STORMWATER REPORT

Proposed Large-Scale Ground-Mounted Solar Photovoltaic Installation

0 Route 25
Wareham, Massachusetts

PREPARED FOR

Wareham PV I, LLC
330 Congress Street, 6th Floor
Boston, MA 02210
617.377.4301

PREPARED BY

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August 2021

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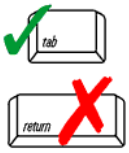
Checklist for Stormwater Report

Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.[1] This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8[2]
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

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[1] The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

[2] For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.

Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature

Signature and Date 09/01/2021

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

☒ New development

☐ Redevelopment

☐ Mix of New Development and Redevelopment

Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

☒ No disturbance to any Wetland Resource Areas

☐ Site Design Practices (e.g. clustered development, reduced frontage setbacks)

☐ Reduced Impervious Area (Redevelopment Only)

☐ Minimizing disturbance to existing trees and shrubs

☐ LID Site Design Credit Requested:

- ☐ Credit 1
- ☐ Credit 2
- ☐ Credit 3

☐ Use of "country drainage" versus curb and gutter conveyance and pipe

☐ Bioretention Cells (includes Rain Gardens)

☐ Constructed Stormwater Wetlands (includes Gravel Wetlands designs)

☐ Treebox Filter

☐ Water Quality Swale

☒ Grass Channel

☐ Green Roof

☒ Other (describe): Surface Infiltration Basins

Standard 1: No New Untreated Discharges

☒ No new untreated discharges

☒ Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth

☐ Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.

Checklist for Stormwater Report

Checklist (continued)

Standard 2: Peak Rate Attenuation

☐ Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.

☐ Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.

☐ Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

☒ Soil Analysis provided.

☒ Required Recharge Volume calculation provided. No net increase of impervious area, therefore required recharge volume is 0. Provided volume calculations are included.

☐ Required Recharge volume reduced through use of the LID site Design Credits.

☒ Sizing the infiltration, BMPs is based on the following method: Check the method used.

☒ Static ☐ Simple Dynamic ☐ Dynamic Field[1]

☐ Runoff from all impervious areas at the site discharging to the infiltration BMP.

☐ Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.

☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume.

☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:

- ☐ Site is comprised solely of C and D soils and/or bedrock at the land surface
- ☐ M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
- ☐ Solid Waste Landfill pursuant to 310 CMR 19.000
- ☐ Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.

☒ Calculations showing that the infiltration BMPs will drain in 72 hours are provided.

☐ Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

[1] 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.

Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

☐ The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.

☐ Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

No net increase of impervious area,
therefore required water quality volume is 0.
Provided volume calculations are included.

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.

☒ A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.

☐ Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:

- ☐ is within the Zone II or Interim Wellhead Protection Area
- ☐ is near or to other critical areas
- ☐ is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
- ☐ involves runoff from land uses with higher potential pollutant loads.

☐ The Required Water Quality Volume is reduced through use of the LID site Design Credits.

☐ Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.

Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

☐ The BMP is sized (and calculations provided) based on:

- ☐ The ½" or 1" Water Quality Volume or
- ☐ The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.

☐ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.

☐ A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs) N/A

☐ The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.

☐ The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted ***prior to*** the discharge of stormwater to the post-construction stormwater BMPs.

☐ The NPDES Multi-Sector General Permit does ***not*** cover the land use.

☐ LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.

☐ All exposure has been eliminated.

☐ All exposure has ***not*** been eliminated and all BMPs selected are on MassDEP LUHPPL list.

☐ The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas Project does not discharge to or near a critical area.

☐ The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.

☐ Critical areas and BMPs are identified in the Stormwater Report.

Checklist for Stormwater Report

Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable N/A

☐ The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:

- ☐ Limited Project
- ☐ Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
- ☐ Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
- ☐ Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
- ☐ Bike Path and/or Foot Path
- ☐ Redevelopment Project
- ☐ Redevelopment portion of mix of new and redevelopment.

☐ Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.

☐ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.

☒ A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.

Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has ***not*** been included in the Stormwater Report but will be submitted ***before*** land disturbance begins.

☐ The project is ***not*** covered by a NPDES Construction General Permit.

☐ The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.

☒ The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

☒ The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:

- ☒ Name of the stormwater management system owners;
- ☐ Party responsible for operation and maintenance;
- ☒ Schedule for implementation of routine and non-routine maintenance tasks;
- ☒ Plan showing the location of all stormwater BMPs maintenance access areas;
- ☐ Description and delineation of public safety features;
- ☐ Estimated operation and maintenance budget; and
- ☒ Operation and Maintenance Log Form.

☐ The responsible party is ***not*** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:

- ☐ A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
- ☐ A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

☒ The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;

☐ An Illicit Discharge Compliance Statement is attached;

☐ NO Illicit Discharge Compliance Statement is attached but will be submitted ***prior to*** the discharge of any stormwater to post-construction BMPs.

Stormwater Report Narrative

This Stormwater Report has been prepared to demonstrate compliance with the Massachusetts Stormwater Management Standards in accordance with the Massachusetts Wetlands Protection Act Regulations (310 CMR 10.00).

Project Description

The Applicant, Wareham PV I, LLC, is proposing to construct a ±3.5-megawatt (AC) large ground-mounted solar energy facility (the Project) located at 0 Route 25 in Wareham, Massachusetts (the Site). As proposed, the Project consists of approximately 12.1 acres of solar panels, utility infrastructure, access road, fencing and related amenities to support this use.

Site Description

The Project Site is a 22.4-acre parcel of land (the Site) located at 0 Route 25 in Wareham, Massachusetts (see Figure 1) which is noted as Map 115, Parcel 1000 according to the Wareham assessor's department. The Site lies within the surface watershed of Buzzards Bay and is bounded by undeveloped woodlands to the north, Route 25 to the south, existing cranberry bogs to the east, and commercial development to the west. See Figure 1, Site Locus Map. The Site is located within the Zone X flood area hazard, with minimal risk of flooding. See Figure 4.

Wetland Resource Areas on the Site include the following:

Table 1 Existing Conditions Hydrologic Data

Name	Critical Area (yes/no)	Zone 1 or Zone A (yes/no)	ORW or SRW (yes/no)	Zone II or IWPA (yes/no)	Other
Wetland 1	No	No	No	No	Isolated Vegetated Wetland
Wetland 2	No	No	No	No	Perennial stream and wetland located off-site, buffer zone encroaches on property

For additional information regarding the wetland resource areas present on the site see the Project Notice of Intent prepared by VHB dated September 2021.

VHB conducted two preliminary test pits on April 14, 2021 in the vicinity of proposed infiltration. Information gathered indicated that the soils on site consisted of Sandy Loam from 4" below surface grade to approximately 24"-27". Groundwater was encountered at 44" below surface grade at TP1-1 and 50" below surface grade at TP1-2. These elevations were used as estimated seasonal high ground water (ESHGW) for modeling purposes. Additional test pits will be conducted prior to construction in accordance with the Massachusetts DEP Stormwater Handbook.

According to the National Resources Conservation Service (NRCS), surface soils on the Site include Carver Coarse Sand (252B), Windsor Loamy Sand (255A), Windsor Loamy Sand (255B), and Deerfield Loamy Fine Sand (256A). On-site soils are classified as Hydrologic Soil Groups (HSG) A. Based on the soils information included in Appendix C, the Site is not considered to be within an area of rapid infiltration (soils with a saturated hydraulic conductivity greater than 2.4 inches per hour).

Existing Drainage Conditions

Under existing conditions, the portion of the Site north of the existing gravel access road is predominately cleared land consisting of bare sandy areas with no tree cover, with rolling flat topography of existing mounds of sand and depressions throughout. The existing depressions have been modeled as detention ponds and are shown to pond approximately 6" in the 100-year storm. For storms greater than the 100-year storm, runoff overtops the depressions and overflows to the adjacent cranberry bogs.

The portion of the Site to the south of the gravel access road is mostly undeveloped forest, with portions of bare sandy areas, and an existing gravel path that runs throughout the Site. This area is mostly hilly, with slopes ranging from 2% to 15%. Based on existing grades, it appears that there is overflow runoff onto the Site from the municipal building directly to the east.

Figure 2 illustrates the existing drainage conditions. Currently, the Site is divided into eight (8) drainage areas as stormwater runoff flows to three (3) Design Points, which have been identified as DP-1: Off-Site, DP-2: Wetlands, and DP-3: Existing Cranberry Bogs. DP-1 is located at the southwestern corner of the site, just north of Route 25. DP-2 is designated as the area of wetlands to the southeast and northern corner of the site, and DP-3 is designated as the existing cranberry bogs to the east of the site along the eastern edge of the site.

Table 2 below provides a summary of the existing conditions hydrologic data.

Table 2 Existing Conditions Hydrologic Data

Drainage Area	Discharge Location	Design Point	Area (Acres)	Curve Number	Time of Concentration (min)
EX-1	Off-Site	1	2.79	30	45.4
EX-2	Wetlands	2	7.52	31	36.6
EX-3	Existing Bogs	3	1.76	37	30.9
EX-4	Existing Bogs	3	2.03	58	20.1
EX-5	Existing Bogs	3	3.79	72	6.5
EX-6	Existing Bogs	3	1.94	67	4.3
EX-7	Existing Bogs	3	2.48	63	1.8
EX-8	Wetlands	2	0.14	45	7.1

Proposed Drainage Conditions

Figure 3 illustrates the proposed "post construction" drainage conditions for the project. As shown, the Site will be divided into nine (9) drainage areas that discharge treated stormwater to the three existing Design Points. Table 3 below provides a summary of the proposed conditions hydrologic data.

