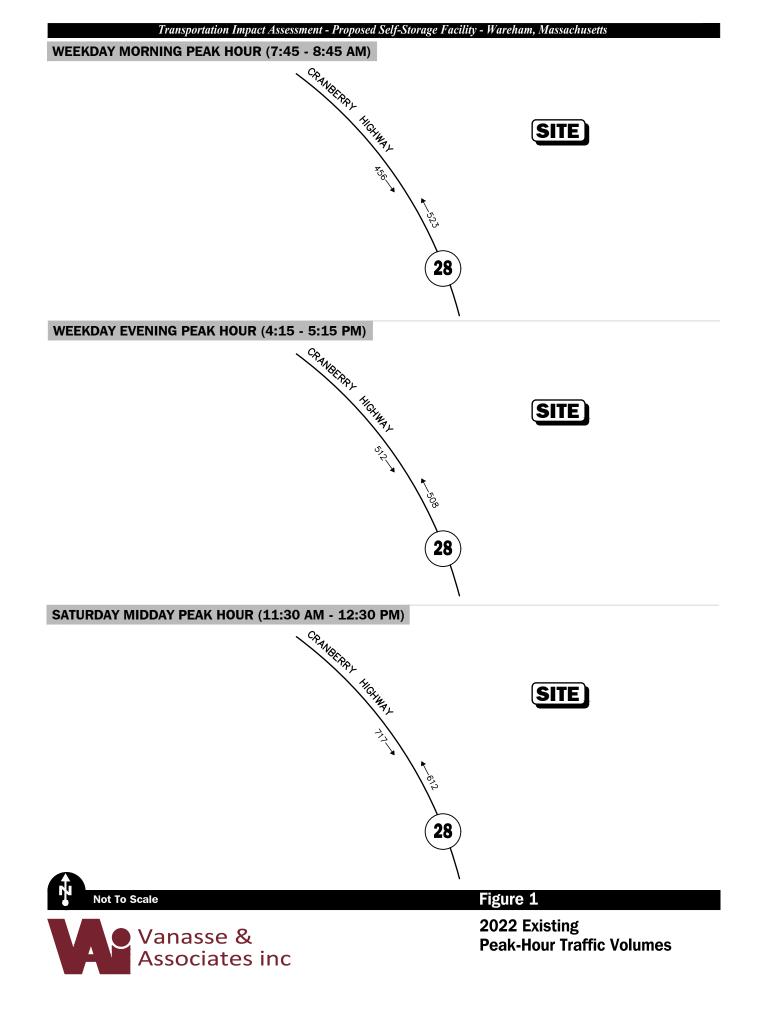
In order to account for the impact on traffic volumes and trip patterns resulting from the COVID-19 pandemic, traffic-volume data collected at MassDOT Continuous Count Station No. 7183 in September 2018 was adjusted to the year 2019 by applying the traffic growth procedure detailed in the April 2020 "Guidance on Traffic Counting Data" published by MassDOT in order to allow for a comparison between the projected September 2019 data<sup>4</sup> to the September 2022 traffic volumes that were collected at the same location. Based on this pre- and post-COVID-19 traffic-volume comparison, the traffic volume that existed prior to the COVID-19 pandemic. As such, no adjustments were made to the raw traffic-volume data.

The 2022 Existing traffic volumes are summarized in Table 2, with the weekday morning, weekday evening, and Saturday midday peak-hour traffic volumes graphically depicted on Figure 1.

<sup>&</sup>lt;sup>4</sup>MassDOT considers 2019 traffic-volume data to be representative of "existing" conditions had the COVID-19 pandemic not occurred.



<sup>&</sup>lt;sup>3</sup>MassDOT Traffic Volumes for the Commonwealth of Massachusetts; 2022.



# Table 22022 EXISTING TRAFFIC VOLUMES

Location/Peak Hour	AWT <sup>a</sup>	Saturday <sup>b</sup>	VPH <sup>c</sup>	K Factor <sup>d</sup>	Directional Distribution <sup>e</sup>
Route 28, east of Great Hill Drive:	13,220	13,710			
Weekday Morning (7:45 – 8:45 AM)			979	7.4	53.4% NWB
Weekday Evening (4:15 – 5:15 PM)			1,020	7.7	50.2% SEB
Saturday Midday (11:30 AM – 12:30 PM)			1,329	9.7	54.0% SEB

<sup>a</sup>Average weekday traffic in vehicles per day. <sup>b</sup>Average Saturday traffic in vehicles per day. <sup>e</sup>Vehicles per hour.

<sup>d</sup>Percent of daily traffic occurring during the peak hour.

<sup>e</sup>Percent traveling in peak direction. NWB = northwestbound; SEB = southeastbound.

As can be seen in Table 2, Route 28 in the vicinity of the Project site was found to accommodate approximately 13,220 vehicles on an average weekday and 13,710 vehicles on a Saturday (both two-way volumes over the operational day of the Project), with approximately 979 vehicles per hour (vph) during the weekday morning peak-hour, 1,020 vph during the weekday evening peak-hour and 1,329 vph during the Saturday midday peak-hour.

#### Pedestrian and Bicycle Facilities

A comprehensive field inventory of pedestrian and bicycle facilities within the study area was undertaken in September 2022. The field inventory consisted of a review of the location of sidewalks and pedestrian crossing locations along the study roadways and at the study intersections, as well as the location of existing and planned future bicycle facilities. A sidewalk is provided along the east (Project) side of Route 28.

Formal bicycle facilities are not provided within the study area; however, Route 28 generally provides sufficient width (combined travel lane and shoulder) to accommodate bicycle travel in a shared traveled-way condition (i.e., motor vehicles and bicyclists sharing the roadway).<sup>5</sup>

#### **Public Transportation Services**

Public transportation services are provided within the study area by the Greater Attleboro Taunton Regional Transit Authority (GATRA). GATRA provides fixed-route bus service along Route 28 within the study area by way of Link 4, *Cranberry Plaza To West Wareham*. The Link 4 bus provides service between Lakeville Station on the Massachusetts Bay Transit Authority (MBTA) Commuter Rail system and Wagner Way in Buzzards Bay, with the closest regular stop located at the Walmart Supercenter approximately 0.3 miles (a 7-minute walk) to the south of the Project site; however, GATRA buses also operate in a "flag stop" mode, where a rider can request a stop (pick-up or drop-off) anywhere along the service route where it is safe for a bus to stop by signaling to the driver. In addition to fixed-route bus services, GATRA provides Dial-a-Ride paratransit services to eligible people who cannot use fixed-route transit all or some of the time due to a physical, cognitive, or mental disability in compliance with the Americans with Disabilities Act (ADA). The public transportation schedules and fare information are provided in the Appendix.

<sup>&</sup>lt;sup>5</sup>A minimum combined travel lane and paved shoulder width of 14-feet is required to support bicycle travel in a shared traveledway condition.



#### Spot Speed Measurements

Vehicle travel speed measurements were performed on Route 28 in the vicinity of the Project site in conjunction with the ATR counts. Table 3 summarizes the vehicle travel speed measurements.

	Rou	ite 28
	Southeastbound	Northwestbound
Mean Travel Speed (mph)	33	34
85th Percentile Speed (mph)	38	40
Posted Speed Limit (mph)	40	40

#### Table 3 VEHICLE TRAVEL SPEED MEASUREMENTS

mph = miles per hour.

As can be seen in Table 3, the mean vehicle travel speed along Route 28 in the vicinity of the Project site was found to be 33 mph in the southeastbound direction and 34 mph northwestbound. The measured 85<sup>th</sup> percentile vehicle travel speed, or the speed at which 85 percent of the observed vehicles traveled at or below, was found to be 38 mph in the southeastbound direction and 40 mph northwestbound, which is consistent with the posted speed limit in the vicinity of the Project site (40 mph). The 85<sup>th</sup> percentile speed is used as the basis of engineering design and in the evaluation of sight distances and is often used in establishing posted speed limits.

#### Motor Vehicle Crash Data

Motor vehicle crash information for the study area intersection was provided by the MassDOT Highway Division Safety Management/Traffic Operations Unit for the most recent five-year period available (2015 through 2019, inclusive) to examine motor vehicle crash trends occurring within the study area. The data is summarized by intersection, type, severity, roadway and weather conditions, and day of occurrence, and is presented in Table 4.

Based on a review of this data, Route 28 at and in the vicinity of the Project site driveway experienced a total of eight (8) crashes over the five-year review period, or an average of 1.6 crashes per year. The majority of the reported crashes occurred on a weekday, during daylight, under clear weather conditions, and involved rear-end type collisions that resulted in property damage only. The intersection was found to have a motor vehicle crash rate that is *below* both the MassDOT statewide and District average crash rates for an unsignalized intersection for the MassDOT Highway Division District in which the intersection is located in (District 5).

A review of the MassDOT statewide high crash location database (Highway Safety Improvement Program (HSIP) listing) indicates that there are no high crash locations in the vicinity of the Project site. In addition, no fatal motor vehicle crashes were reported to have occurred at the study area intersection over the five-year review period.

The detailed MassDOT Crash Rate Worksheet and High Crash Location mapping are provided in the Appendix.



	Route 28/
	Project Site Driveway
Traffic Control Type <sup>b</sup>	U
Year:	
2015	0
2016	3
2017	1
2018	3
<u>2019</u>	<u>1</u>
Total	8
Average	1.60
Crash Rate <sup>c</sup>	0.30
MassDOT Crash Rate: <sup>d</sup>	0.57/0.57
Significant? <sup>e</sup>	No
Type:	
Angle	2
Head-On	$\overline{0}$
Rear-End	4
Rear-to-Rear	0
Sideswipe	0
Fixed Object	1
Pedestrian/Bicycle	0
Unknown/Other	<u>1</u>
Total	8
Conditions:	
Clear	8
Cloudy	0
Rain	0
Snow/Ice	0
Not Reported/Other	<u>0</u>
Total	8
Lighting:	
Daylight	5
Dawn/Dusk	0
Dark (Road Lit)	2
Dark (Road Unlit)	$\frac{1}{8}$
Total	8
Day of Week:	
Monday-Friday	7
Saturday	0
Sunday	$\frac{1}{8}$
Total	8
Severity:	
Property Damage Only	5
Non-fatal Injury	5 3 <u>0</u>
Not Reported	<u>0</u>
Total	8

#### Table 4 MOTOR VEHICLE CRASH DATA SUMMARY<sup>a</sup>

<sup>a</sup>Source: MassDOT Safety Management/Traffic Operations Unit records, 2015 through 2019. <sup>b</sup>Traffic Control Type: U = unsignalized. <sup>c</sup>Crash rate per million vehicles entering the intersection.

<sup>d</sup>Statewide/District crash rate.

eThe intersection crash rate is significant if it is found to exceed the MassDOT crash rate for the MassDOT Highway Division District in which the Project is located (District 5).



#### **FUTURE CONDITIONS**

Traffic volumes in the study area were projected to the year 2029, which reflects a seven-year planning horizon from the date of publication of this assessment, consistent with MassDOT guidelines. Independent of the Project, traffic volumes on the roadway network in the year 2029 under No-Build conditions include all existing traffic and new traffic resulting from background traffic growth. Anticipated Project-generated traffic volumes superimposed upon the 2029 No-Build traffic volumes reflect 2029 Build traffic-volume conditions with the Project.

#### **Future Traffic Growth**

Future traffic growth is a function of the expected land development in the immediate area and the surrounding region. Several methods can be used to estimate this growth. A procedure frequently employed estimates an annual percentage increase in traffic growth and applies that percentage to all traffic volumes under study. The drawback to such a procedure is that some turning volumes may actually grow at either a higher or a lower rate at particular intersections.

An alternative procedure identifies the location and type of planned development, estimates the traffic to be generated, and assigns it to the area roadway network. This procedure produces a more realistic estimate of growth for local traffic; however, potential population growth and development external to the study area would not be accounted for in the resulting traffic projections.

To provide a conservative analysis framework, both procedures were used, the salient components of which are described below.

#### **Specific Development by Others**

The Town of Wareham Planning Department was contacted in order to determine if there were any projects planned within the study area that would have an impact on future traffic volumes at the study intersections. Based on this consultation, no developments were identified at this time that are expected to result in an increase in traffic within the study area beyond the general background traffic growth (discussion follows).

#### **General Background Traffic Growth**

Traffic-volume data compiled by MassDOT from permanent count stations located in Wareham and Rochester were reviewed in order to determine general traffic growth trends in the area. This data indicates that traffic volumes have fluctuated over the 10-year period between 2009 and 2019, with an average traffic growth rate of 1.42 percent per year. In order to provide a prudent planning condition for the Project, a slightly higher 1.5 percent per year compounded annual background traffic growth rate was used in order to account for future traffic growth and presently unforeseen development within the study area.