Table 3 Proposed Conditions Hydrologic Data

Drainage Area	Discharge Location	Design Point	Area (Acres)	Curve Number	Time of Concentration (min)
PR-1	Off-site	DP-1	2.47	35	14.4
PR-2	Wetlands	DP-2	7.18	33	19.0
PR-3	Wetlands	DP-2	1.49	38	17.3
PR-4	Existing Bogs	DP-3	0.93	30	10.6
PR-5	Existing Bogs	DP-3	1.37	35	12.8
PR-6	Existing Bogs	DP-3	4.44	36	22.0
PR-7	Existing Bogs	DP-3	1.94	34	17.1
PR-8	Existing Bogs	DP-3	2.48	33	8.4
PR-9	Wetlands	DP-2	0.14	45	11.5

Overall, there are no changes to the drainage patterns on site as there is minimal grading proposed on site. Any grading occurring on site will be due to the construction of a level gravel access path, or from partially leveling out the sand stockpiles in order to install the panels. Any runoff from the adjacent municipal site will continue to take the same drainage path as it currently does under existing conditions.

Within the limit of work, excluding the proposed gravel drive and infiltration basins, a meadow mix seed mix is proposed for groundcover. The meadow mix has no impact on the rate of runoff from the wooded areas of the project and provides a decreased curve number in the areas of existing sand.

A 20' wide gravel path has been proposed for fire and maintenance access to the panels. Where feasible, the path follows the alignment of the existing path to minimize disturbance. New gravel roads have been graded to drain towards grassed swales, conveying runoff to the surface infiltration basins. Electrical conduit is proposed to be installed underground within the limits of the existing 20' wide access easement from the Site to Charge Pond Road. Where disturbance for construction occurs, the gravel road will be rebuilt in kind, and the landscape area adjacent to the road will be loamed and seeded.

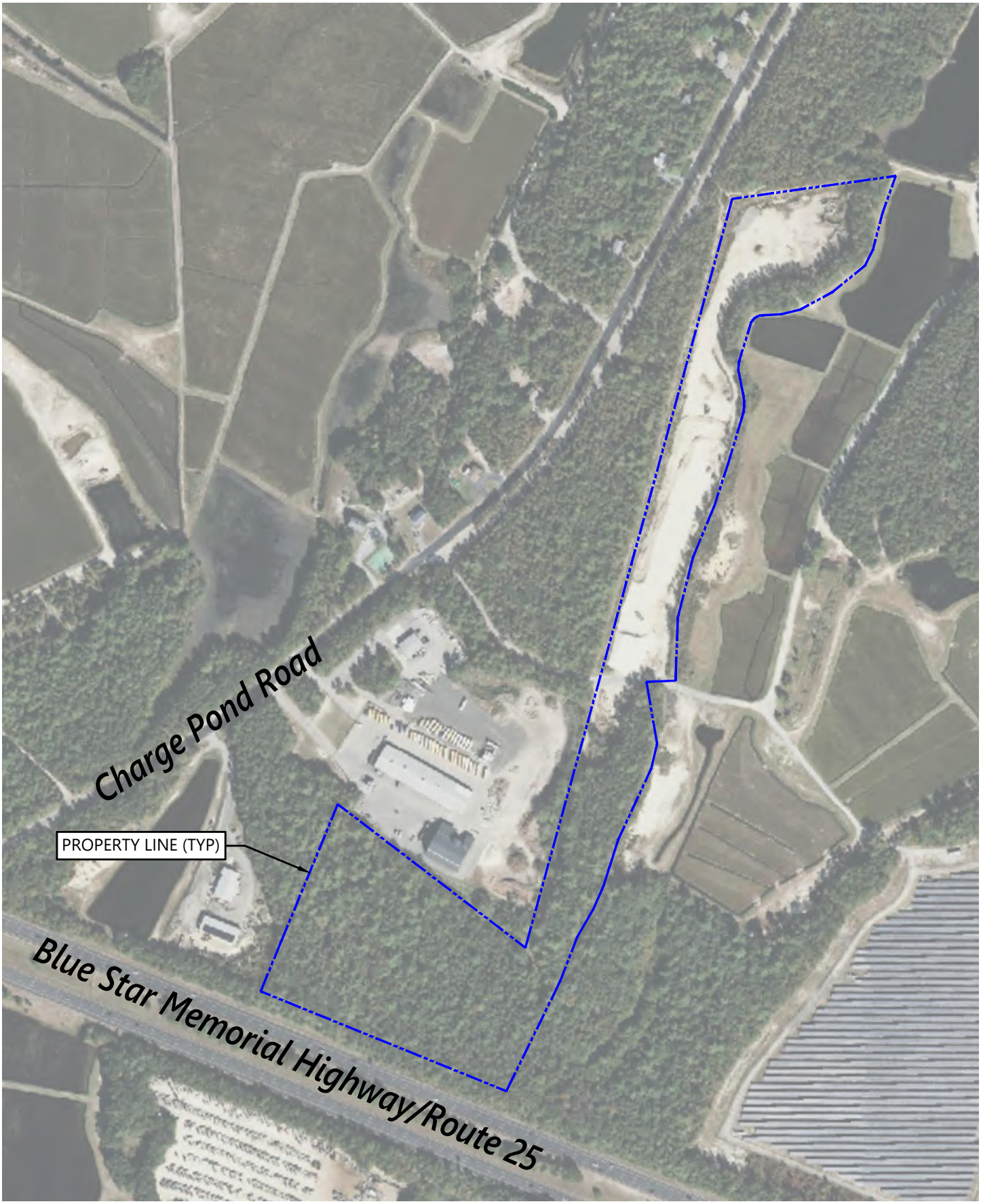
In general, stormwater runoff from drainage areas of the site where clearing is to occur and the gravel path is proposed, (proposed subcatchments PR-2 and PR-3) is collected by infiltration basins designed to mitigate peak runoff. The basins have been designed to provide a 2-foot separation between the bottom of basin and estimated seasonal high groundwater (ESHGW).

Due to the similar groundcover proposed within the tree clearing of the southeast portion of the site (proposed subcatchment PR-1), and improved groundcover in the remaining drainage areas on the northern portion of the site (proposed subcatchments PR-4 through PR-9), peaks are mitigated without the need of additional infiltration.

As there is a de minimis increase in impervious area based on the installation of three small concrete pads, the site design integrates a comprehensive stormwater management system that has been developed in accordance with the Massachusetts Stormwater Handbook.

Low Impact Development (LID) Techniques

Low Impact Development (LID) techniques and stormwater Best Management Practices (BMPs) implemented into the site design include maintaining existing stormwater runoff conditions, no net increase of impervious area, and no impact to adjacent wetlands.



Locus Map

Figure 1

0 Route 25
Wareham, MA

8/2/2021

0 200 400 Feet

0 50 100 200Feet

Legend

SYMBOLS

- DESIGN POINT
- DRAINAGE AREA DESIGNATION
- POND

LINETYPES

- DRAINAGE AREA BOUNDARY
- TIME OF CONCENTRATION FLOW LINE
- SOIL TYPE BOUNDARY
- 100' BUFFER ZONE
- WETLAND BOUNDARY

- EX-1
- EX-2
- EX-3
- EX-4
- EX-5
- EX-6
- EX-7
- EX-8

SCS SOIL CLASSIFICATIONS

- 37A — MASSASOIT - MASHPEE COMPLEX, 0 TO 3 PERCENT SLOPES, HSG D
- 60A — SWANSEA COARSE SAND, 0 TO 2 PERCENT SLOPES, HSG B/D
- 252B — CARVER COARSE SAND, 3 TO 8 PERCENT SLOPES, HSG A
- 255A — WINDSOR LOAMY SAND, 0 TO 3 PERCENT SLOPES, HSG A
- 255B — WINDSOR LOAMY SAND, 3 TO 8 PERCENT SLOPES, HSG A
- 256A — DEERFIELD LOAMY FINE SAND, 0 TO 3 PERCENT SLOPES, HSG A
- 602B — URBAN LAND, 0 TO 8 PERCENT SLOPES
- 665B — UDIPSAMMENTS, 0 TO 8 PERCENT SLOPES, HSG A

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0 Route 25
Wareham, MA

No.	Revision	Date	Appvd.

Designed by SKE | Checked by JRG

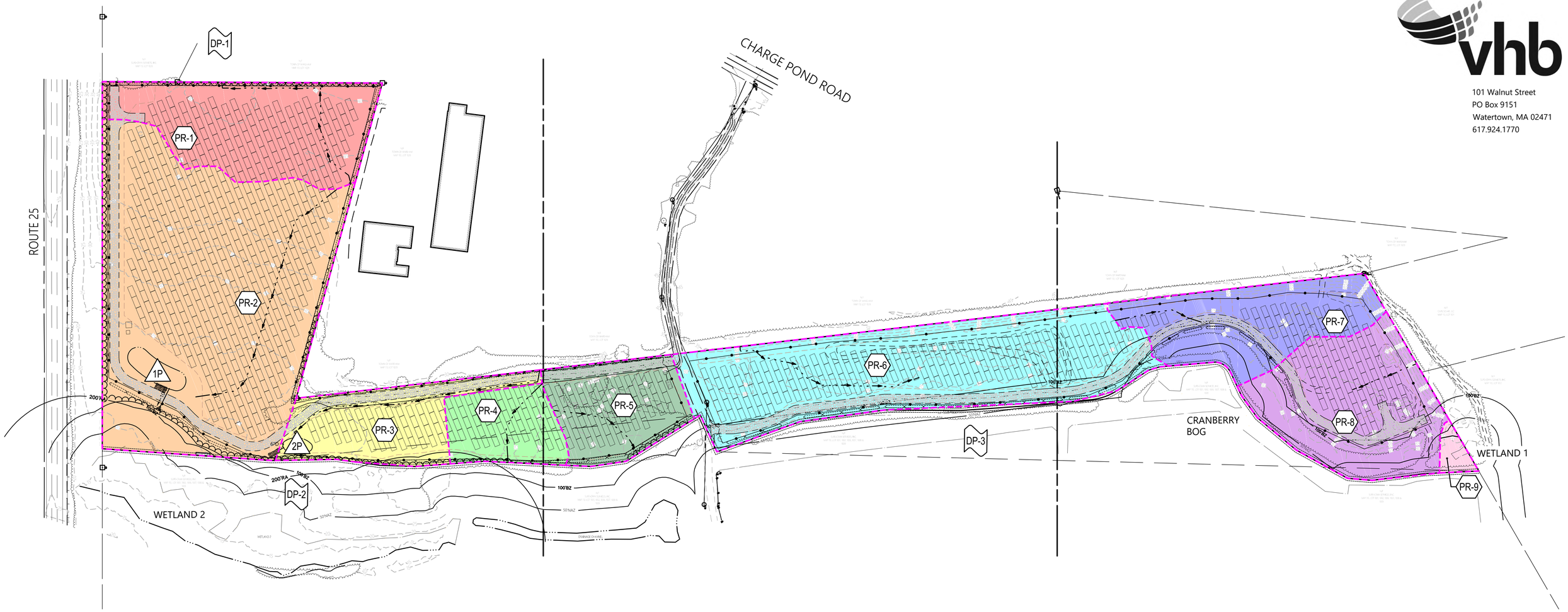
Issued for: Local Approvals — Date: 8/2/2021

Drawing Title: Existing Drainage

Drawing Number: Fig 2

Sheet 2 of 3

Project Number: 15225.01



0 50 100 200Feet

Legend

SYMBOLS

- DESIGN POINT
- DRAINAGE AREA DESIGNATION
- INFILTRATION BASIN

LINETYPES

- DRAINAGE AREA BOUNDARY
- TIME OF CONCENTRATION FLOW LINE
- SOIL TYPE BOUNDARY
- 100' BUFFER ZONE
- WETLAND BOUNDARY

- PR-1
- PR-2
- PR-3
- PR-4
- PR-5
- PR-6
- PR-7
- PR-8
- PR-9

SCS SOIL CLASSIFICATIONS

- MASSASOIT - MASHPEE COMPLEX, 0 TO 3 PERCENT SLOPES, HSG D
- SWANSEA COARSE SAND, 0 TO 2 PERCENT SLOPES, HSG B/D
- CARVER COARSE SAND, 3 TO 8 PERCENT SLOPES, HSG A
- WINDSOR LOAMY SAND, 0 TO 3 PERCENT SLOPES, HSG A
- WINDSOR LOAMY SAND, 3 TO 8 PERCENT SLOPES, HSG A
- DEERFIELD LOAMY FINE SAND, 0 TO 3 PERCENT SLOPES, HSG A
- URBAN LAND, 0 TO 8 PERCENT SLOPES
- UDIPSAMMENTS, 0 TO 8 PERCENT SLOPES, HSG A

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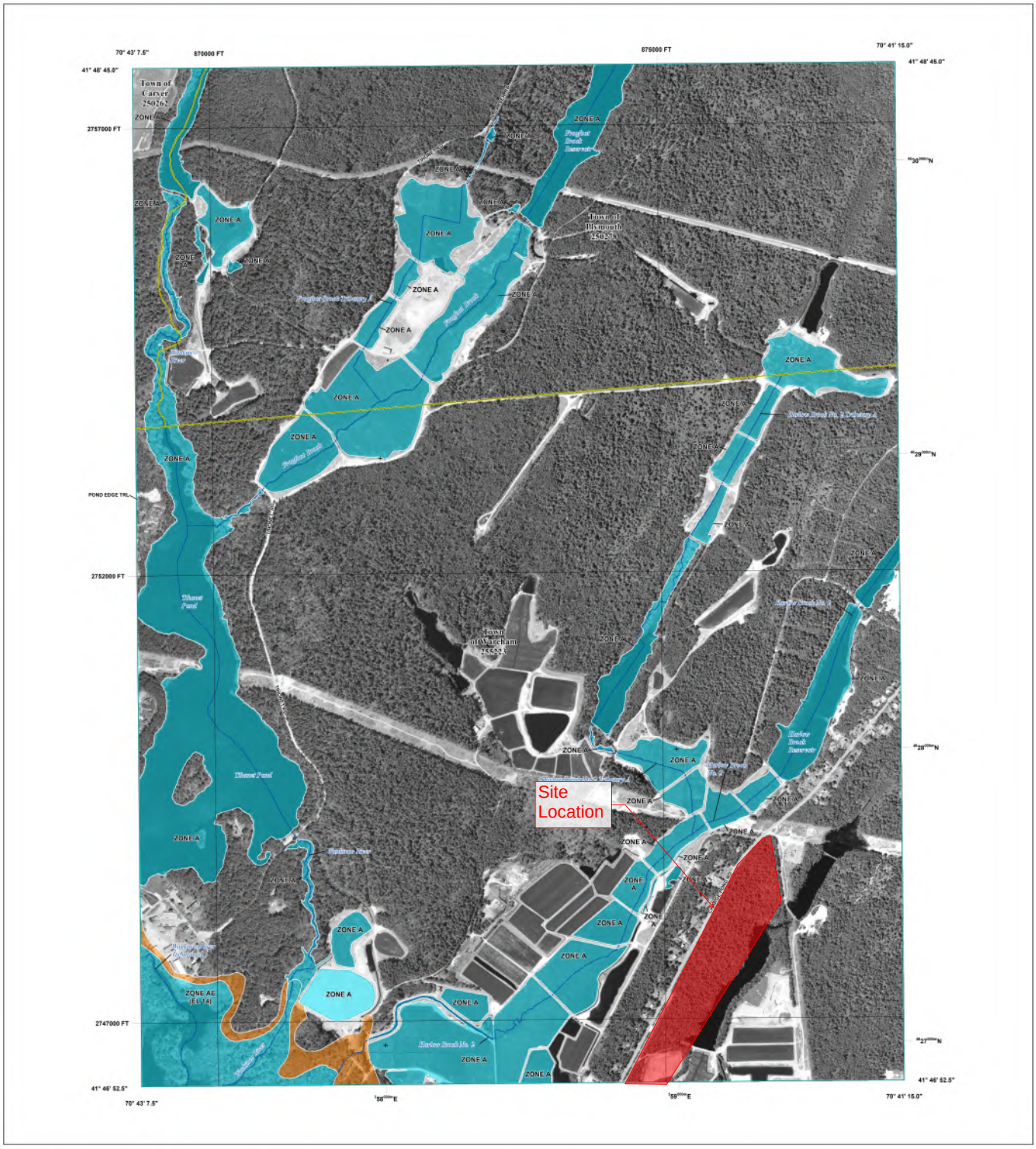
Drawing Title
Proposed Drainage

Drawing Number
Fig 3

Sheet 3 of 3

Project Number
15225.01



FLOOD HAZARD INFORMATION

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

THE INFORMATION DEPICTED ON THIS MAP AND SUPPORTING DOCUMENTATION ARE ALSO AVAILABLE IN DIGITAL FORMAT AT HTTPS://MSC.FEMA.GOV

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) Zone A, V, A99
- With BFE or Depth Zone AE, AO, AH, VE, AR
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
- Future Conditions 1% Annual Chance Flood Hazard Zone X
- Area with Reduced Flood Risk due to Levee See Notes. Zone X
- Area with Flood Risk due to Levee Zone D

OTHER AREAS
- NO SCREEN Area of Minimal Flood Hazard Zone X
- Area of Undetermined Flood Hazard Zone D

GENERAL STRUCTURES
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall
- 16.2 / 17.5 Cross Sections with 1% Annual Chance Water Surface Elevation
- Coastal Transect
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature
- 513 Base Flood Elevation Line (BFE)

OTHER FEATURES
- Limit of Study
- Jurisdiction Boundary

NOTES TO USERS

For information and questions about this Flood Insurance Rate Map (FIRM), available products associated with this FIRM, including historic versions, the current map date for each FIRM panel, how to order products, or the National Flood Insurance Program (NFIP) in general, please call the FEMA Map Information eXchange at 1-877-FEMA-MAP (1-877-336-2627) or visit the FEMA Flood Map Service Center website at https://msc.fema.gov. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. Many of these products can be ordered or obtained directly from the website.

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For community and countywide map dates refer to the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

Base map information shown on the FIRM was 2013 and 2014 imagery provided by the U.S. Geological Survey and 2016 transportation data provided by the U.S. Census Bureau, with all other vector data unchanged from the previous FIRM.

SCALE

Map Projection:
NAD 1983 State Plane, Massachusetts Mainland, FIPS 2001, Feet;
Western Hemisphere; Vertical Datum: NAVD 88

1 inch = 500 feet 1:6,000

0 500 1,000 2,000 Feet

0 130 260 520 Meters

PANEL LOCATOR

* PANEL NOT PRINTED

FEMA

National Flood Insurance Program

NATIONAL FLOOD INSURANCE PROGRAM

FLOOD INSURANCE RATE MAP

PLYMOUTH COUNTY, MASSACHUSETTS

(ALL JURISDICTIONS)