#### **Roadway Improvement Projects**

The Town of Wareham and MassDOT were contacted in order to determine if there were any planned future roadway improvement projects expected to be completed by 2029 within the study area. Based on these discussions, no roadway improvement projects aside from routine maintenance activities were identified to be planned within the study area at this time.



#### **No-Build Traffic Volumes**

The 2029 No-Build condition peak-hour traffic volumes were developed by applying the 1.5 percent per year compounded annual background traffic growth rate to the 2022 Existing peak-hour traffic volumes. The resulting 2029 No-Build weekday morning, weekday evening, and Saturday midday peak-hour traffic volumes are shown on Figure 2.

#### **Project-Generated Traffic**

Design year (2029 Build) traffic volumes for the study area roadways were determined by estimating Project-generated traffic volumes and assigning those volumes on the study roadways. The following sections describe the methodology used to develop the anticipated traffic characteristics of the Project

As proposed, the Project will entail the construction of a  $60,000\pm$  sf self-storage facility. In order to develop the traffic characteristics of the Project, trip-generation statistics published by the ITE<sup>6</sup> for similar a land use as that proposed was used. ITE Land Use Code (LUC) 151, *Mini-Warehouse*, was used to develop the traffic characteristics of the Project, the results of which are summarized in Table 5.

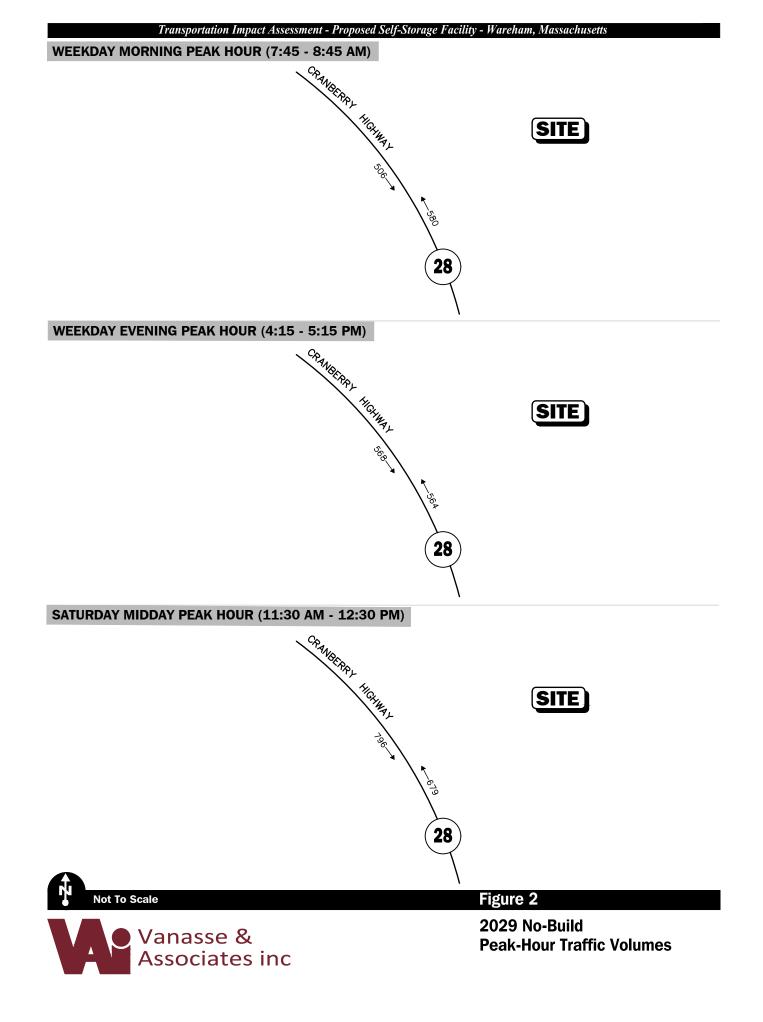
	Vehicle Trips
	Self-Storage
Time Period/Direction	(60,000 sf) <sup>a</sup>
Average Weekday Daily:	
Entering	44
Exiting	$\frac{44}{88}$
Total	88
Weekday Morning Peak Hour:	
Entering	3
Exiting	$\frac{3}{2}$
Total	5
Weekday Evening Peak Hour:	
Entering	4
Exiting	4 <u>5</u> 9
Total	9
Saturday:	
Entering	53
Exiting	53
Total	106
Saturday Midday Peak Hour:	
Entering	6
Exiting	4
Total	10

# Table 5TRIP-GENERATION SUMMARY

<sup>a</sup>Based on ITE LUC 151, *Mini-Warehouse*.



<sup>6</sup>Ibid 1.



#### **Project-Generated Traffic-Volume Summary**

As can be seen in Table 5, the Project is expected to generate approximately 88 vehicle trips on an average weekday and 106 vehicle trips on a Saturday (both two-way volumes over the operational day of the Project), with approximately 5 vehicle trips (3 vehicles entering and 2 exiting) expected during the weekday morning peak-hour, 9 vehicle trips (4 vehicles entering and 5 exiting) expected during the weekday evening peak-hour and 10 vehicle trips (6 vehicles entering and 4 exiting) expected during the Saturday midday peak-hour.

#### **Trip Distribution and Assignment**

The directional distribution of generated trips to and from the Project site was determined based on a review of existing traffic patterns within the study area. The general trip distribution for the Project is graphically depicted on Figure 3. The additional traffic expected to be generated by the Project was assigned on the study area roadway network as shown on Figure 4.

#### **Build Traffic Volumes**

The 2029 Build condition traffic volumes consist of the 2029 No-Build traffic volumes with the addition of trips associated with the Project. The 2029 Build weekday morning, weekday evening, and Saturday midday peak-hour traffic volumes are graphically depicted on Figure 5.

#### TRAFFIC OPERATIONS ANALYSIS

In order to assess the potential impact of the Project on the roadway network, a detailed traffic operations analysis (motorist delays, vehicle queuing, and level-of-service) was performed for the study intersections. Capacity analyses provide an indication of how well transportation facilities serve the traffic demands placed upon them, with vehicle queue analyses providing a secondary measure of the operational characteristics of an intersection or section of roadway under study.

In brief, six levels of service are defined for each type of facility. They are given letter designations ranging from A to F, with LOS "A" representing the best-operating conditions and LOS "F" representing congested or constrained operations. An LOS of "E" is representative of a transportation facility that is operating at its design capacity with an LOS of "D" generally defined as the limit of "acceptable" traffic operations. Since the level-of-service of a traffic facility is a function of the flows placed upon it, such a facility may operate at a wide range of levels of service depending on the time of day, day of week, or period of the year. The Synchro® intersection capacity analysis software, which is based on the analysis methodologies and procedures presented in the 2010 *Highway Capacity Manual* (HCM)<sup>7</sup> for unsignalized intersections, was used to complete the level-of-service and vehicle queue analyses.

#### Analysis Results

Level-of-service and vehicle queue analysis were conducted for 2022 Existing, 2019 No-Build, and 2029 Build conditions for the intersection within the study area. The results of the intersection capacity and vehicle queue analyses are summarized in Table 6, with the detailed analysis results presented in the appendix. For context, we note that an LOS of "D" or better is generally defined as "acceptable" operating conditions.

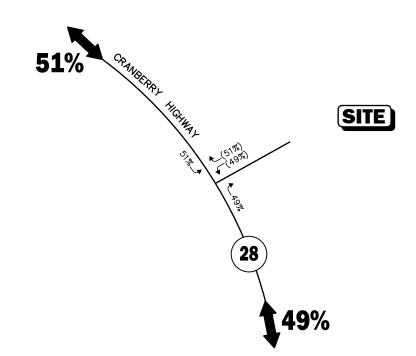


<sup>&</sup>lt;sup>7</sup>*Highway Capacity Manual*, Transportation Research Board; Washington, DC; 2010.

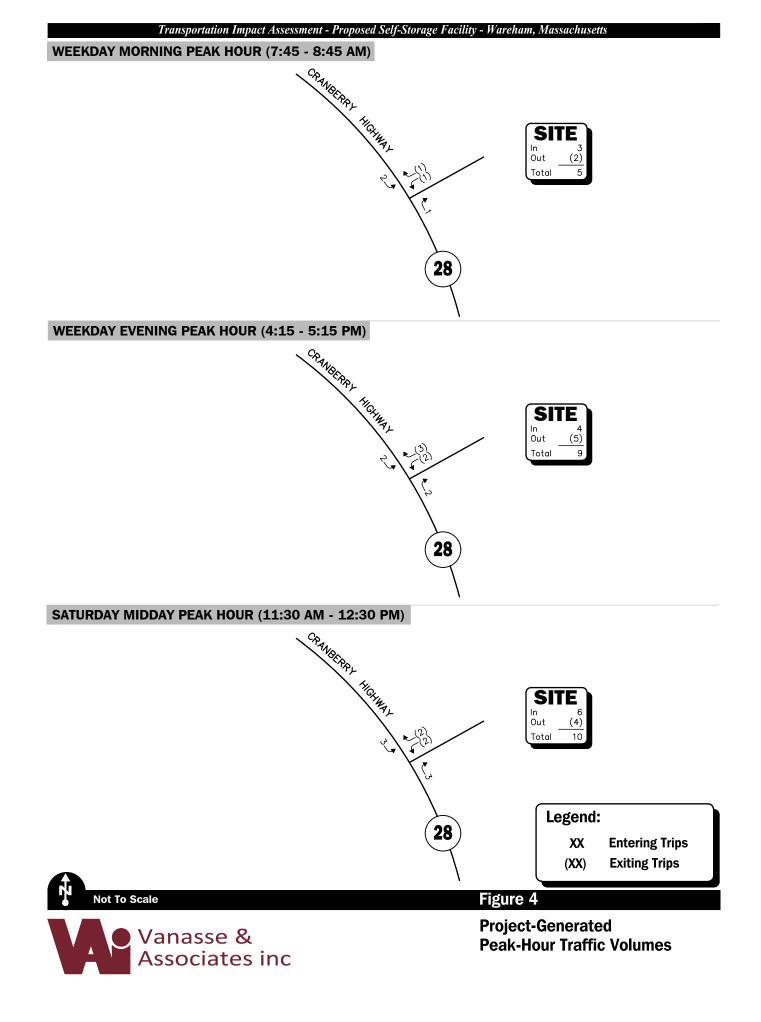
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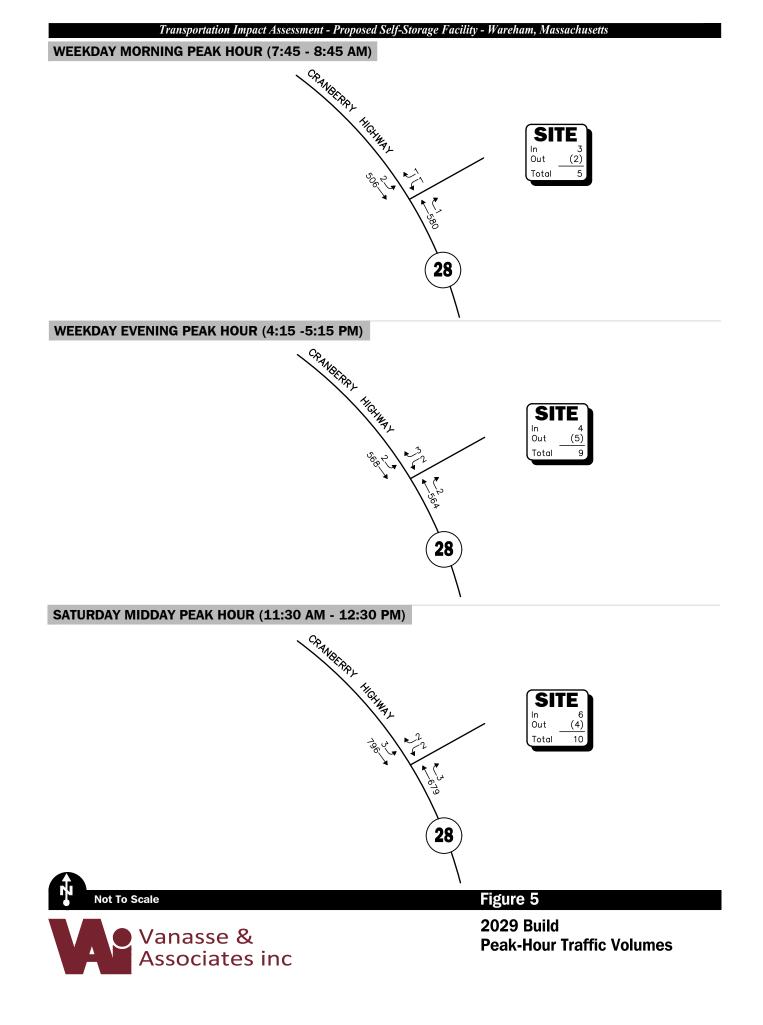
Legend:

XX Entering Trips (XX) Exiting Trips









As can be seen in Table 6, all movements exiting the Project site driveway to Route 28 were shown to operate at LOS C during the weekday morning and evening peak hours and at LOS D during the Saturday midday peak-hour, with no material vehicle queuing predicted. All movements along Route 28 approaching the Project site driveway are expected to operate at LOS A during all peak hours also with no material vehicle queuing predicted.