PANEL 487 OF 650

FEMA

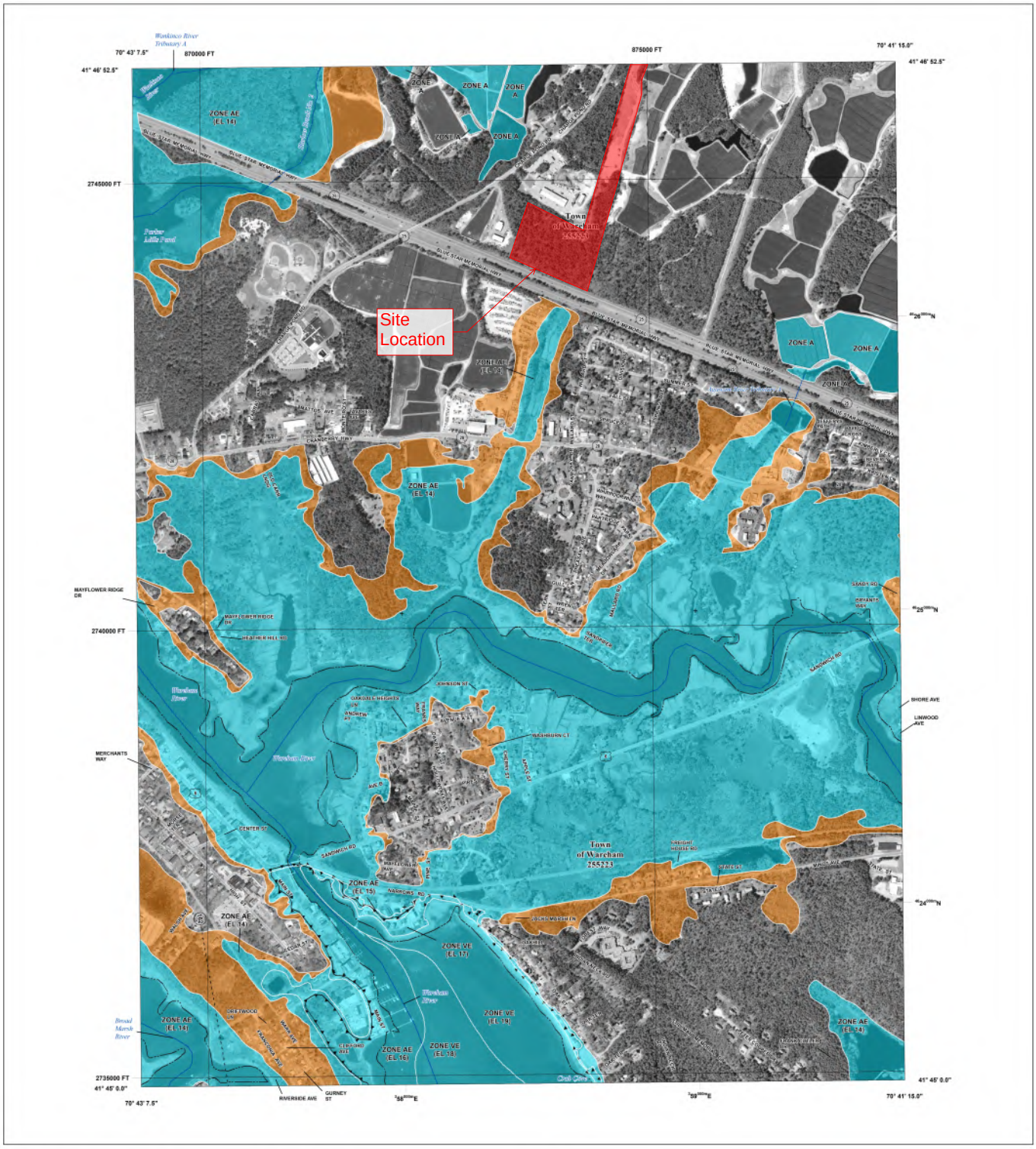
Panel Contains:

COMMUNITY	NUMBER	PANEL	SUFFIX
CARVER, TOWN OF	250262	0487	K
PLYMOUTH, TOWN OF	250278	0487	K
WAREHAM, TOWN OF	255223	0487	K

VERSION NUMBER
2.6.3.5

MAP NUMBER
25023C0487K

MAP REVISED
July 6, 2021



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- Profile Baseline
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Base map information shown on the FIRM uses 2013 and 2014 imagery provided by the U.S. Geological Survey and 2016 transportation data provided by the U.S. Census Bureau, with all other vector data unchanged from the previous FIRM.

LIMIT OF MODERATE WAVE ACTION: Zone AE has been divided by a Limit of Moderate Wave Action (LiMWA). The LiMWA represents the approximate landward limit of the 1.5-foot breaking wave. The effects of wave hazards between Zone VE and the LiMWA (or between the shoreline and the LiMWA for areas where Zone VE is not identified) will be similar to, but less severe than, those in the Zone VE.

Limit of Moderate Wave Action (LiMWA)

SCALE

Map Projection: NAD 1983 State Plane, Massachusetts Mainland, FIPS 2001, Feet; Western Hemisphere; Vertical Datum: NAVD 88

1 inch = 500 feet 1:6,000

0 500 1,000 2,000 Feet

0 130 260 520 Meters

PANEL LOCATOR

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FEMA

National Flood Insurance Program

NATIONAL FLOOD INSURANCE PROGRAM

FLOOD INSURANCE RATE MAP

PLYMOUTH COUNTY, MASSACHUSETTS

(ALL JURISDICTIONS)

PANEL 489 OF 650

FEMA

Panel Contains:

COMMUNITY	NUMBER	PANEL	SUFFIX
WAREHAM, TOWN OF	255223	0489	L

VERSION NUMBER 2.6.3.5

MAP NUMBER 25023C0489L

MAP REVISED July 6, 2021

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Regulatory Compliance

Massachusetts Department of Environmental Protection (DEP) – Stormwater Management Standards

As demonstrated below, the proposed Project fully complies with the DEP Stormwater Management Standards.

Standard 1: No New Untreated Discharges or Erosion to Wetlands

The Project has been designed to comply with Standard 1.

The Best Management Practices (BMPs) included in the proposed stormwater management system have been designed in accordance with the Massachusetts Stormwater Handbook. Supporting information and computations demonstrating that no new untreated discharges will result from the Project are presented through compliance with Standards 4 through 6.

All proposed Project stormwater outlets and conveyances from infiltration basins have been designed to not cause erosion or scour to wetlands or receiving waters. Outlets from closed drainage systems have been designed with flared end sections and stone protection to dissipate discharge velocities. Overflows from BMP's that impound stormwater have been designed with stone to protect downgradient areas from erosion.

Calculations and supporting information are included in Appendix A.

Standard 2: Peak Rate Attenuation

The Project has been designed to comply with Standard 2.

The rainfall-runoff response of the Site under existing and proposed conditions was analyzed for storm events with recurrence intervals of 2, 10, 25 and 100 years. Rainfall volumes used for this analysis were based on the Natural Resources Conservation Service (NRCS) Type III, 24-hour storm and NOAA Atlas 14 precipitation depths for the site: 3.44, 5.04, 6.04, and 7.58 inches, respectively. The results of the analysis, as summarized in Table 4 below, indicate that there is no increase in peak discharge rates between the existing and proposed conditions.

Computations and supporting information regarding the hydrologic modeling are included in Appendix B.

Table 4 Peak Discharge Rates (cfs*)

Design Point	2-year	10-year	25-year	100-year
DP-1: Off-Site				
Existing	0.0	0.0	0.0	0.1
Proposed	0.0	0.0	0.0	0.0
DP-2: Wetlands				
Existing	0.0	0.0	0.1	0.6
Proposed	0.0	0.0	0.1	0.2
DP-3: Existing Cranberry Bogs				
Existing	0.4	1.6	2.6	4.5
Proposed	0.0	0.1	0.4	2.6

Standard 3: Stormwater Recharge

The Project has been designed to comply with Standard 3.

The Project does not result in an increase of impervious area. Therefore, in accordance with the Stormwater Handbook, the Required Recharge Volume for the Project is 0 cubic feet. The proposed infiltration basins provide a total Recharge Volume of 9,250 cubic feet.

Soil evaluation (including Geotechnical Report), computations, and supporting information are included in Appendix C.

Standard 4: Water Quality

The Project has been designed to comply with Standard 4.

The Project does not result in an increase of impervious area. Therefore, in accordance with the Stormwater Handbook, the Required Water Quality Treatment Volume is 0 cubic feet.

However, the stormwater management system implements a treatment train of BMPs that have been designed to provide a water quality treatment volume of 9,250 cubic feet within the two infiltration basins.

Supporting calculations are included in Appendix D.

Standard 5: Land Uses with Higher Potential Pollutant Loads (LUHPPLs)

The Project is not considered a LUHPPL.

Standard 6: Critical Areas

The Project will not discharge stormwater near or to a critical area.

Infiltration basins are proposed to mitigate stormwater runoff rates. Runoff will infiltrate or flow overland through undisturbed wooded areas before entering the bordering vegetated wetlands. The infiltration basins are located outside of the 50' no-disturb-zone (NDZ).

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the Maximum Extent Practicable

The Project has been designed to comply with all ten of the Stormwater Management Standards.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Controls

The Project will disturb approximately 19.3 acres of land and is therefore required to obtain coverage under the Environmental Protection Agency (EPA) National Pollutant Discharge Elimination System (NPDES) Construction General Permit. As required under this permit, a Stormwater Pollution Prevention Plan (SWPPP) will be developed and submitted before land disturbance begins. Recommended construction period pollution prevention and erosion and sedimentation controls to be finalized in the SWPPP are included in Appendix E.

Standard 9: Operation and Maintenance Plan

In compliance with Standard 9, a Post Construction Stormwater Operation and Maintenance (O&M) Plan has been developed for the Project. The O&M Plan is included in Appendix F.

Standard 10: Prohibition of Illicit Discharges

The site was previously undeveloped, and no sanitary sewer or storm drainage infrastructure is known to exist on the site. The design plans submitted with this report have been designed in full compliance with current standards. The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges.

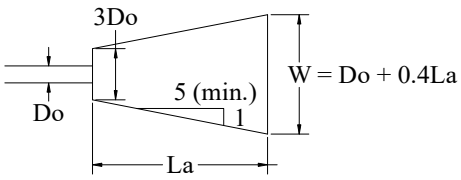
Appendix A: Standard 1 Computations and Supporting Information

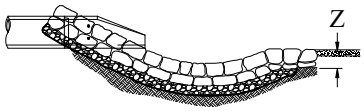
Riprap Sizing & Velocity Calculations

Outfall Riprap Sizing and Velocity Calculations

Project	Proposed Solar Array	**Project #**	15225.01
Calculated by	SKE	**Date**	07/15/21
Checked by	JRG	**Date**	07/15/21

Tw ≥ 0.5Do





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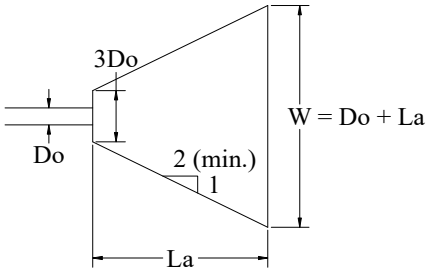
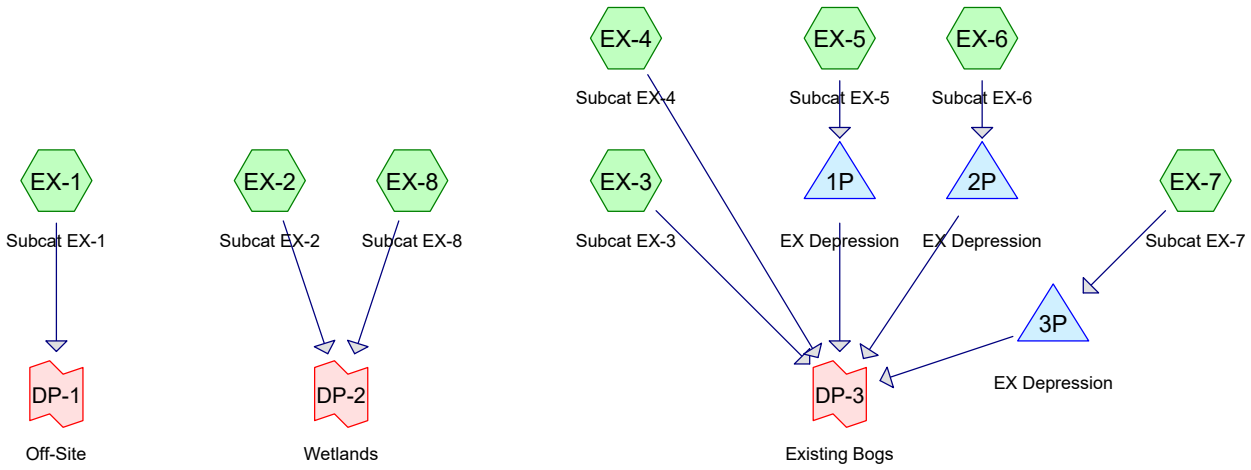


OUTLET DESCRIPTION:		**FES-1**	**FES-2**	
Design Storm	(yr)	100	100	
Flow / Discharge (Q)	(cfs)	0.0	0.2	
Defined Channel ?	-	YES	YES	
Defined Channel Width	(ft)	3	3	
Outlet Pipe Diameter (D_O)	(in)	10	6	
Tailwater Condition (T_W)	(ft)	TW < 0.5D	TW < 0.5D	
Apron Length (L_A)	(ft)	10	10	minimum length
Apron Width at Outlet ($3D_O$)	(ft)	3	3	minimum width
Apron Width at End (W)	(ft)	3	3	minimum width
Median Stone Diameter (d_{50})	(in)	6	6	
Largest Stone Diameter	(in)	9	9	
Apron Depth (Z)	(in)	13.5	13.5	

Appendix B: Standard 2 Computations and Supporting Information

The rainfall-runoff response of the Site under existing and proposed conditions was evaluated for storm events with recurrence intervals of 2, 10, 25 and 100-years. Rainfall volumes used for this analysis were based on the Natural Resources Conservation Service (NRCS) Type III, 24-hour storm and NOAA Atlas 14 precipitation depths for the site: 3.44, 5.04, 6.04, and 7.58 inches, respectively. Runoff coefficients for the pre- and post-development conditions, as previously shown in Tables 2 and 3 respectively, were determined using NRCS Technical Release 55 (TR-55) methodology as provided in HydroCAD. Drainage areas used in the analyses were described in previous sections and shown on Figures 2 and 3. The HydroCAD model is based on the NRCS Technical Release 20 (TR-20) Model for Project Formulation Hydrology.

HydroCAD Analysis: Existing Conditions



Subcat Reach Pond Link

Routing Diagram for EX-Drainage
Prepared by VHB, Printed 07/16/21

Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-Year	Type III 24-hr		Default	24.00	1	3.44	2
2	10-Year	Type III 24-hr		Default	24.00	1	5.04	2
3	25-Year	Type III 24-hr		Default	24.00	1	6.04	2
4	100-Year	Type III 24-hr		Default	24.00	1	7.58	2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
9.341	72	Dirt roads, HSG A (EX-2, EX-3, EX-4, EX-5, EX-6, EX-7, EX-8)
0.003	89	Dirt roads, HSG D (EX-8)
0.013	98	Water Surface, 0% imp, HSG A (EX-8)
13.080	30	Woods, Good, HSG A (EX-1, EX-2, EX-3, EX-4, EX-5, EX-6, EX-7, EX-8)
22.437	**48**	**TOTAL AREA**

2-Year Storm Event – Existing

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EX-1: Subcat EX-1 Runoff Area=2.785 ac 0.00% Impervious Runoff Depth=0.00"
Flow Length=357' Tc=45.4 min CN=30 Runoff=0.00 cfs 0.000 af

Subcatchment EX-2: Subcat EX-2 Runoff Area=7.521 ac 0.00% Impervious Runoff Depth=0.00"
Flow Length=668' Tc=36.6 min CN=31 Runoff=0.00 cfs 0.000 af

Subcatchment EX-3: Subcat EX-3 Runoff Area=1.759 ac 0.00% Impervious Runoff Depth=0.00"
Flow Length=384' Tc=30.9 min CN=37 Runoff=0.00 cfs 0.000 af

Subcatchment EX-4: Subcat EX-4 Runoff Area=2.031 ac 0.00% Impervious Runoff Depth=0.43"
Flow Length=207' Tc=20.1 min CN=58 Runoff=0.42 cfs 0.073 af

Subcatchment EX-5: Subcat EX-5 Runoff Area=3.788 ac 0.00% Impervious Runoff Depth=1.08"
Flow Length=433' Tc=6.5 min CN=72 Runoff=4.39 cfs 0.342 af

Subcatchment EX-6: Subcat EX-6 Runoff Area=1.939 ac 0.00% Impervious Runoff Depth=0.82"
Flow Length=212' Tc=5.0 min CN=67 Runoff=1.63 cfs 0.132 af

Subcatchment EX-7: Subcat EX-7 Runoff Area=2.477 ac 0.00% Impervious Runoff Depth=0.63"
Flow Length=153' Tc=5.0 min CN=63 Runoff=1.44 cfs 0.130 af

Subcatchment EX-8: Subcat EX-8 Runoff Area=0.137 ac 0.00% Impervious Runoff Depth=0.06"
Flow Length=41' Slope=0.1960 '/' Tc=7.1 min CN=44 Runoff=0.00 cfs 0.001 af

Pond 1P: EX Depression Peak Elev=25.09' Storage=2,502 cf Inflow=4.39 cfs 0.342 af
Discarded=1.66 cfs 0.342 af Primary=0.00 cfs 0.000 af Outflow=1.66 cfs 0.342 af

Pond 2P: EX Depression Peak Elev=25.14' Storage=1,071 cf Inflow=1.63 cfs 0.132 af
Discarded=0.55 cfs 0.132 af Primary=0.00 cfs 0.000 af Outflow=0.55 cfs 0.132 af

Pond 3P: EX Depression Peak Elev=25.02' Storage=378 cf Inflow=1.44 cfs 0.130 af
Discarded=1.07 cfs 0.130 af Primary=0.00 cfs 0.000 af Outflow=1.07 cfs 0.130 af

Link DP-1: Off-Site Inflow=0.00 cfs 0.000 af
Primary=0.00 cfs 0.000 af

Link DP-2: Wetlands Inflow=0.00 cfs 0.001 af
Primary=0.00 cfs 0.001 af

Link DP-3: Existing Bogs Inflow=0.42 cfs 0.073 af
Primary=0.42 cfs 0.073 af

Total Runoff Area = 22.437 ac Runoff Volume = 0.677 af Average Runoff Depth = 0.36"
100.00% Pervious = 22.437 ac 0.00% Impervious = 0.000 ac

Summary for Subcatchment EX-1: Subcat EX-1

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
2.785	30	Woods, Good, HSG A
2.785		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
39.4	50	0.0040	0.02		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
6.0	307	0.0290	0.85		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
45.4	357	Total			

Summary for Subcatchment EX-2: Subcat EX-2

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
0.234	72	Dirt roads, HSG A
7.287	30	Woods, Good, HSG A
7.521	31	Weighted Average
7.521		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
26.3	50	0.0110	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
10.3	618	0.0400	1.00		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
36.6	668	Total			

Summary for Subcatchment EX-3: Subcat EX-3

Runoff = 0.00 cfs @ 24.18 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
0.306	72	Dirt roads, HSG A
1.453	30	Woods, Good, HSG A
1.759	37	Weighted Average
1.759		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.7	50	0.0200	0.04		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
10.2	334	0.0120	0.55		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
30.9	384	Total			

Summary for Subcatchment EX-4: Subcat EX-4

Runoff = 0.42 cfs @ 12.43 hrs, Volume= 0.073 af, Depth= 0.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
1.339	72	Dirt roads, HSG A
0.692	30	Woods, Good, HSG A
2.031	58	Weighted Average
2.031		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.8	50	0.0340	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
3.3	157	0.0250	0.79		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
20.1	207	Total			

Summary for Subcatchment EX-5: Subcat EX-5

Runoff = 4.39 cfs @ 12.11 hrs, Volume= 0.342 af, Depth= 1.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
3.782	72	Dirt roads, HSG A
0.006	30	Woods, Good, HSG A
3.788	72	Weighted Average
3.788		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	50	0.2040	0.94		**Sheet Flow, Dirt** Fallow n= 0.050 P2= 3.44"
5.6	383	0.0050	1.14		**Shallow Concentrated Flow, Dirt** Unpaved Kv= 16.1 fps
6.5	433	Total			