#### Table 6 UNSIGNALIZED INTERSECTION LEVEL-OF-SERVICE AND VEHICLE QUEUE SUMMARY

		2022 Ex	isting			2029 No	-Build		2029 Build					
Unsignalized Intersection/Peak Hour/Movement	Demand	Delay	LOS	Queue 95 <sup>th</sup>	Demand	Delay	LOS	Queue 95 <sup>th</sup>	Demand	Delay	LOS	Queue 95 <sup>th</sup>		
Route 28 at Project Site Driveway														
Weekday Morning:														
Project Site Driveway LT/RT									2	18.2	С	0		
Route 28 NWB LT/TH/RT	523	0.0	А	0	580	0.0	А	0	582	0.0	А	0		
Route 28 SEB LT/TH/RT	456	0.0	А	0	506	0.0	А	0	507	0.0	А	0		
Weekday Evening:														
Project Site Driveway LT/RT									5	17.3	С	0		
Route 28 NWB LT/TH/RT	508	0.0	А	0	564	0.0	А	0	566	0.0	А	0		
Route 28 SEB LT/TH/RT	512	0.0	А	0	568	0.0	А	0	570	0.0	А	0		
Saturday Midday:														
Project Site Driveway LT/RT									4	25.0	D	0		
Route 28 NWB LT/TH/RT	612	0.0	А	0	679	0.0	А	0	682	0.0	А	0		
Route 28 SEB LT/TH/RT	717	0.0	А	0	796	0.0	А	0	799	0.0	А	0		

<sup>a</sup>Demand in vehicles per hour. <sup>b</sup>Average control delay per vehicle (in seconds).

<sup>c</sup>Level of service.

<sup>d</sup>Queue length in vehicles.

SEB= southeastbound; NWB = northwestbound; LT = left-turning movements; TH = through movements; RT = right-turning movements.



#### SIGHT DISTANCE ASSESSMENT

Sight distance measurements were performed at the Project site driveway intersection with Route 28 in accordance with MassDOT and American Association of State Highway and Transportation Officials (AASHTO)<sup>8</sup> requirements. Both stopping sight distance (SSD) and intersection sight distance (ISD) measurements were performed. In brief, SSD is the distance required by a vehicle traveling at the design speed of a roadway, on wet pavement, to stop prior to striking an object in its travel path. ISD or corner sight distance (CSD) is the sight distance required by a driver entering or crossing an intersecting roadway to perceive an oncoming vehicle and safely complete a turning or crossing maneuver with oncoming traffic. In accordance with AASHTO standards, if the measured ISD is at least equal to the required SSD value for the appropriate design speed, the intersection can operate in a safe manner. Table 7 presents the measured SSD and ISD at the subject intersection.

#### Table 7 SIGHT DISTANCE MEASUREMENTS<sup>a</sup>

		Feet	
Intersection/Sight Distance Measurement	Required Minimum (SSD)	Desirable (ISD) <sup>b</sup>	Measured
Route 28 at Project Site Driveway			
Stopping Sight Distance:			
Site Driveway approaching from the south	305		650+
Site Driveway approaching from the north	305		650 +
Intersection Sight Distance:			
Looking to the south from the Site Driveway	305	385	650+
Looking to the north from the Site Driveway	305	445	650+

<sup>a</sup>Recommended minimum values obtained from *A Policy on Geometric Design of Highways and Streets*, 7<sup>th</sup> Edition; American Association of State Highway and Transportation Officials (AASHTO); 2018; and based on a 40 mph approach speed on Route 28.

<sup>b</sup>Values shown are the intersection sight distance for a vehicle turning right or left exiting a roadway under STOP control such that motorists approaching the intersection on the major street should not need to adjust their travel speed to less than 70 percent of their initial approach speed

As can be seen in Table 7, the available lines of sight at the Project site driveway intersection were found to exceed the recommended minimum sight distance to function in a safe (SSD) and efficient (ISD) manner based on a 40 mph approach speed, which is consistent with both the measured 85<sup>th</sup> percentile vehicle travel speed along Route 28 (38/40 mph) and the posted speed limit in the vicinity of the Project site (40 mph).

<sup>&</sup>lt;sup>8</sup>A Policy on Geometric Design of Highway and Streets, 7<sup>th</sup> Edition; American Association of State Highway and Transportation Officials (AASHTO); Washington D.C.; 2018.



#### **SUMMARY**

VAI has completed a detailed assessment of the potential impacts on the transportation infrastructure associated with the proposed construction of a self-storage facility to be located at 2400 and 2402 Cranberry Highway (Route 28) in Wareham, Massachusetts. The following specific areas have been evaluated as they relate to the Project: i) access requirements; ii) potential off-site improvements; and iii) safety considerations; under existing and future conditions, both with and without the Project. Based on this assessment, we have concluded the following with respect to the Project:

- 1. Using trip-generation statistics published by the ITE,<sup>9</sup> the Project is expected to generate approximately 88 vehicle trips on an average weekday and 106 vehicle trips on a Saturday (both two-way volumes over the operational day of the Project), with 5 vehicle trips expected during the weekday morning peak hour, 9 vehicle trips expected during the weekday evening peak hour and 10 vehicle trips expected during the Saturday midday peak hour;
- 2. No apparent safety deficiencies were noted with respect to the motor vehicle crash history along Route 28 in the vicinity of the Project site;
- 3. All movements exiting the Project site driveway to Route 28 are predicted to operate at LOS D or better with negligible vehicle queuing, where an LOS of "D" or better is generally defined as "acceptable" operating conditions; and
- 4. Lines of sight at the Route 28/Project site driveway intersection were found to exceed the recommended minimum distances for the intersections to operate in a safe and efficient manner based on the appropriate approach speed.

In consideration of the above, we have concluded that the Project can be accommodated within the confines of the existing transportation infrastructure in a safe and efficient manner with the implementation of the recommendations that follow.

#### **RECOMMENDATIONS**

A detailed transportation improvement program has been developed that is designed to provide safe and efficient access to the Project site and address any deficiencies identified as a part of this assessment. The following improvements have been recommended as a part of this evaluation and, where applicable, will be completed in conjunction with the Project subject to receipt of all necessary rights, permits, and approvals.

#### **Project Access**

Access to the Project will be provided by a full access driveway that will intersect the east side of Route 28 at the location of the existing driveway in the southern portion of the Project site that is situated approximately 400 feet south of Great Hill Drive. The existing driveway in the northern portion of the Project site will be closed in conjunction with the Project, thereby reducing the number of conflict points for vehicles, pedestrians and bicyclists. The following recommendations are offered with respect to the design and operation of the Project site access and internal circulation, many of which are reflected on the Site Plans:



<sup>&</sup>lt;sup>9</sup>Ibid 1.

- The Project site driveway should be 24 feet in width and designed to accommodate the turning and maneuvering requirements of the largest anticipated responding emergency vehicle.
- Vehicles exiting the Project site should be placed under STOP control with a marked STOP-line provided.
- ➤ Where perpendicular parking is proposed, the drive aisle behind the parking should be a minimum of 23 feet in order to facilitate parking maneuvers.
- A sidewalk should be provided along one side of the Project site driveway that extends to the existing sidewalk along Route 28, with Americans with Disabilities Act (ADA) compliant wheelchair ramps provided for crossing the Project site driveway and at pedestrian crossings located within the Project site.
- ➤ All signs and pavement markings to be installed within the Project site shall conform to the applicable standards of the Manual on Uniform Traffic Control Devices (MUTCD).<sup>10</sup>
- Signs and landscaping to be installed as a part of the Project within the intersection sight triangle areas of the Project site driveway should be designed and maintained so as not to restrict lines of sight.
- Snow accumulations (windrows) within sight triangle areas will be promptly removed where such accumulations would impede sight lines.

#### **Transportation Demand Management**

In an effort to encourage the use of alternative modes of transportation to single-occupancy vehicles (SOVs) and to promote healthy transportation options to employees of the Project, the following Transportation Demand Management (TDM) measures should be considered for implementation as a part of the Project:

- > A transportation coordinator should be assigned for the Project to coordinate the TDM program;
- Information regarding public transportation services, maps, schedules, and fare information should be posted in a central location and/or otherwise made available to employees;
- A "welcome packet" should be provided to employees detailing available commuter options and should include the contact information for the transportation coordinator;
- Specific amenities should be provided to discourage off-site trips, including one or more of the following measures: providing a breakroom equipped with a microwave and refrigerator; offering direct deposit of paychecks; coordinating with a dry-cleaning service for on-site pick-up and delivery; allowing telecommuting or flexible work schedules; and other such measures to reduce overall traffic volumes and travel during peak traffic-volume periods; and
- > Secure bicycle parking should be provided at an appropriate location within the Project site.

With implementation of the above recommendations, safe and efficient access will be provided to the Project site and the Project can be accommodated within the confines of the existing transportation infrastructure.

<sup>&</sup>lt;sup>10</sup>Manual on Uniform Traffic Control Devices (MUTCD); Federal Highway Administration; Washington, D.C.; 2009.



## APPENDIX

AUTOMATIC TRAFFIC RECORDER COUNT DATA SEASONAL ADJUSTMENT DATA COVID-19 ADJUSTMENT DATA PUBLIC TRANSPORTATION SCHEDULES VEHICLE TRAVEL SPEED DATA MASSDOT CRASH RATE WORKSHEETS AND HIGH CRASH LOCATION MAP GENERAL BACKGROUND TRAFFIC GROWTH TRIP-GENERATION CALCULATIONS CAPACITY ANALYSIS WORKSHEETS



AUTOMATIC TRAFFIC RECORDER COUNT DATA



Location : Route 28 Location : East of Great Hill Drive City/State: Wareham, MA

#### Accurate Counts 978-664-2565

9/22/2022	EE	3,	Hour T	otals	WE	3,	Hour	Totals	Combined Totals		
Time	Morning	Afternoon	Morning	Afternon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	
12:00	9	150	× ×		9	145			· · · · ·		
12:15	3	121			6	133					
12:30	11	153			6	146					
12:45	6	137			5	152					
1:00	6	134			2	139					
1:15	4	95			7	138					
1:30	8	88			10	113					
1:45	2	88			6	111					
2:00	5	98			7	127					
2:15	5	71			6	96					
2:30	6	116			7	95					
2:45	3	104			6	120					
3:00	4	104			3	103					
3:15	2	102			7	122					
3:30	7	137			12	120					
3:45	6	139			10	101					
4:00	9	133			15	130					
4:15	11	132			25	116					
4:30	17	122			29	129					
4:45	11	123			22	119					
5:00	18	135			31	144					
5:15	24	89			31	121					
5:30	27	111			56	97					
5:45	32	82			61	97					
6:00	33	91			55	86					
6:15	51	66			85	81					
6:30	74	63			94	88					
6:45	119	74			134	77					
7:00	94	68			94	64					
7:15	96	51			127	57					
7:30	86	44			122	51					
7:45	133	31			136	56					
8:00	99	36			121	53					
8:15	113	35			146	61					
8:30	111	35			120	32					
8:45	108	30			102	33					
9:00	91	37			94	45					
9:15	113	18			94	20					
9:30	101	22			109	32					
9:45	126	16			95	27					
10:00	127	12			107	21					
10:15	121	9			103	16					
10:30	129	12			117	20					
10:45	113	12			121	12					
11:00	121	10			106	6					
11:15	124	8			117	19					
11:30	111	6			117	7					
11:45	129	3			129	6					
tal	2759	3553			3024	3884			C	)	
rcent	43.7%	56.3%			43.8%	56.2%				ł	

94810001

Location : Route 28 Location : East of Great Hill Drive City/State: Wareham, MA

#### Accurate Counts 978-664-2565

9/19/2022	Monday	/	Tuesday	1	Wednesday		Thursda		Friday		Saturda		Sunday		Week Ave	
Time	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,
12:00 AM	*	*	*	*	*	*	29	26	15	21	27	32	*	*	24	26
1:00	*	*	*	*	*	*	20	25	18	25	29	28	*	*	22	26
2:00	*	*	*	*	*	*	19	26	17	27	20	17	*	*	19	23
3:00	*	*	*	*	*	*	19	32	23	40	21	24	*	*	21	32
4:00	*	*	*	*	*	*	48	91	46	89	23	42	*	*	39	74
5:00	*	*	*	*	*	*	101	179	96	180	61	77	*	*	86	145
6:00	*	*	*	*	*	*	277	368	281	378	125	126	*	*	228	291
7:00	*	*	*	*	*	*	409	479	384	473	212	228	*	*	335	393
8:00	*	*	*	*	*	*	431	489	434	483	362	343	*	*	409	438
9:00	*	*	*	*	*	*	431	392	512	404	450	439	*	*	464	412
10:00	*	*	*	*	*	*	490	448	514	480	608	572	*	*	537	500
11:00	*	*	*	*	*	*	485	469	617	605	636	582	*	*	579	552
12:00 PM	*	*	*	*	*	*	561	576	613	644	664	591	*	*	613	604
1:00	*	*	*	*	*	*	405	501	596	586	575	548	*	*	525	545
2:00	*	*	*	*	*	*	389	438	540	594	581	595	*	*	503	542
3:00	*	*	*	*	*	*	482	446	724	615	516	494	*	*	574	518
4:00	*	*	*	*	*	*	510	494	719	590	502	515	*	*	577	533
5:00	*	*	*	*	*	*	417	459	581	580	388	457	*	*	462	499
6:00	*	*	*	*	*	*	294	332	449	471	378	358	*	*	374	387
7:00	*	*	*	*	*	*	194	228	322	327	235	267	*	*	250	274
8:00	*	*	*	*	*	*	136	179	194	239	156	300	*	*	162	239
9:00	*	*	*	*	*	*	93	124	123	167	129	167	*	*	115	153
10:00	*	*	*	*	*	*	45	69	63	102	77	100	*	*	62	90
11:00	*	*	*	*	*	*	27	38	44	71	13	19	*	*	28	43
Total	0	0	0	0	0	0	6312	6908	7925	8191	6788	6921	0	0	7008	7340
Day	0		0		0		13220		16116		13709		0		14348	
AM Peak							10:00	8:00	11:00	11:00	11:00	11:00			11:00	11:00
Volume							490	489	617	605	636	582			579	552
PM Peak							12:00 PM	12:00 PM	3:00	12:00 PM	12:00 PM	2:00			12:00 PM	12:00 PN
Volume							561	576	724	644	664	595			613	604
Comb Total	0		0		0		13220		16116		13709		0		14348	3

ADT ADT: 14,373 AADT: 14,373

94810001

SEASONAL ADJUSTMENT DATA



## 2018 Average Count Data – Sta. 7183

September 2018 ADT: 37,770

2018 Average Count Data – Sta. 7183

Year 2018 ADT: 35,113

Seasonal Adjustment

35,113 / 37,770 = 0.930

## Massachusetts Highway Department 7183: Monthly Hourly Volume for September 2018

Locati Count				'183 Plymout	th								l Factor ctor Gro	•	: I	J1-Sout	theast									
Functi Locatio	onal Cla on:	SS	1 I	-	ATE 19	5							tor Gro Factor		ι	J1-Sout	theast									
	0:00	1:00	2:00	3:00	4:00	5:00	6:00	7:00	8:00	9:00	10:00	11:00	12:00	13:00	14:00	15:00	16:00	17:00	18:00	19:00	20:00	21:00	22:00	23:00	TOTAL	QC Status
1	483	247	171	174	233	438	910	1492	1898	2334	2840	2984	2751	2759	2645	2697	2543	2287	1957	1637	1374	1274	1034	615	37777	Accepted
2	337	185	140	112	148	285	594	955	1330	1937	2404	2827	2777	2550	2290	2378	2302	2298	2086	1714	1564	1192	873	546	33824	Accepted
3	283	141	114	123	198	329	688	1190	1673	2176	2568	2815	2800	2756	2620	2626	2498	2519	1981	1675	1302	882	538	317	34812	Accepted
4	156	100	112	163	425	1071	2592	3314	2918	2537	2545	2492	2513	2465	2683	3252	3487	3175	2186	1359	1120	721	545	391	42322	Accepted
5	181	138	104	154	371	1056	2512	3368	2671	2144	2134	2200	2177	2177	2491	3235	3428	2964	2089	1413	1082	838	569	399	39895	Accepted
6	229	146	126	159	369	1063	2476	3128	2682	2171	2300	2223	2285	2237	2583	3273	3563	3104	2240	1422	1081	793	646	480	40779	Accepted
8	362	258	184	187	261	466	1064	1615	1794	2414	2602	2950	3046	2777	2707	2794	2556	2503	1982	1434	1261	1058	932	601	37808	Accorted
9	288	258 196	131	124	157	274	620	1015	1794	1852	2002	2586	2829	2634	2409	2794	2350	2303	1982	1366	1201	678	512	355	31430	Accepted Accepted
10	195	138	109	165	390	1044	2463	3181	2691	2251	2188	2268	2299	2034	2588	2972	3216	2980	1954	1195	869	614	451	328	38727	Accepted
11	160	121	97	156	340	1050	2455	3351	2565	2026	2131	2072	2186	2080	2577	3140	3441	3095	2068	1372	1000	713	553	345	39094	Accepted
12	183	132	88	140	397	1025	2522	3274	2695	2053	1960	2038	2072	2162	2588	3123	3336	3091	1998	1264	1002	780	521	382	38826	Accepted
13	226	127	97	144	323	1004	2237	2671	2536	2442	2142	2075	2113	2177	2519	3267	3356	3166	2340	1513	1166	814	597	502	39554	Accepted
14	240	169	103	148	345	1030	2398	3156	2778	2284	2415	2375	2492	2579	3029	3653	3724	3408	2733	1855	1405	1179	916	638	45052	Accepted
15	389	232	153	156	233	480	969	1503	1801	2268	2649	2839	2962	2761	2739	2769	2522	2270	1982	1561	1308	1017	901	622	37086	Accepted
16	317	204	133	99	149	319	565	1018	1362	1945	2542	2776	2839	2715	2634	2865	2754	2327	1994	1547	1262	779	479	341	33965	Accepted
17	156	123	100	161	378	1084	2568	3206	2694	2108	2074	2148	2200	2143	2439	3077	3398	3101	2021	1329	969	649	495	351	38972	Accepted
18	163	112	99	131	350	967	2370	3148	2555	2006	1850	1908	1804	1940	2139	2674	3012	2689	1604	1096	815	613	462	324	34831	Accepted
19	151	105	78	110	329	1043	2502	3245	2601	2108	1931	2090	2073	2118	2417	3097	3268	3154	2048	1289	978	701	521	376	38333	Accepted
20	210	135	112	117	363	1076	2569	3199	2715	2231	2146	2179	2165	2190	2639	3322	3503	3226	2254	1446	1086	786	665	482	40816	Accepted
21	289	146	101	155	357	981	2437	3100	2712	2191	2344	2298	2469	2555	3000	3571	3608	3446	2593	1705	1262	1055	910	635	43920	Accepted
22	296	214	145	150	231	472	1022	1362	1820	2075	2467	2777	2751	2749	2691	2684	2512	2418	1963	1452	1133	1040	820	593	35837	Accepted
23 24	301	179	125	123	142	318	633	881	1229	1813	2311	2680	2863	2593	2519	2625	2523	2296	2026	1418	1027	587	439	328	31979	Accepted
24	152 159	92 119	87 98	135 122	350 344	1061 996	2479 2478	3409 3143	2617 2659	2101 2013	2104 1888	2017 1926	2132 1940	2077 1976	2470 2298	2972 2779	3181 3027	3048 2792	2013 1731	1271 1089	878 838	598 631	452 457	318 325	38014 35828	Accepted
26	165	102	98 96	122	344 328	996 965	2478	3143	2659	2013	1888	2000	2016	2063	2298	3067	3027	3049	2016	1089	838 992	699	457 536	325	35828	Accepted Accepted
27	183	102	94	119	328	1071	2420	3266	2701	2160	2047	2000	2010	2003	2552	3047	3422	3049	2309	1586	1126	783	606	437	40016	Accepted
28	241	146	100	138	332	992	2189	2953	2474	2030	2055	22147	2349	2382	2747	3123	3428	3256	2303	1528	1120	1013	835	547	40521	Accepted
29	358	180	135	154	217	485	997	1408	1761	2141	2550	2823	2853	2634	2772	2642	2529	2343	2036	1584	1207	1002	822	605	36238	Accepted
30	364	162	120	119	145	296	576	872	1235	1820	2319	2619	2723	2643	2506	2371	2492	2346	1987	1513	1023	645	481	308	31685	Accepted
																							s	ept	37769.66	·
																							Y	'ear	35,113	
																							4	diust	92 97%	

Adjust 92.97%

COVID-19 ADJUSTMENT DATA



### 2018 Average Count Data – Sta. 7183

September 2018 ADT: 37,770

Growth Rate (2018-2019): 0% [MassDOT Yearly Growth Rates (U1 - Southeast)]

37,770 x (1.00) = 37,770

## 2022 Average Count Data – Sta. 7183

September 2022 ADT: 37,977

**COVID Adjustment** 

37,770 / 37,977 = 0.995

## Massachusetts Highway Department 7183: Monthly Hourly Volume for September 2022

Location ID: County: Functional Class Location:		SS	7183 Plymouth 1 INTERSTATE 195				Seasonal Factor Group: Daily Factor Group: Axle Factor Group: Growth Factor Group:																			
	0:00	1:00	2:00	3:00	4:00	5:00	6:00	7:00	8:00	9:00	10:00	11:00	12:00	13:00	14:00	15:00	16:00	17:00	18:00	19:00	20:00	21:00	22:00	23:00	TOTAL	QC Status
1	199	116	100	124	357	1072	2630	3095	2676	2484	2303	2333	2367	2436	2816	3353	3532	3155	2215	1609	1257	910	772	649	42560	Accepted
2	371	176	114	145	333	935	2285	2685	2352	2132	2486	2722	2789	2720	2935	3190	3106	3051	2278	1713	1477	1246	1009	717	42967	Accepted
3	298	165	118	155	255	477	884	1360	1804	2472	2722	2930	2781	2624	2512	2490	2295	2183	1884	1464	1248	1010	888	507	35526	Accepted
4	236	178	104	115	179	295	540	943	1282	1883	2440	2629	2664	2677	2423	2321	2207	2151	2080	1767	1288	934	654	468	32458	Accepted
5	221	130	95	131	188	336	739	1158	1539	1920	2328	2519	2691	2563	2655	2290	2085	1665	1621	1240	844	513	394	264	30129	Accepted
6	117	79	71	120	342	968	2315	3051	2581	2149	2179	2138	2140	2199	2512	2924	3029	2745	1699	1113	788	582	425	301	36567	Accepted
7	137	96	74	138	370	1069	2650	3230	2662	2197	2168	2088	2075	2354	2617	3142	3397	3158	2027	1231	988	659	515	319	39361	Accepted
8	159	99	85	136	369	1083	2632	3180	2658	2195	2241	2193	2292	2450	2758	3399	3500	2998	2064	1432	1067	768	585	394	40737	Accepted
9	203	119	106	126	316	1022	2452	2880	2477	2186	2326	2491	2468	2569	3001	3430	3431	3134	2315	1598	1158	1064	813	525	42210	Accepted
10	308	170	136	124	240	556	1154	1511	1759	2172	2722	2911	2855	2846	2586	2557	2488	2328	1862	1465	1217	1003	737	547	36254	Accepted
11	321	189	108	107	166	312	599	1026	1448	1963	2412	2538	2877	2634	2472	2569	2516	2236	1791	1444	976	606	445	262	32017	Accepted
12 13	120	75	89	115	359	1050	2515	3043	2527	2094	2122	2064	2222	2245	2492	3078	3264	2869	1732	1159	827	557	430	314	37362	Accepted
14	158	92	83	109	365	1097	2595	3178	2690	2081	2141	2086	2130	2210	2539	3249	3530	3065	1950	1414	964	688	549	399	39362	Accepted
15	167	94	76	127	371	1132	2670	3168	2638	2286	2156	2261	2287	2420	2733	3287	3616	3138	2125	1432	1077	833	601	409	41104	Accepted
16	186	120	107	134	348	1010	2487	2907	2634	2309	2326	2435	2701	2669	2977	3549	3600	3186	2502	1661	1249	968	838	581	43484	Accepted
17	303	165	122	126	250	478	1006	1436	1824	2351	2711	3052	2889	2746	2603	2643	2433	2335	2074	1473	1197	909	801	578	36505	Accepted
18	265	140	88	101	151	304	577	982	1419	1976	2298	2662	2883	2788	2492	2548	2451	2310	2012	1500	1061	645	435	298	32386	Accepted
19	169	112	79	129	374	1111	2570	3076	2685	2140	2142	2112	2097	2149	2488	3098	3215	2817	1716	1120	768	500	426	300	37393	Accepted
20	161	100	86	113	351	1118	2617	3233	2633	2156	1997	1942	2065	2158	2478	3117	3349	2891	1873	1159	818	639	461	299	37814	Accepted
21	154	89	71	132	349	1137	2611	3242	2738	2088	2071	2182	2006	2210	2530	3154	3434	2873	1939	1406	990	637	474	335	38852	Accepted
22																										
23	189	99	101	120	335	1063	2484	2884	2551	2303	2407	2504	2545	2650	3048	3503	3586	3255	2335	1713	1203	900	822	524	43124	Accepted
24	306	155	102	105	231	469	962	1314	1670	2127	2592	2873	2956	2902	2888	2661	2677	2355	1974	1492	1275	916	769	532	36303	Accepted
25	264	153	114	121	142	291	549	881	1238	1876	2289	2650	2721	2511	2446	2314	2266	2007	1701	1309	906	574	395	292	30010	Accepted
26	122	105	60	123	353	1092	2506	3118	2624	2244	2240	2248	2186	2201	2586	3193	3303	2859	1825	1260	786	553	458	306	38351	Accepted
27	133	87	77	148	371	1073	2677	3202	2692	2229	2135	2100	2189	2220	2595	3076	3403	2992	1850	1274	895	604	463	294	38779	Accepted
28	138	80	86	121	352	1121	2592	3197	2607	2067	2067	1912	2034	2100	2619	3140	3522	3106	1968	1291	1012	643	559	326	38660	Accepted
29	185	91	71	124	346	1184	2600	3216	2697	2304	2154	2153	2299	2241	2676	3307	3533	3162	2087	1374	1001	729	605	399	40538	Accepted
30	197	109	100	122	362	1070	2521	3019	2617	2118	2314	2405	2458	2543	2947	3553	3490	3213	2306	1596	1178	965	804	542	42549	Accepted
																								2022	37977.21	
																								2018	37770	
																							A	Adjust	99.45%	