Summary for Subcatchment EX-6: Subcat EX-6

Runoff = 1.63 cfs @ 12.09 hrs, Volume= 0.132 af, Depth= 0.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
1.698	72	Dirt roads, HSG A
0.241	30	Woods, Good, HSG A
1.939	67	Weighted Average
1.939		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.3	50	0.0200	0.37		**Sheet Flow,** Fallow n= 0.050 P2= 3.44"
2.0	162	0.0070	1.35		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
4.3	212	Total, Increased to minimum Tc = 5.0 min			

Summary for Subcatchment EX-7: Subcat EX-7

Runoff = 1.44 cfs @ 12.10 hrs, Volume= 0.130 af, Depth= 0.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
1.960	72	Dirt roads, HSG A
0.517	30	Woods, Good, HSG A
2.477	63	Weighted Average
2.477		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.7	50	0.3700	1.19		**Sheet Flow,** Fallow n= 0.050 P2= 3.44"
1.1	103	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
1.8	153	Total, Increased to minimum Tc = 5.0 min			

Summary for Subcatchment EX-8: Subcat EX-8

Runoff = 0.00 cfs @ 15.13 hrs, Volume= 0.001 af, Depth= 0.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
0.022	72	Dirt roads, HSG A
0.003	89	Dirt roads, HSG D
0.013	98	Water Surface, 0% imp, HSG A
0.099	30	Woods, Good, HSG A
0.137	44	Weighted Average
0.137		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.1	41	0.1960	0.10		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"

Summary for Pond 1P: EX Depression

Inflow Area = 3.788 ac, 0.00% Impervious, Inflow Depth = 1.08" for 2-Year event
Inflow = 4.39 cfs @ 12.11 hrs, Volume= 0.342 af
Outflow = 1.66 cfs @ 12.44 hrs, Volume= 0.342 af, Atten= 62%, Lag= 19.9 min
Discarded = 1.66 cfs @ 12.44 hrs, Volume= 0.342 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 25.09' @ 12.44 hrs Surf.Area= 29,003 sf Storage= 2,502 cf

Plug-Flow detention time= 11.2 min calculated for 0.341 af (100% of inflow)
Center-of-Mass det. time= 11.2 min (875.6 - 864.4)

Volume	Invert	Avail.Storage	Storage Description
#1	25.00'	111,585 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
25.00	25,532	0	0
26.00	63,362	44,447	44,447
27.00	70,913	67,138	111,585

Device	Routing	Invert	Outlet Devices
#1	Discarded	25.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 21.40'
#2	Primary	26.50'	**30.0' long x 38.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Discarded OutFlow Max=1.66 cfs @ 12.44 hrs HW=25.09' (Free Discharge)
1=Exfiltration (Controls 1.66 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=25.00' (Free Discharge)
2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2P: EX Depression

Inflow Area = 1.939 ac, 0.00% Impervious, Inflow Depth = 0.82" for 2-Year event
Inflow = 1.63 cfs @ 12.09 hrs, Volume= 0.132 af
Outflow = 0.55 cfs @ 12.47 hrs, Volume= 0.132 af, Atten= 66%, Lag= 22.6 min
Discarded = 0.55 cfs @ 12.47 hrs, Volume= 0.132 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 25.14' @ 12.47 hrs Surf.Area= 9,608 sf Storage= 1,071 cf

Plug-Flow detention time= 14.5 min calculated for 0.132 af (100% of inflow)
Center-of-Mass det. time= 14.5 min (894.6 - 880.0)

Volume	Invert	Avail.Storage	Storage Description
#1	25.00'	57,563 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
25.00	5,405	0	0
26.00	34,860	20,133	20,133
27.00	40,000	37,430	57,563

Device	Routing	Invert	Outlet Devices
#1	Discarded	25.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 21.40'
#2	Primary	26.50'	**68.0' long x 53.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Discarded OutFlow Max=0.55 cfs @ 12.47 hrs HW=25.14' (Free Discharge)
1=Exfiltration (Controls 0.55 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=25.00' (Free Discharge)
2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 3P: EX Depression

Inflow Area = 2.477 ac, 0.00% Impervious, Inflow Depth = 0.63" for 2-Year event
Inflow = 1.44 cfs @ 12.10 hrs, Volume= 0.130 af
Outflow = 1.07 cfs @ 12.19 hrs, Volume= 0.130 af, Atten= 26%, Lag= 5.5 min
Discarded = 1.07 cfs @ 12.19 hrs, Volume= 0.130 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 25.02' @ 12.19 hrs Surf.Area= 21,324 sf Storage= 378 cf

Plug-Flow detention time= 5.9 min calculated for 0.130 af (100% of inflow)
Center-of-Mass det. time= 5.9 min (901.8 - 895.9)

Volume	Invert	Avail.Storage	Storage Description
#1	25.00'	72,787 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
25.00	20,982	0	0
26.00	40,160	30,571	30,571
27.00	44,271	42,216	72,787

Device	Routing	Invert	Outlet Devices
#1	Discarded	25.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 21.40'
#2	Primary	26.50'	**78.0' long x 16.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Discarded OutFlow Max=1.20 cfs @ 12.19 hrs HW=25.02' (Free Discharge)
↑—**1=Exfiltration** (Controls 1.20 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=25.00' (Free Discharge)
↑—**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link DP-1: Off-Site

Inflow Area = 2.785 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: Wetlands

Inflow Area = 7.658 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.00 cfs @ 15.13 hrs, Volume= 0.001 af
Primary = 0.00 cfs @ 15.13 hrs, Volume= 0.001 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-3: Existing Bogs

Inflow Area = 11.994 ac, 0.00% Impervious, Inflow Depth = 0.07" for 2-Year event
Inflow = 0.42 cfs @ 12.43 hrs, Volume= 0.073 af
Primary = 0.42 cfs @ 12.43 hrs, Volume= 0.073 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

10-Year Storm Event – Existing

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EX-1: Subcat EX-1 Runoff Area=2.785 ac 0.00% Impervious Runoff Depth=0.01"
Flow Length=357' Tc=45.4 min CN=30 Runoff=0.00 cfs 0.001 af

Subcatchment EX-2: Subcat EX-2 Runoff Area=7.521 ac 0.00% Impervious Runoff Depth=0.02"
Flow Length=668' Tc=36.6 min CN=31 Runoff=0.02 cfs 0.009 af

Subcatchment EX-3: Subcat EX-3 Runoff Area=1.759 ac 0.00% Impervious Runoff Depth=0.14"
Flow Length=384' Tc=30.9 min CN=37 Runoff=0.03 cfs 0.021 af

Subcatchment EX-4: Subcat EX-4 Runoff Area=2.031 ac 0.00% Impervious Runoff Depth=1.19"
Flow Length=207' Tc=20.1 min CN=58 Runoff=1.64 cfs 0.202 af

Subcatchment EX-5: Subcat EX-5 Runoff Area=3.788 ac 0.00% Impervious Runoff Depth=2.23"
Flow Length=433' Tc=6.5 min CN=72 Runoff=9.50 cfs 0.704 af

Subcatchment EX-6: Subcat EX-6 Runoff Area=1.939 ac 0.00% Impervious Runoff Depth=1.83"
Flow Length=212' Tc=5.0 min CN=67 Runoff=4.05 cfs 0.296 af

Subcatchment EX-7: Subcat EX-7 Runoff Area=2.477 ac 0.00% Impervious Runoff Depth=1.53"
Flow Length=153' Tc=5.0 min CN=63 Runoff=4.20 cfs 0.317 af

Subcatchment EX-8: Subcat EX-8 Runoff Area=0.137 ac 0.00% Impervious Runoff Depth=0.41"
Flow Length=41' Slope=0.1960 '/' Tc=7.1 min CN=44 Runoff=0.02 cfs 0.005 af

Pond 1P: EX Depression Peak Elev=25.27' Storage=8,381 cf Inflow=9.50 cfs 0.704 af
Discarded=2.13 cfs 0.704 af Primary=0.00 cfs 0.000 af Outflow=2.13 cfs 0.704 af

Pond 2P: EX Depression Peak Elev=25.35' Storage=3,621 cf Inflow=4.05 cfs 0.296 af
Discarded=0.92 cfs 0.296 af Primary=0.00 cfs 0.000 af Outflow=0.92 cfs 0.296 af

Pond 3P: EX Depression Peak Elev=25.12' Storage=2,626 cf Inflow=4.20 cfs 0.317 af
Discarded=1.34 cfs 0.317 af Primary=0.00 cfs 0.000 af Outflow=1.34 cfs 0.317 af

Link DP-1: Off-Site Inflow=0.00 cfs 0.001 af
Primary=0.00 cfs 0.001 af

Link DP-2: Wetlands Inflow=0.02 cfs 0.014 af
Primary=0.02 cfs 0.014 af

Link DP-3: Existing Bogs Inflow=1.64 cfs 0.223 af
Primary=1.64 cfs 0.223 af

Total Runoff Area = 22.437 ac Runoff Volume = 1.554 af Average Runoff Depth = 0.83"
100.00% Pervious = 22.437 ac 0.00% Impervious = 0.000 ac

Summary for Subcatchment EX-1: Subcat EX-1

Runoff = 0.00 cfs @ 24.04 hrs, Volume= 0.001 af, Depth= 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
2.785	30	Woods, Good, HSG A
2.785		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
39.4	50	0.0040	0.02		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
6.0	307	0.0290	0.85		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
45.4	357	Total			

Summary for Subcatchment EX-2: Subcat EX-2

Runoff = 0.02 cfs @ 22.65 hrs, Volume= 0.009 af, Depth= 0.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
0.234	72	Dirt roads, HSG A
7.287	30	Woods, Good, HSG A
7.521	31	Weighted Average
7.521		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
26.3	50	0.0110	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
10.3	618	0.0400	1.00		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
36.6	668	Total			