PUBLIC TRANSPORTATION SCHEDULES

### System Alert: MASKS ARE RECOMMENDED ON ALL GATRA VEHICLES



Home / Routes / Route Link 4

Link 4 Cranberry Plaza To West Wareham

Link 4 travels between Cranberry Plaza and Rt. 28 & Rt. 58 South. Stops include YMCA, Super Walmart, and Wareham Crossing.

DAYS: **DIRECTION:** Inbound Outbound Weekday Saturday Run #3 Run #4 Run #5 Stop Run #1 Run #2 **Run #6** Run #7 Run # Lakeville MBTA 7:50am Station South 8:05am Middleborough **Route 28 and Route** 8:10am 8:56am 9:56am 10:56am 11:56am 12:56pm 1:56pm 2:56r 58 Walmart RR 9:01am 10:01am 11:01am 12:01pm 1:01pm 2:01pm **3:01 Supercenter** Wareham Crossing RR 10:04am 11:04am 12:04pm 9:04am 1:04pm 2:04pm 3:04r All RR 9:08am 10:08am 11:08am 12:08pm 1:08pm 2:08pm 3:08r American/Rosebrook Mill Pond 8:14am 9:12am 10:12am 11:12am 12:12pm 1:12pm 2:12pm 3:12r **Cranberry Plaza** 8:20am 9:18am 10:18am 11:18am 12:18pm 1:18pm 2:18pm 3:18r **Onset Pier** 

Service not provided on: New Years Day, Memorial Day, 4th of July, Labor Day, Columbus Day, Thanksgiving, Christmas

## Fares

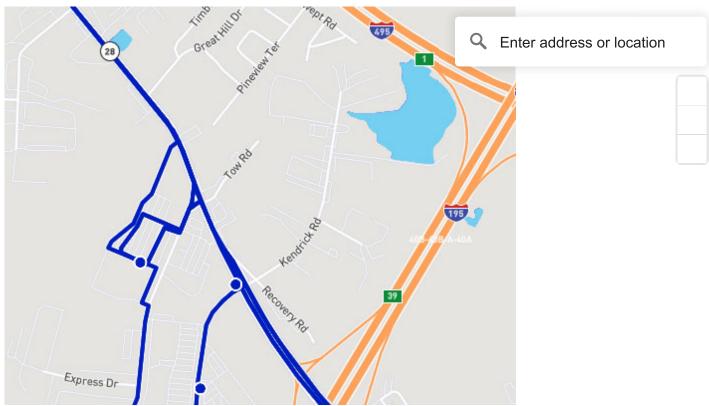


LINK 2

LINK 1

Regular	\$1.50
Seniors (60+)/Medicare Card Holder	\$0.75
Student	\$0.75
Children (6 & under with adult)	Free

Route Link 4 - GATRA



© Mapbox © OpenStreetMap

## Stop Zone Legend

Flag stop zone: Flag down the bus anywhere along this segment to ride. No stop zone: Bus will only stop at marked, designated stops in this area.

#### **About GATRA**

GATRA oversees operations of fixed-route bus services in the cities of Attleboro and Taunton, and the towns of Bellingham, Berkley, Carver, Dighton, Duxbury, Foxborough, Franklin, Hanover, Kingston, Lakeville, Mansfield, Marshfield, Medway, Middleborough, Norfolk, North Attleboro, Norton, Pembroke, Plainville, Plymouth, Plympton, Raynham, Rehoboth, Scituate, Seekonk, Wareham, and Wrentham. All 29 member communities have demand response (Dial-A-Ride) services for people with disabilities and seniors. **Contact Us** 800-483-2500 TTD: 508-824-7439 10 Oak Street Taunton, MA 02780

#### **Requests For Information In Alternate Formats**

All documents and information available on this website are available in alternate formats upon request.

How to Ride About Us MassHealth/PT-1 Rides Employment GATRA Advisory Board Open Government Procurement Reports and Policies Safety Reporting & Protocols Title VI Attleboro Commuter Parking Lot

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### System Alert: MASKS ARE RECOMMENDED ON ALL GATRA VEHICLES





# Dial-A-Ride: ADA & Senior Transportation

Home / Dial-A-Ride: ADA & Senior Transportation

## **Contact The Dial-A-Ride Office In Your Area**

Select your town to get the correct phone number: Select a town... •

## **Dial-A-Ride**

Dial-a-Ride is a curb to curb transportation service available to eligible passengers in each of GATRA's towns. Eligible residents are seniors (60 and over) or persons with a disability who cannot access public transportation.

Please use the drop box above to research the Dial-a-Ride options in your town.

If you believe you are eligible for the service, please download and fill out the ADA or Senior application below.

Senior Dial-A-Ride Application

## **ADA Paratransit**

#### Purpose

The Americans with Disabilities Act of 1990 mandated that Americans with disabilities have equal access to public transportation as well as places of "public accommodation".

In conformance with the Americans with Disabilities Act (ADA), the Greater Attleboro Regional Transit Authority (GATRA) offers a paratransit service to individuals with disabilities who cannot use the fixed route bus system.

#### **Service Description**

Operating in compliance with the FTA' s ADA regulations, GATRA's ADA Paratransit Service provides origin to destination transportation to ADA eligible individuals who are unable to us the fixed route bus service due to a disability. This service operates the same days and times as the fixed route bus service and travels within <sup>3</sup>/<sub>4</sub> of a mile of the route. It is a shared ride and multiple users could be grouped together allowing GATRA to provide efficient trips that will meet all ADA requests.

### Qualifying for ADA Paratransit

A person must apply to see if they qualify for this transportation. Download the application and fill out Part A. A licensed professional who can attest to the disability that prevents use of a fixed route bus must complete Part B. The application should be sent to the administrative office (10 Oak Street Taunton, MA 02780).

Within 21 days of receipt of the application, GATRA will review and determine rider's eligibility for ADA services. If determination has not been made within the 21 days, the applicant will be considered eligible for ADA services on day 22 until formal determination by GATRA.

#### ADA Dial-A-Ride Application Part A

ADA Dial-A-Ride Application Part B

## Eligibility for out-of-area visitors

Visitors to the GATRA area who wish to use our Paratransit service should call for information concerning eligibility. The number is 508-823-8828. The Americans with Disabilities Act (ADA) allows you to travel as a visitor for 21 days in a 12-month period. Please provide a copy of your ADA Paratransit Certificate of Eligibility from your home transit agency, along with your contact information while in the area.

# **Procedure For Filing A Complaint:**

Any person, who believes his/her ADA rights have been violated, may file a complaint with GATRA's Customer Service Department. For ADA complaints, please call Joanne LaFerrara, Director of Customer Relations at 800-283-2500 ext 275.

ADA Service Complaint Form Download

### **About GATRA**

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**Contact Us** 800-483-2500 TTD: 508-824-7439 10 Oak Street Taunton, MA 02780

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VEHICLE TRAVEL SPEED DATA

Location : Route 28 Location : East of Great Hill Drive City/State: Wareham, MA Direction: EB,

rection: EB,														
9/22/2022	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	0	4	4	6	10	5	0	0	0	0	0	0	29
1:00	0	0	0	6	8	3	1	2	0	0	0	0	0	20
2:00	0	0	3	3	5	3	3	2	0	0	0	0	0	19
3:00	0	0	1	6	6	4	1	0	0	1	0	0	0	19
4:00	0	1	2	15	17	10	2	1	0	0	0	0	0	48
5:00	0	0	6	27	35	27	3	3	0	0	0	0	0	101
6:00	0	2	9	30	109	92	31	4	0	0	0	0	0	277
7:00	0	3	10	60	170	132	29	4	1	0	0	0	0	409
8:00	4	3	27	74	178	120	21	3	1	0	0	0	0	431
9:00	0	4	22	98	165	118	21	3	0	0	0	0	0	431
10:00	7	7	32	106	204	107	26	1	0	0	0	0	0	490
11:00	5	4	26	126	199	108	15	1	1	0	0	0	0	485
12:00 PM	1	4	42	139	233	114	24	2	1	0	1	0	0	561
1:00	1	6	25	111	156	81	22	1	2	0	0	0	0	405
2:00	3	12	38	89	145	76	24	2	0	0	0	0	0	389
3:00	6	5	29	142	180	88	28	2	2	0	0	0	0	482
4:00	0	3	21	110	231	119	21	4	0	1	0	0	0	510
5:00	0	0	18	83	172	120	15	7	1	0	0	0	1	417
6:00	0	2	15	67	142	57	9	2	0	0	0	0	0	294
7:00	1	1	5	35	105	33	10	4	0	0	0	0	0	194
8:00	0	0	7	27	63	30	6	3	0	0	0	0	0	136
9:00	0	2	7	13	36	24	11	0	0	0	0	0	0	93
10:00	0	0	2	11	16	9	6	1	0	0	0	0	0	45
11:00	0	0	0	7	11	6	3	0	0	0	0	0	0	27
Total	28	59	351	1389	2592	1491	337	52	9	2	1	0	1	6312
New Line			Percentile	15th	50th	85th	95th							
			Speed	28	32	37	40							
	Mea	an Speed	(Average)	32.5										

10 MPH Pace Speed Number in Pace 30-39 4081 Percent in Pace 64.7%

Number > 35 MPH 1893 Percent > 35 MPH 30.0% Location : Route 28 Location : East of Great Hill Drive City/State: Wareham, MA Direction: EB,

ection: EB,														
9/23/2022	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	0	1	0	8	4	2	0	0	0	0	0	0	15
1:00	0	0	0	9	7	0	1	1	0	0	0	0	0	18
2:00	0	1	6	3	5	1	1	0	0	0	0	0	0	17
3:00	0	0	4	4	9	5	0	1	0	0	0	0	0	23
4:00	0	0	4	10	19	5	3	4	1	0	0	0	0	46
5:00	0	1	5	18	42	19	10	1	0	0	0	0	0	96
6:00	1	4	11	41	111	94	15	4	0	0	0	0	0	281
7:00	3	2	8	49	157	138	23	4	0	0	0	0	0	384
8:00	2	3	18	65	176	124	41	5	0	0	0	0	0	434
9:00	0	6	14	88	225	137	39	3	0	0	0	0	0	512
10:00	1	2	41	84	226	122	36	2	0	0	0	0	0	514
11:00	8	23	35	141	252	127	29	1	1	0	0	0	0	617
12:00 PM	0	6	55	159	231	128	30	4	0	0	0	0	0	613
1:00	2	4	22	140	263	144	19	2	0	0	0	0	0	596
2:00	0	6	18	110	217	133	48	8	0	0	0	0	0	540
3:00	3	5	46	167	294	178	30	1	0	0	0	0	0	724
4:00	0	23	33	131	296	188	45	2	1	0	0	0	0	719
5:00	0	1	8	61	267	199	40	5	0	0	0	0	0	581
6:00	1	0	12	45	193	162	33	3	0	0	0	0	0	449
7:00	2	0	11	52	125	95	35	2	0	0	0	0	0	322
8:00	0	0	10	24	77	60	19	4	0	0	0	0	0	194
9:00	0	1	3	20	42	39	15	2	1	0	0	0	0	123
10:00	1	0	1	6	24	25	6	0	0	0	0	0	0	63
11:00	0	0	3	7	12	12	8	1	1	0	0	0	0	44
Total	24	88	369	1434	3278	2139	528	60	5	0	0	0	0	7925
New Line			Percentile	15th	50th	85th	95th							
			Speed	29	34	38	41							
	Mea	an Speed	(Average)	33.1										

Mean Speed (Average) 33.1 10 MPH Pace Speed 30-39 Number in Pace 5403 Percent in Pace 68.2%

Number > 35 MPH 2732 Percent > 35 MPH 34.5% Location : Route 28 Location : East of Great Hill Drive City/State: Wareham, MA Direction: EB,

Direction: EB,														
9/24/2022	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	0	1	6	8	3	5	3	1	0	0	0	0	27
1:00	0	0	1	9	12	6	1	0	0	0	0	0	0	29
2:00	0	0	2	6	6	3	3	0	0	0	0	0	0	20
3:00	0	2	2	7	3	4	1	2	0	0	0	0	0	21
4:00	1	0	2	6	7	5	2	0	0	0	0	0	0	23
5:00	0	1	7	12	18	19	4	0	0	0	0	0	0	61
6:00	0	0	5	26	39	42	12	1	0	0	0	0	0	125
7:00	0	0	1	20	71	85	32	2	1	0	0	0	0	212
8:00	0	4	8	33	128	131	42	15	0	1	0	0	0	362
9:00	0	2	11	54	171	161	40	11	0	0	0	0	0	450
10:00	1	5	23	108	272	160	37	2	0	0	0	0	0	608
11:00	0	3	28	107	261	181	53	2	1	0	0	0	0	636
12:00 PM	0	2	17	112	270	207	47	9	0	0	0	0	0	664
1:00	0	1	27	112	245	147	37	5	1	0	0	0	0	575
2:00	0	7	23	93	245	177	34	1	1	0	0	0	0	581
3:00	0	4	12	62	211	198	25	4	0	0	0	0	0	516
4:00	0	0	16	48	198	188	50	1	1	0	0	0	0	502
5:00	1	3	15	36	148	149	33	3	0	0	0	0	0	388
6:00	0	1	11	44	142	145	29	5	1	0	0	0	0	378
7:00	0	2	2	33	106	75	14	2	0	0	1	0	0	235
8:00	0	0	8	33	66	37	10	2	0	0	0	0	0	156
9:00	0	0	6	17	51	39	13	1	0	1	1	0	0	129
10:00	0	0	1	12	29	29	3	3	0	0	0	0	0	77
11:00	0	0	0	2	5	5	1	0	0	0	0	0	0	13
Total	3	37	229	998	2712	2196	528	74	7	2	2	0	0	6788
New Line		ŀ	Percentile	15th	50th	85th	95th							
			Speed	29	34	38	42							
	Mea	an Speed (	(Average)	34.0										
	10	) MPH Pa	ce Speed	30-39										
		Numbe	er in Pace	4884										
		Percer	nt in Pace	72.0%										
		Number >	> 35 MPH	2809										
		Percent >	• 35 MPH	41.4%										
Grand Total		ſ	Percentile	15th	50th	85th	95th							
			Speed	29	34	38	41							
	Mea	an Speed (	(Average)	33.2										
	10	) MPH Pa	ce Speed	30-39										
		Numbe	er in Pace	14368										
		Percer	nt in Pace	68.3%										
		Number >	• 35 MPH	7434										
		Percent >	• 35 MPH	35.4%										

Location : Route 28 Location : East of Great Hill Drive City/State: Wareham, MA Direction: WB,

Direction: WB,														
9/22/2022	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	0	0	3	5	6	8	2	2	0	0	0	0	26
1:00	0	1	1	0	7	5	8	3	0	0	0	0	0	25
2:00	0	0	2	2	4	11	6	1	0	0	0	0	0	26
3:00	0	1	0	3	7	10	9	2	0	0	0	0	0	32
4:00	0	0	1	8	12	42	13	10	3	0	1	1	0	91
5:00	4	2	3	15	50	57	38	6	1	2	1	0	0	179
6:00	0	3	6	20	117	152	58	9	1	2	0	0	0	368
7:00	6	3	7	52	139	175	83	13	0	0	0	0	1	479
8:00	4	7	21	65	159	152	58	14	3	1	1	0	4	489
9:00	4	7	7	85	144	94	42	8	0	0	0	0	1	392
10:00	4	24	37	63	168	115	33	3	0	0	0	1	0	448
11:00	4	2	31	102	141	140	42	7	0	0	0	0	0	469
12:00 PM	4	22	51	87	226	152	24	10	0	0	0	0	0	576
1:00	6	6	44	127	195	101	19	3	0	0	0	0	0	501
2:00	6	16	43	115	151	91	15	1	0	0	0	0	0	438
3:00	2	13	39	142	139	90	20	0	0	0	1	0	0	446
4:00	1	4	22	101	214	117	33	2	0	0	0	0	0	494
5:00	0	9	24	86	185	112	38	4	0	1	0	0	0	459
6:00	2	2	14	52	105	116	34	5	0	0	0	0	2	332
7:00	1	4	9	34	90	62	22	6	0	0	0	0	0	228
8:00	1	5	8	12	48	68	33	3	1	0	0	0	0	179
9:00	0	0	4	14	31	51	21	1	2	0	0	0	0	124
10:00	0	0	1	5	15	22	20	5	1	0	0	0	0	69
11:00	0	0	0	3	6	17	10	1	1	0	0	0	0	38
Total	49	131	375	1196	2358	1958	687	119	15	6	4	2	8	6908
New Line			Percentile	15th	50th	85th	95th							
			Speed	28	34	39	43							
	Me	an Speed	(Average)	33.6										

Iean Speed (Average)33.610 MPH Pace Speed30-39Number in Pace4301Percent in Pace62.3%

 Number > 35 MPH
 2799

 Percent > 35 MPH
 40.5%

94810001

Location : Route 28 Location : East of Great Hill Drive City/State: Wareham, MA Direction: WB,

Direction: WB,														
9/23/2022	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	0	1	1	4	7	2	5	1	0	0	0	0	21
1:00	0	1	2	0	7	7	5	0	0	0	3	0	0	25
2:00	0	0	1	6	6	7	3	1	1	0	0	1	1	27
3:00	0	0	0	6	6	17	9	2	0	0	0	0	0	40
4:00	0	0	1	3	20	26	21	9	3	0	1	2	3	89
5:00	3	0	0	16	42	62	38	13	2	2	1	0	1	180
6:00	1	4	6	30	135	136	52	13	1	0	0	0	0	378
7:00	5	1	23	44	149	176	63	8	0	0	0	1	3	473
8:00	5	8	23	56	158	156	65	11	0	0	0	0	1	483
9:00	2	8	22	52	148	110	51	8	1	0	0	1	1	404
10:00	5	5	46	95	165	123	34	5	0	1	0	0	1	480
11:00	23	24	39	97	213	162	39	5	1	1	0	0	1	605
12:00 PM	15	14	40	121	227	193	27	6	1	0	0	0	0	644
1:00	5	15	25	92	222	172	44	8	1	0	1	0	1	586
2:00	3	2	36	98	190	197	61	6	1	0	0	0	0	594
3:00	6	16	29	98	252	172	29	10	2	1	0	0	0	615
4:00	1	10	33	73	174	220	67	11	1	0	0	0	0	590
5:00	1	9	28	53	173	234	69	12	1	0	0	0	0	580
6:00	3	12	16	50	154	175	51	8	1	1	0	0	0	471
7:00	0	1	8	35	131	116	30	5	0	0	0	1	0	327
8:00	1	1	5	21	67	102	33	5	3	0	0	0	1	239
9:00	0	0	2	9	47	66	36	6	0	0	0	0	1	167
10:00	1	0	0	9	20	43	21	7	1	0	0	0	0	102
11:00	0	0	0	5	10	31	16	6	3	0	0	0	0	71
Total	80	131	386	1070	2720	2710	866	170	25	6	6	6	15	8191
New Line			Percentile	15th	50th	85th	95th							
			Speed	29	35	40	43							

Mean Speed (Average) 34.3 10 MPH Pace Speed 30-39 Number in Pace 5397 Percent in Pace 65.9%

 Number > 35 MPH
 3804

 Percent > 35 MPH
 46.4%

94810001

Location : Route 28
Location : East of Great Hill Drive
City/State: Wareham, MA

Direction: WB,

Direction. WD,														
9/24/2022	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH									70 MPH	MPH	Total
12:00 AM	0			3				4		0	0	0	0	32
1:00	0	0	0	2	3	13	6	3	1	0	0	0	0	28
2:00	0		0	3			4	2		1	0	0	0	17
3:00	0	1	1	1	6	6	6	2	0	1	0	0	0	24
4:00	0	0	1	0	9	13	10	7	1	0	0	1	0	42
5:00	1	0	1	10	19	18	19	8	0	0	1	0	0	77
6:00	0	0	2	13	28	48	22	8	3	1	0	0	1	126
7:00	0	0	1	12	68	74	60	12	1	0	0	0	0	228
8:00	0	0	3	26				6	0	1	0	0	0	343
9:00	3	3	6	46	143			13		0	1	1	0	439
10:00	5	8		87				10	1	0	1	0	0	572
11:00	2		12	66				10	3	0	0	0	1	582
12:00 PM	2			76				1	1	0	0	1	0	591
1:00	0			72				4	1	0	0	0	0	548
2:00	2			61				6	0	1	1	1	0	595
3:00	2			73				10	1	0	0	1	1	494
4:00	0			50				14		0	0	0	0	515
5:00	0			46				6	2	0	1	0	0	457
6:00	1			26				3	0	0	0	0	0	358
7:00	2			23				5	1	0	0	0	0	267
8:00	1			34				13		0	0	0	0	300
9:00	0			8				7		1	1	1	1	167
10:00	0		1	4				8	0	1	0	0	0	100
11:00	0			0				2		0	0	0	0	19
Total	21	46		742				164	21	7	6	6	4	6921
New Line			Percentile	15th										
			Speed	30		40	43							
		an Speed		35.3										
	10	0 MPH Pa	•	30-39										
			er in Pace	4685										
			nt in Pace	67.7%										
			> 35 MPH	3734										
Grand Total			> 35 MPH	54.0%		85th	95th							
Grand Total		ļ	Percentile	15th										
	NA	on Snood	Speed	29 34.4		40	43							
		an Speed												
	10	0 MPH Pa	er in Pace	30-39 14382										
			nt in Pace	65.3%										
			> 35 MPH	10337										
			> 35 MPH	46.9%										
			55 WI 11	40.370										

Location : Route 28 Location : East of Great Hill Drive City/State: Wareham, MA

Direction: Combined 9/22/2022 0 - 15 > 25 -> 30 -> 40 -> 50 -> 15 -> 20 -> 35 -> 45 -> 55 -> 60 -> 65 -> 70 MPH 20 MPH 25 MPH 30 MPH 35 MPH 40 MPH 45 MPH 50 MPH 55 MPH 60 MPH 65 MPH 70 MPH MPH Time Total 12:00 AM 1:00 2:00 3:00 4:00 5:00 6:00 7:00 8:00 9:00 10:00 11:00 12:00 PM 1:00 2:00 3:00 4:00 5:00 6:00 7:00 8:00 9:00 10:00 11:00 Total New Line Percentile 15th 50th 85th 95th 