Summary for Subcatchment EX-3: Subcat EX-3

Runoff = 0.03 cfs @ 14.85 hrs, Volume= 0.021 af, Depth= 0.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
0.306	72	Dirt roads, HSG A
1.453	30	Woods, Good, HSG A
1.759	37	Weighted Average
1.759		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.7	50	0.0200	0.04		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
10.2	334	0.0120	0.55		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
30.9	384	Total			

Summary for Subcatchment EX-4: Subcat EX-4

Runoff = 1.64 cfs @ 12.32 hrs, Volume= 0.202 af, Depth= 1.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
1.339	72	Dirt roads, HSG A
0.692	30	Woods, Good, HSG A
2.031	58	Weighted Average
2.031		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.8	50	0.0340	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
3.3	157	0.0250	0.79		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
20.1	207	Total			

Summary for Subcatchment EX-5: Subcat EX-5

Runoff = 9.50 cfs @ 12.10 hrs, Volume= 0.704 af, Depth= 2.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
3.782	72	Dirt roads, HSG A
0.006	30	Woods, Good, HSG A
3.788	72	Weighted Average
3.788		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	50	0.2040	0.94		**Sheet Flow, Dirt** Fallow n= 0.050 P2= 3.44"
5.6	383	0.0050	1.14		**Shallow Concentrated Flow, Dirt** Unpaved Kv= 16.1 fps
6.5	433	Total			

Summary for Subcatchment EX-6: Subcat EX-6

Runoff = 4.05 cfs @ 12.08 hrs, Volume= 0.296 af, Depth= 1.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
1.698	72	Dirt roads, HSG A
0.241	30	Woods, Good, HSG A
1.939	67	Weighted Average
1.939		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.3	50	0.0200	0.37		**Sheet Flow,** Fallow n= 0.050 P2= 3.44"
2.0	162	0.0070	1.35		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
4.3	212	Total, Increased to minimum Tc = 5.0 min			

Summary for Subcatchment EX-7: Subcat EX-7

Runoff = 4.20 cfs @ 12.09 hrs, Volume= 0.317 af, Depth= 1.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
1.960	72	Dirt roads, HSG A
0.517	30	Woods, Good, HSG A
2.477	63	Weighted Average
2.477		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.7	50	0.3700	1.19		**Sheet Flow,** Fallow n= 0.050 P2= 3.44"
1.1	103	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
1.8	153	Total, Increased to minimum Tc = 5.0 min			

Summary for Subcatchment EX-8: Subcat EX-8

Runoff = 0.02 cfs @ 12.34 hrs, Volume= 0.005 af, Depth= 0.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
0.022	72	Dirt roads, HSG A
0.003	89	Dirt roads, HSG D
0.013	98	Water Surface, 0% imp, HSG A
0.099	30	Woods, Good, HSG A
0.137	44	Weighted Average
0.137		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.1	41	0.1960	0.10		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"

Summary for Pond 1P: EX Depression

Inflow Area = 3.788 ac, 0.00% Impervious, Inflow Depth = 2.23" for 10-Year event
Inflow = 9.50 cfs @ 12.10 hrs, Volume= 0.704 af
Outflow = 2.13 cfs @ 12.55 hrs, Volume= 0.704 af, Atten= 78%, Lag= 26.8 min
Discarded = 2.13 cfs @ 12.55 hrs, Volume= 0.704 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 25.27' @ 12.55 hrs Surf.Area= 35,860 sf Storage= 8,381 cf

Plug-Flow detention time= 29.8 min calculated for 0.703 af (100% of inflow)
Center-of-Mass det. time= 29.8 min (872.4 - 842.6)

Volume	Invert	Avail.Storage	Storage Description
#1	25.00'	111,585 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
25.00	25,532	0	0
26.00	63,362	44,447	44,447
27.00	70,913	67,138	111,585

Device	Routing	Invert	Outlet Devices
#1	Discarded	25.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 21.40'
#2	Primary	26.50'	**30.0' long x 38.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Discarded OutFlow Max=2.13 cfs @ 12.55 hrs HW=25.27' (Free Discharge)
1=Exfiltration (Controls 2.13 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=25.00' (Free Discharge)
2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2P: EX Depression

Inflow Area = 1.939 ac, 0.00% Impervious, Inflow Depth = 1.83" for 10-Year event
Inflow = 4.05 cfs @ 12.08 hrs, Volume= 0.296 af
Outflow = 0.92 cfs @ 12.52 hrs, Volume= 0.296 af, Atten= 77%, Lag= 26.4 min
Discarded = 0.92 cfs @ 12.52 hrs, Volume= 0.296 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 25.35' @ 12.52 hrs Surf.Area= 15,573 sf Storage= 3,621 cf

Plug-Flow detention time= 34.4 min calculated for 0.296 af (100% of inflow)
Center-of-Mass det. time= 34.4 min (888.4 - 854.1)

Volume	Invert	Avail.Storage	Storage Description
#1	25.00'	57,563 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
25.00	5,405	0	0
26.00	34,860	20,133	20,133
27.00	40,000	37,430	57,563

Device	Routing	Invert	Outlet Devices
#1	Discarded	25.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 21.40'
#2	Primary	26.50'	**68.0' long x 53.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Discarded OutFlow Max=0.92 cfs @ 12.52 hrs HW=25.34' (Free Discharge)
1=Exfiltration (Controls 0.92 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=25.00' (Free Discharge)
2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 3P: EX Depression

Inflow Area = 2.477 ac, 0.00% Impervious, Inflow Depth = 1.53" for 10-Year event
Inflow = 4.20 cfs @ 12.09 hrs, Volume= 0.317 af
Outflow = 1.34 cfs @ 12.46 hrs, Volume= 0.317 af, Atten= 68%, Lag= 22.2 min
Discarded = 1.34 cfs @ 12.46 hrs, Volume= 0.317 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 25.12' @ 12.46 hrs Surf.Area= 23,259 sf Storage= 2,626 cf

Plug-Flow detention time= 13.7 min calculated for 0.316 af (100% of inflow)
Center-of-Mass det. time= 13.7 min (878.7 - 865.0)

Volume	Invert	Avail.Storage	Storage Description
#1	25.00'	72,787 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
25.00	20,982	0	0
26.00	40,160	30,571	30,571
27.00	44,271	42,216	72,787

Device	Routing	Invert	Outlet Devices
#1	Discarded	25.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 21.40'
#2	Primary	26.50'	**78.0' long x 16.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Discarded OutFlow Max=1.34 cfs @ 12.46 hrs HW=25.12' (Free Discharge)
↑—**1=Exfiltration** (Controls 1.34 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=25.00' (Free Discharge)
↑—**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link DP-1: Off-Site

Inflow Area = 2.785 ac, 0.00% Impervious, Inflow Depth = 0.01" for 10-Year event
Inflow = 0.00 cfs @ 24.04 hrs, Volume= 0.001 af
Primary = 0.00 cfs @ 24.04 hrs, Volume= 0.001 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: Wetlands

Inflow Area = 7.658 ac, 0.00% Impervious, Inflow Depth = 0.02" for 10-Year event
Inflow = 0.02 cfs @ 12.34 hrs, Volume= 0.014 af
Primary = 0.02 cfs @ 12.34 hrs, Volume= 0.014 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-3: Existing Bogs

Inflow Area = 11.994 ac, 0.00% Impervious, Inflow Depth = 0.22" for 10-Year event
Inflow = 1.64 cfs @ 12.32 hrs, Volume= 0.223 af
Primary = 1.64 cfs @ 12.32 hrs, Volume= 0.223 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

25-Year Storm Event – Existing

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EX-1: Subcat EX-1 Runoff Area=2.785 ac 0.00% Impervious Runoff Depth=0.08"
Flow Length=357' Tc=45.4 min CN=30 Runoff=0.03 cfs 0.018 af

Subcatchment EX-2: Subcat EX-2 Runoff Area=7.521 ac 0.00% Impervious Runoff Depth=0.11"
Flow Length=668' Tc=36.6 min CN=31 Runoff=0.10 cfs 0.066 af

Subcatchment EX-3: Subcat EX-3 Runoff Area=1.759 ac 0.00% Impervious Runoff Depth=0.35"
Flow Length=384' Tc=30.9 min CN=37 Runoff=0.14 cfs 0.052 af

Subcatchment EX-4: Subcat EX-4 Runoff Area=2.031 ac 0.00% Impervious Runoff Depth=1.78"
Flow Length=207' Tc=20.1 min CN=58 Runoff=2.62 cfs 0.302 af

Subcatchment EX-5: Subcat EX-5 Runoff Area=3.788 ac 0.00% Impervious Runoff Depth=3.03"
Flow Length=433' Tc=6.5 min CN=72 Runoff=12.99 cfs 0.955 af

Subcatchment EX-6: Subcat EX-6 Runoff Area=1.939 ac 0.00% Impervious Runoff Depth=2.56"
Flow Length=212' Tc=5.0 min CN=67 Runoff=5.76 cfs 0.414 af

Subcatchment EX-7: Subcat EX-7 Runoff Area=2.477 ac 0.00% Impervious Runoff Depth=2.20"
Flow Length=153' Tc=5.0 min CN=63 Runoff=6.23 cfs 0.455 af

Subcatchment EX-8: Subcat EX-8 Runoff Area=0.137 ac 0.00% Impervious Runoff Depth=0.75"
Flow Length=41' Slope=0.1960 '/' Tc=7.1 min CN=44 Runoff=0.06 cfs 0.009 af

Pond 1P: EX Depression Peak Elev=25.39' Storage=12,877 cf Inflow=12.99 cfs 0.955 af
Discarded=2.45 cfs 0.955 af Primary=0.00 cfs 0.000 af Outflow=2.45 cfs 0.955 af

Pond 2P: EX Depression Peak Elev=25.46' Storage=5,640 cf Inflow=5.76 cfs 0.414 af
Discarded=1.14 cfs 0.414 af Primary=0.00 cfs 0.000 af Outflow=1.14 cfs 0.414 af