Speed Mean Speed (Average) 33.1 10 MPH Pace Speed 30-39 Number in Pace Percent in Pace 63.4%

Number > 35 MPH Percent > 35 MPH 35.5% Location : Route 28 Location : East of Great Hill Drive City/State: Wareham, MA

0.1.97 0.10110	
Direction:	Combined

9/23/2022	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	-	30 MPH	35 MPH		45 MPH		55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	0	2	1	12	11	4	5	1	0	0	0	0	36
1:00	0	1	2	9	14	7	6	1	0	0	3	0	0	43
2:00	0	1	7	9	11	8	4	1	1	0	0	1	1	44
3:00	0	0	4	10	15	22	9	3	0	0	0	0	0	63
4:00	0	0	5	13	39	31	24	13	4	0	1	2	3	135
5:00	3	1	5	34	84	81	48	14	2	2	1	0	1	276
6:00	2	8	17	71	246	230	67	17	1	0	0	0	0	659
7:00	8	3	31	93	306	314	86	12	0	0	0	1	3	857
8:00	7	11	41	121	334	280	106	16	0	0	0	0	1	917
9:00	2	14	36	140	373	247	90	11	1	0	0	1	1	916
10:00	6	7	87	179	391	245	70	7	0	1	0	0	1	994
11:00	31	47	74	238	465	289	68	6	2	1	0	0	1	1222
12:00 PM	15	20	95	280	458	321	57	10	1	0	0	0	0	1257
1:00	7	19	47	232	485	316	63	10	1	0	1	0	1	1182
2:00	3	8	54	208	407	330	109	14	1	0	0	0	0	1134
3:00	9	21	75	265	546	350	59	11	2	1	0	0	0	1339
4:00	1	33	66	204	470	408	112	13	2	0	0	0	0	1309
5:00	1	10	36	114	440	433	109	17	1	0	0	0	0	1161
6:00	4	12	28	95	347	337	84	11	1	1	0	0	0	920
7:00	2	1	19	87	256	211	65	7	0	0	0	1	0	649
8:00	1	1	15	45	144	162	52	9	3	0	0	0	1	433
9:00	0	1	5	29	89	105	51	8	1	0	0	0	1	290
10:00	2	0	1	15	44	68	27	7	1	0	0	0	0	165
11:00	0		3	12	22		24	7	4	0	0	0	0	115
Total	104	219	-	2504	5998	4849	1394	230	30	6	6	6	15	16116
New Line			Percentile	15th	50th	85th	95th						-	
			Speed		34	39	42							
	Mo	an Sneed		33.7	2.									

Mean Speed (Average) 33.7 10 MPH Pace Speed 30-39 Number in Pace 10800 Percent in Pace 67.0%

 Number > 35 MPH
 6536

 Percent > 35 MPH
 40.6%

Location : Route 28
Location : East of Great Hill Drive
City/State: Wareham, MA
Direction: Combined

irection: Comb														
9/24/2022	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH										MPH	Total
12:00 AM	0	0		9	14	12	15		1	0	0	0	0	59
1:00	0	0		11	15	19	7	3	1	0	0	0	0	57
2:00	0	1	2	9	6	7	7	2	2	1	0	0	0	37
3:00	0	3		8	9	10	7	4	0	1	0	0	0	45
4:00	1	0		6	16	18	12		1	0	0	1	0	65
5:00	1	1	8	22	37	37	23	8	0	0	1	0	0	138
6:00	0	0		39	67	90	34	9	3	1	0	0	1	251
7:00	0	0			139	159	92	14	2	0	0	0	0	440
8:00	0	4		59	221	276	111	21	0	2	0	0	0	705
9:00	3	5		100	314	311	113	24	0	0	1	1	0	889
10:00	6	13	34		460	377	81	12	1	0	1	0	0	1180
11:00	2	4		173	443	414	125	12	4	0	0	0	1	1218
12:00 PM	2	4		188	487	422	103	10	1	0	0	1	0	1255
1:00	0	5		184	461	322	99	9	2	0	0	0	0	1123
2:00	2	14		154	453	392	87	7	1	1	1	1	0	1176
3:00	2	9		135	360	376	75	14	1	0	0	1	1	1010
4:00	0	1	32	98	340	401	128	15	2	0	0	0	0	1017
5:00	1	10		82	280	308	101	9	2	0	1	0	0	845
6:00	1	3		70	244	298	95	8	1	0	0	0	0	736
7:00	2	3		56	203	167	51	7	1	0	1	0	0	502
8:00	1	2		67	158	150	43	15	2	0	0	0	0	456
9:00	0	0		25	86	105	51	8	0	2	2	1	1	296
10:00	0	1	2	16	46	77	23	11	0	1	0	0	0	177
11:00	0	0		2	8	14	5		0	0	0	0	0	32
Total	24	83		1740	4867	4762	1488	238	28	9	8	6	4	13709
New Line			Percentile	15th	50th	85th	95th							
			Speed	30	35	40	43							
		an Speed		34.7										
	1(	) MPH Pa	•	30-39										
			er in Pace	9569										
			nt in Pace	69.8%										
			> 35 MPH	6543										
			> 35 MPH	47.7%										
Grand Total		I	Percentile	15th	50th	85th	95th							
		-	Speed	29	34	39	42							
		an Speed		33.8										
	1(	) MPH Pa	•	30-39										
			er in Pace	28750										
			nt in Pace	66.8%										
			> 35 MPH	17772										
		Percent >	> 35 MPH	41.3%										



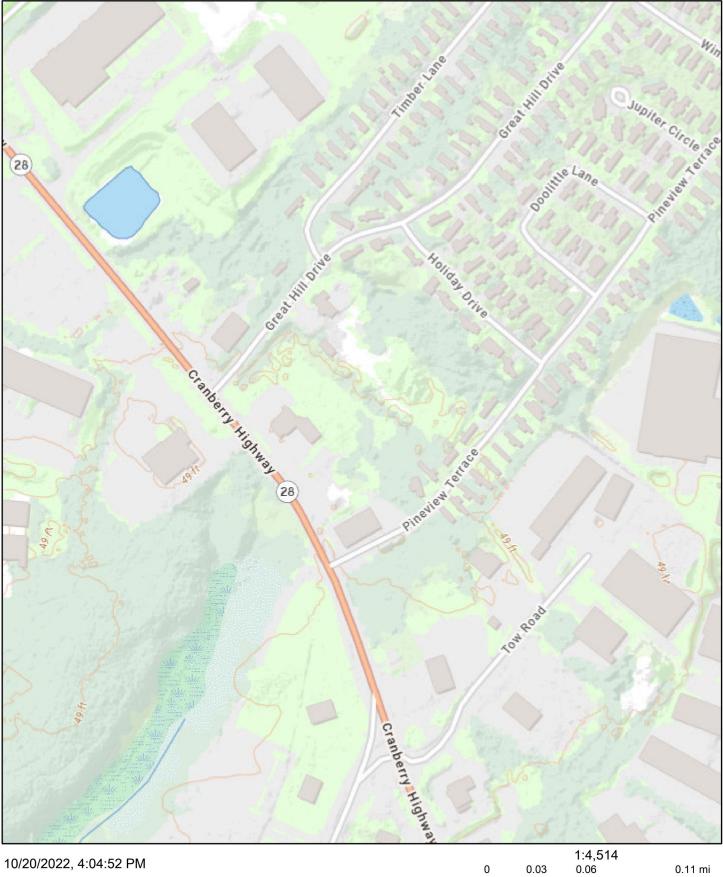
### MASSDOT CRASH RATE WORKSHEETS AND HIGH CRASH LOCATION MAP

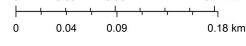


## **INTERSECTION CRASH RATE WORKSHEET**

CITY/TOWN :	Wareham			COUNT DA	TE:	Sep-22
DISTRICT : 5	UNSIGN	ALIZED :	X	SIGNA	LIZED :	
		~ IN1	TERSECTION	I DATA ~		
MAJOR STREET :	Route 28					
MINOR STREET(S) :	Project Site [	Driveway				
INTERSECTION DIAGRAM (Label Approaches)	↑ North					
			PEAK HOUF		_	Total Peak
APPROACH :	1	2	3	4	5	Hourly Approach
DIRECTION : PEAK HOURLY	EB	WB	NB	SB		Volume
VOLUMES (Midday) :	717	612				1,329
"K" FACTOR:	0.090	INTERSI	ECTION ADT APPROACH		AL DAILY	14,767
TOTAL # OF CRASHES :	8	# OF YEARS :	5	CRASHES	GE # OF PER YEAR( 、):	1.60
CRASH RATE CALCU	JLATION :	0.30	RATE =	<u>(A*1,</u> (V	000,000) * 365)	
Comments : <u>Below Sta</u> Project Title & Date:		strict Crash Ra				

## MassDOT Top Crash Locations





MassGIS



GENERAL BACKGROUND TRAFFIC GROWTH

#### General Background Traffic Growth - Daily Traffic Volumes

CITY/TOWN	ROUTE/STREET	LOCATION	2009	2010	2011	2012	2013	2014	2015	2016	2017	2018	2019	Average Annual Growth Rate
Wareham	Kendrick Road	North of Cranbery Highway						2,398	2,453	2,605	2,649	2,657	2,646	2.15%
Wareham	Main Street	East of Tobey Road					6,870	7,083	7,246	7,695	7,826	7,849	7,553	1.99%
Wareham	Hathaway Street	South of Park Street							3,057	3,247	3,302	3,312	3,299	1.79%
Wareham	Doty Street	North of Cranberry Highway						3,475	3,555	3,775	3,839	3,851	3,836	2.16%
Rochester	County Road	North of Main Street							3,941	4,185	4,256	4,269	4,252	1.78%
Wareham	Fearing Hill Road	East of County Road							3,487	3,703	3,766	3,777	3,762	1.78%
Wareham	County Road	North of Fearing Hill Road							2,837	3,013	3,064	3,073	3,061	1.78%
Wareham	Gibbs Avenue	South of Main Street	8,837	8,214	8,496	8,379	8,332	7,988	8,172	8,679	9,301	9,329	9,292	0.96%
Wareham	Main Street	East of Tremont Road	14,099	13,106	14,933	14,705	14,687	14,495	14,828	15,747	16,015	16,063	15,999	1.65%
Wareham	I-495NB	South of Route 58	70,257	62,454	64,100	59,488	59,845	61,421	64,492	68,491	51,744	52,468	49,522	-2.50%
Wareham	Main Street	East of Tobey Road					6,870	7,083	7,246	7,695	7,826	7,849	7,553	1.99%
Wareham	Mary's Pond Road	West of County Road							3,483	3,699	3,762	3,773	3,758	1.78%
Wareham	Main Street	South of Route 6					1,919	1,978	2,023			2,156	2,147	1.93%
Wareham	County Road	South of Fearing Hill Road							2,101	2,231	2,269	2,276	2,267	1.78%
Wareham	Route 25	East of I-195	46,277	47,854	56,656	55,949	55,945	56,840	59,114	61,951	67,131	67,802	69,158	3.96%
Wareham	Chapel Street	West of Main Street		8,500	8,913	8,799	7,475	7,707	7,884	7,325	7,450	7,472	7,043	-2.20%

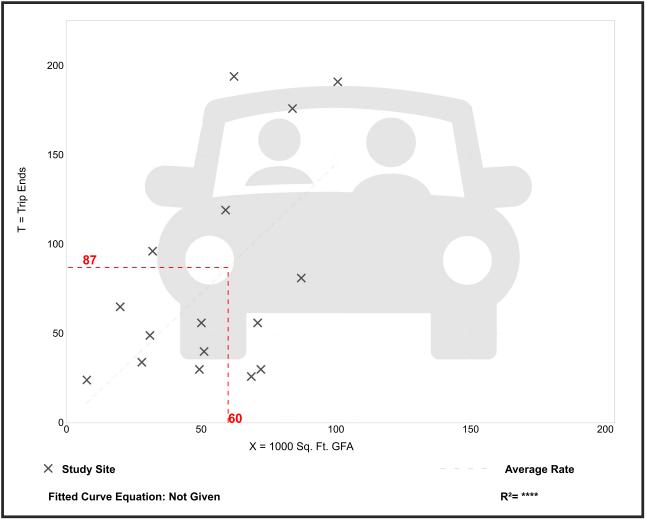
1.42%



TRIP-GENERATION CALCULATIONS

		<b>arehouse</b> 51)	
	Vehicle Trip Ends vs: On a:	1000 Sq. Ft. GFA Weekday	
	Number of Studies: Avg. 1000 Sq. Ft. GFA:		
Vehicle Trip Gener	ation per 1000 Sq. Ft. Range o	GFA	Standard Deviation
1.45	0.38 - 3		0.92

## **Data Plot and Equation**



Trip Gen Manual, 11th Edition

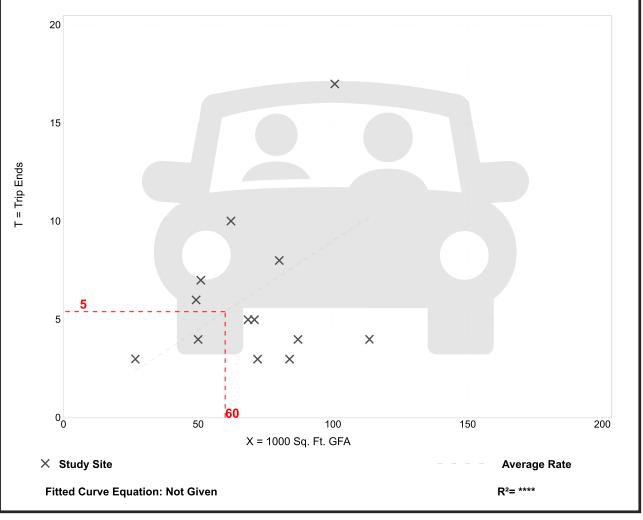
• Institute of Transportation Engineers

	<b>arehouse</b> 51)
Vehicle Trip Ends vs:	1000 Sq. Ft. GFA
On a:	Weekday,
	Peak Hour of Adjacent Street Traffic,
	One Hour Between 7 and 9 a.m.
Setting/Location:	General Urban/Suburban
Number of Studies:	13
Avg. 1000 Sq. Ft. GFA:	70
	59% entering, 41% exiting

### Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.09	0.04 - 0.17	0.05

### **Data Plot and Equation**



Trip Gen Manual, 11th Edition

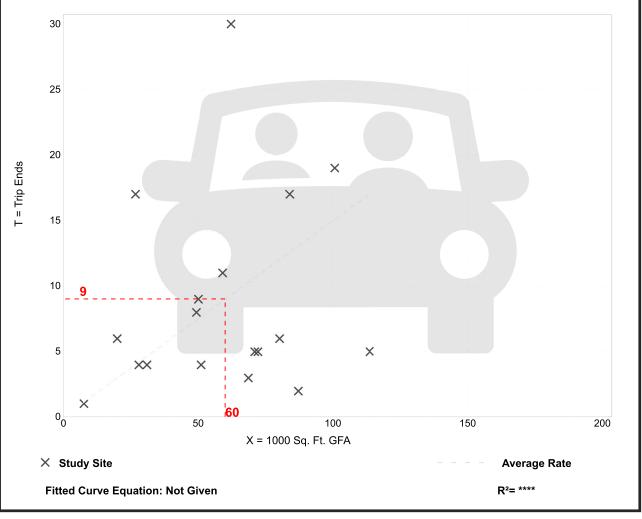
• Institute of Transportation Engineers

	<b>arehouse</b> 51)
Vehicle Trip Ends vs:	1000 Sq. Ft. GFA
On a:	Weekday,
	Peak Hour of Adjacent Street Traffic,
	One Hour Between 4 and 6 p.m.
Setting/Location:	General Urban/Suburban
Number of Studies:	18
Avg. 1000 Sq. Ft. GFA:	59
Directional Distribution:	47% entering, 53% exiting

### Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.15	0.02 - 0.64	0.14

### **Data Plot and Equation**



Trip Gen Manual, 11th Edition

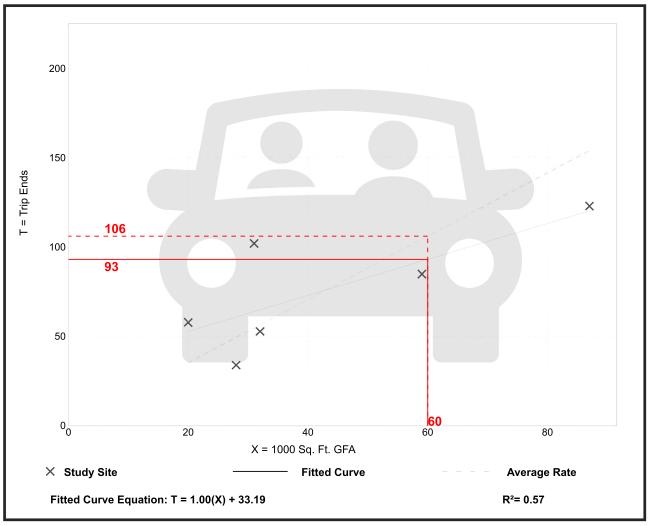
• Institute of Transportation Engineers

		<b>arehouse</b> 51)	
	Vehicle Trip Ends vs: On a:	1000 Sq. Ft. GFA Saturday	
	Number of Studies:	•	ourban
	Avg. 1000 Sq. Ft. GFA: Directional Distribution:		exiting
hicle Trip Gener	ation per 1000 Sq. Ft.	GFA	
Average Rate	Range o	f Rates	Standard Deviation

1.21 - 3.29

## **Data Plot and Equation**

1.77



Trip Gen Manual, 11th Edition

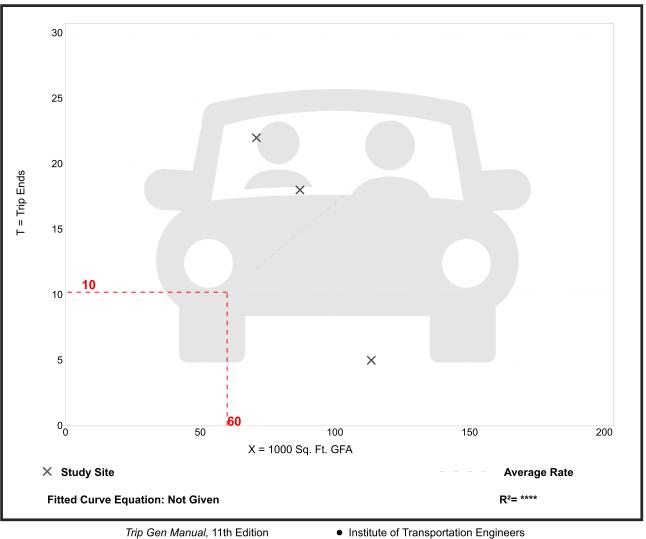
• Institute of Transportation Engineers

0.76

		<b>arehouse</b> 51)	
	Vehicle Trip Ends vs: On a:	•	FA < Hour of Generator
	Setting/Location:	General Urban	/Suburban
	Number of Studies: Avg. 1000 Sq. Ft. GFA: Directional Distribution:	90	38% exiting
Vehicle Trip Gener	ation per 1000 Sq. Ft	. GFA	
Average Rate	Range o	f Rates	Standard Deviation
0.17	0.04 - (	0.31	0.14

#### **Data Plot and Equation**

Caution – Small Sample Size



## CAPACITY ANALYSIS WORKSHEETS

Route 28 Cranberry Highway at Project Site Driveway



Route 28 Cranberry Highway at Project Site Driveway



	4	•	1	1	1	Ŧ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		eî 👘			र्च
Traffic Volume (vph)	1	1	580	2	1	506
Future Volume (vph)	1	1	580	2	1	506
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.932					
Flt Protected	0.976					
Satd. Flow (prot)	1694	0	1801	0	0	1863
Flt Permitted	0.976					
Satd. Flow (perm)	1694	0	1801	0	0	1863
Adj. Flow (vph)	1	1	644	2	1	588
Lane Group Flow (vph)	2	0	646	0	0	589
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized	1					
Control Type. Unsignalized						

Lanes, Volumes, Timings TJH Vanasse & Associates

#### Intersection

Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		et			<del>ا</del>
Traffic Vol, veh/h	1	1	580	2	1	506
Future Vol, veh/h	1	1	580	2	1	506
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	90	90	86	86
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	1	644	2	1	588

Major/Minor	Minor1	Ν	lajor1	Ν	/lajor2	
Conflicting Flow All	1235	645	0	0	646	0
Stage 1	645	-	-	-	-	-
Stage 2	590	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	195	472	-	-	939	-
Stage 1	522	-	-	-	-	-
Stage 2	554	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	195	472	-	-	939	-
Mov Cap-2 Maneuver	195	-	-	-	-	-
Stage 1	522	-	-	-	-	-
Stage 2	553	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	18.1	0	0
HCM LOS	С		

Minor Lane/Major Mvmt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)	-	-	276	939	-
HCM Lane V/C Ratio	-	-	0.008	0.001	-
HCM Control Delay (s)	-	-	18.1	8.8	0
HCM Lane LOS	-	-	С	А	Α
HCM 95th %tile Q(veh)	-	-	0	0	-

	4	•	1	1	1	Ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		el el			ę
Traffic Volume (vph)	2	3	564	2	2	568
Future Volume (vph)	2	3	564	2	2	568
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.919					
Flt Protected	0.980					
Satd. Flow (prot)	1678	0	1801	0	0	1863
Flt Permitted	0.980					
Satd. Flow (perm)	1678	0	1801	0	0	1863
Adj. Flow (vph)	2	3	641	2	2	598
Lane Group Flow (vph)	5	0	643	0	0	600
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized						
Control Type. Onsignalized						

Lanes, Volumes, Timings TJH Vanasse & Associates

#### Intersection

Int Delay, s/veh	0.1						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	•
Lane Configurations	Y		et -			<del>ا</del>	1
Traffic Vol, veh/h	2	3	564	2	2	568	5
Future Vol, veh/h	2	3	564	2	2	568	;
Conflicting Peds, #/hr	0	0	0	0	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free	;
RT Channelized	-	None	-	None	-	None	,
Storage Length	0	-	-	-	-	-	
Veh in Median Storage	,# 0	-	0	-	-	0	1
Grade, %	0	-	0	-	-	0	1
Peak Hour Factor	92	92	88	88	95	95	5
Heavy Vehicles, %	2	2	2	2	2	2	,
Mvmt Flow	2	3	641	2	2	598	5

Major/Minor	Minor1	Ν	/lajor1	Ν	/lajor2	
Conflicting Flow All	1244	642	0	0	643	0
Stage 1	642	-	-	-	-	-
Stage 2	602	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	192	474	-	-	942	-
Stage 1	524	-	-	-	-	-
Stage 2	547	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	191	474	-	-	942	-
Mov Cap-2 Maneuver	191	-	-	-	-	-
Stage 1	524	-	-	-	-	-
Stage 2	545	-	-	-	-	-
					~ ~ ~	

Approach	WB	NB	SB
HCM Control Delay, s	17.3	0	0
HCM LOS	С		

Minor Lane/Major Mvmt	NBT	NBRW	VBLn1	SBL	SBT
Capacity (veh/h)	-	-	298	942	-
HCM Lane V/C Ratio	-	-	0.018	0.002	-
HCM Control Delay (s)	-	-	17.3	8.8	0
HCM Lane LOS	-	-	С	А	Α
HCM 95th %tile Q(veh)	-	-	0.1	0	-

	4	•	1	1	1	Ŧ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		ę.			र्स
Traffic Volume (vph)	2	2	679	3	3	796
Future Volume (vph)	2	2	679	3	3	796
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.932		0.999			
Flt Protected	0.976					
Satd. Flow (prot)	1694	0	1799	0	0	1863
Flt Permitted	0.976					
Satd. Flow (perm)	1694	0	1799	0	0	1863
Adj. Flow (vph)	2	2	730	3	3	847
Lane Group Flow (vph)	4	0	733	0	0	850
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized						

Control Type: Unsignalized

#### Intersection

Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		et F			<del>ا</del>
Traffic Vol, veh/h	2	2	679	3	3	796
Future Vol, veh/h	2	2	679	3	3	796
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	93	93	94	94
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	2	2	730	3	3	847

Major/Minor	Minor1	Ν	/lajor1	Ν	/lajor2	
Conflicting Flow All	1585	732	0	0	733	0
Stage 1	732	-	-	-	-	-
Stage 2	853	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	119	421	-	-	872	-
Stage 1	476	-	-	-	-	-
Stage 2	418	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	118	421	-	-	872	-
Mov Cap-2 Maneuver	118	-	-	-	-	-
Stage 1	476	-	-	-	-	-
Stage 2	415	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	25	0	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)	-	-	184	872	-
HCM Lane V/C Ratio	-	-	0.024	0.004	-
HCM Control Delay (s)	-	-	25	9.1	0
HCM Lane LOS	-	-	D	А	А
HCM 95th %tile Q(veh)	-	-	0.1	0	-