Pond 3P: EX Depression Peak Elev=25.21' Storage=4,903 cf Inflow=6.23 cfs 0.455 af
Discarded=1.47 cfs 0.455 af Primary=0.00 cfs 0.000 af Outflow=1.47 cfs 0.455 af

Link DP-1: Off-Site Inflow=0.03 cfs 0.018 af
Primary=0.03 cfs 0.018 af

Link DP-2: Wetlands Inflow=0.11 cfs 0.075 af
Primary=0.11 cfs 0.075 af

Link DP-3: Existing Bogs Inflow=2.63 cfs 0.353 af
Primary=2.63 cfs 0.353 af

Total Runoff Area = 22.437 ac Runoff Volume = 2.270 af Average Runoff Depth = 1.21"
100.00% Pervious = 22.437 ac 0.00% Impervious = 0.000 ac

Summary for Subcatchment EX-1: Subcat EX-1

Runoff = 0.03 cfs @ 16.09 hrs, Volume= 0.018 af, Depth= 0.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
2.785	30	Woods, Good, HSG A
2.785		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
39.4	50	0.0040	0.02		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
6.0	307	0.0290	0.85		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
45.4	357	Total			

Summary for Subcatchment EX-2: Subcat EX-2

Runoff = 0.10 cfs @ 15.57 hrs, Volume= 0.066 af, Depth= 0.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
0.234	72	Dirt roads, HSG A
7.287	30	Woods, Good, HSG A
7.521	31	Weighted Average
7.521		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
26.3	50	0.0110	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
10.3	618	0.0400	1.00		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
36.6	668	Total			

Summary for Subcatchment EX-3: Subcat EX-3

Runoff = 0.14 cfs @ 12.79 hrs, Volume= 0.052 af, Depth= 0.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
0.306	72	Dirt roads, HSG A
1.453	30	Woods, Good, HSG A
1.759	37	Weighted Average
1.759		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.7	50	0.0200	0.04		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
10.2	334	0.0120	0.55		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
30.9	384	Total			

Summary for Subcatchment EX-4: Subcat EX-4

Runoff = 2.62 cfs @ 12.31 hrs, Volume= 0.302 af, Depth= 1.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
1.339	72	Dirt roads, HSG A
0.692	30	Woods, Good, HSG A
2.031	58	Weighted Average
2.031		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.8	50	0.0340	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
3.3	157	0.0250	0.79		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
20.1	207	Total			

Summary for Subcatchment EX-5: Subcat EX-5

Runoff = 12.99 cfs @ 12.10 hrs, Volume= 0.955 af, Depth= 3.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
3.782	72	Dirt roads, HSG A
0.006	30	Woods, Good, HSG A
3.788	72	Weighted Average
3.788		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	50	0.2040	0.94		**Sheet Flow, Dirt** Fallow n= 0.050 P2= 3.44"
5.6	383	0.0050	1.14		**Shallow Concentrated Flow, Dirt** Unpaved Kv= 16.1 fps
6.5	433	Total			

Summary for Subcatchment EX-6: Subcat EX-6

Runoff = 5.76 cfs @ 12.08 hrs, Volume= 0.414 af, Depth= 2.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
1.698	72	Dirt roads, HSG A
0.241	30	Woods, Good, HSG A
1.939	67	Weighted Average
1.939		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.3	50	0.0200	0.37		**Sheet Flow,** Fallow n= 0.050 P2= 3.44"
2.0	162	0.0070	1.35		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
4.3	212	Total, Increased to minimum Tc = 5.0 min			

Summary for Subcatchment EX-7: Subcat EX-7

Runoff = 6.23 cfs @ 12.08 hrs, Volume= 0.455 af, Depth= 2.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
1.960	72	Dirt roads, HSG A
0.517	30	Woods, Good, HSG A
2.477	63	Weighted Average
2.477		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.7	50	0.3700	1.19		**Sheet Flow,** Fallow n= 0.050 P2= 3.44"
1.1	103	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
1.8	153	Total, Increased to minimum Tc = 5.0 min			

Summary for Subcatchment EX-8: Subcat EX-8

Runoff = 0.06 cfs @ 12.16 hrs, Volume= 0.009 af, Depth= 0.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
0.022	72	Dirt roads, HSG A
0.003	89	Dirt roads, HSG D
0.013	98	Water Surface, 0% imp, HSG A
0.099	30	Woods, Good, HSG A
0.137	44	Weighted Average
0.137		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.1	41	0.1960	0.10		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"

Summary for Pond 1P: EX Depression

Inflow Area = 3.788 ac, 0.00% Impervious, Inflow Depth = 3.03" for 25-Year event
Inflow = 12.99 cfs @ 12.10 hrs, Volume= 0.955 af
Outflow = 2.45 cfs @ 12.58 hrs, Volume= 0.955 af, Atten= 81%, Lag= 28.7 min
Discarded = 2.45 cfs @ 12.58 hrs, Volume= 0.955 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 25.39' @ 12.58 hrs Surf.Area= 40,326 sf Storage= 12,877 cf

Plug-Flow detention time= 43.1 min calculated for 0.955 af (100% of inflow)
Center-of-Mass det. time= 43.1 min (876.8 - 833.7)

Volume	Invert	Avail.Storage	Storage Description
#1	25.00'	111,585 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
25.00	25,532	0	0
26.00	63,362	44,447	44,447
27.00	70,913	67,138	111,585

Device	Routing	Invert	Outlet Devices
#1	Discarded	25.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 21.40'
#2	Primary	26.50'	**30.0' long x 38.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Discarded OutFlow Max=2.44 cfs @ 12.58 hrs HW=25.39' (Free Discharge)
1=Exfiltration (Controls 2.44 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=25.00' (Free Discharge)
2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2P: EX Depression

Inflow Area = 1.939 ac, 0.00% Impervious, Inflow Depth = 2.56" for 25-Year event
Inflow = 5.76 cfs @ 12.08 hrs, Volume= 0.414 af
Outflow = 1.14 cfs @ 12.55 hrs, Volume= 0.414 af, Atten= 80%, Lag= 27.9 min
Discarded = 1.14 cfs @ 12.55 hrs, Volume= 0.414 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 25.46' @ 12.55 hrs Surf.Area= 19,012 sf Storage= 5,640 cf

Plug-Flow detention time= 46.0 min calculated for 0.413 af (100% of inflow)
Center-of-Mass det. time= 45.9 min (890.0 - 844.1)

Volume	Invert	Avail.Storage	Storage Description
#1	25.00'	57,563 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
25.00	5,405	0	0
26.00	34,860	20,133	20,133
27.00	40,000	37,430	57,563

Device	Routing	Invert	Outlet Devices
#1	Discarded	25.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 21.40'
#2	Primary	26.50'	**68.0' long x 53.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Discarded OutFlow Max=1.14 cfs @ 12.55 hrs HW=25.46' (Free Discharge)
1=Exfiltration (Controls 1.14 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=25.00' (Free Discharge)
2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 3P: EX Depression

Inflow Area = 2.477 ac, 0.00% Impervious, Inflow Depth = 2.20" for 25-Year event
Inflow = 6.23 cfs @ 12.08 hrs, Volume= 0.455 af
Outflow = 1.47 cfs @ 12.52 hrs, Volume= 0.455 af, Atten= 76%, Lag= 25.9 min
Discarded = 1.47 cfs @ 12.52 hrs, Volume= 0.455 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 25.21' @ 12.52 hrs Surf.Area= 25,066 sf Storage= 4,903 cf

Plug-Flow detention time= 23.7 min calculated for 0.455 af (100% of inflow)
Center-of-Mass det. time= 23.7 min (877.5 - 853.8)

Volume	Invert	Avail.Storage	Storage Description
#1	25.00'	72,787 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
25.00	20,982	0	0
26.00	40,160	30,571	30,571
27.00	44,271	42,216	72,787

Device	Routing	Invert	Outlet Devices
#1	Discarded	25.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 21.40'
#2	Primary	26.50'	**78.0' long x 16.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Discarded OutFlow Max=1.47 cfs @ 12.52 hrs HW=25.21' (Free Discharge)
↑—**1=Exfiltration** (Controls 1.47 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=25.00' (Free Discharge)
↑—**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link DP-1: Off-Site

Inflow Area = 2.785 ac, 0.00% Impervious, Inflow Depth = 0.08" for 25-Year event
Inflow = 0.03 cfs @ 16.09 hrs, Volume= 0.018 af
Primary = 0.03 cfs @ 16.09 hrs, Volume= 0.018 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: Wetlands

Inflow Area = 7.658 ac, 0.00% Impervious, Inflow Depth = 0.12" for 25-Year event
Inflow = 0.11 cfs @ 15.50 hrs, Volume= 0.075 af
Primary = 0.11 cfs @ 15.50 hrs, Volume= 0.075 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-3: Existing Bogs

Inflow Area = 11.994 ac, 0.00% Impervious, Inflow Depth = 0.35" for 25-Year event
Inflow = 2.63 cfs @ 12.31 hrs, Volume= 0.353 af
Primary = 2.63 cfs @ 12.31 hrs, Volume= 0.353 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

100-Year Storm Event – Existing

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EX-1: Subcat EX-1 Runoff Area=2.785 ac 0.00% Impervious Runoff Depth=0.32"
Flow Length=357' Tc=45.4 min CN=30 Runoff=0.13 cfs 0.075 af

Subcatchment EX-2: Subcat EX-2 Runoff Area=7.521 ac 0.00% Impervious Runoff Depth=0.39"
Flow Length=668' Tc=36.6 min CN=31 Runoff=0.52 cfs 0.242 af

Subcatchment EX-3: Subcat EX-3 Runoff Area=1.759 ac 0.00% Impervious Runoff Depth=0.82"
Flow Length=384' Tc=30.9 min CN=37 Runoff=0.55 cfs 0.120 af

Subcatchment EX-4: Subcat EX-4 Runoff Area=2.031 ac 0.00% Impervious Runoff Depth=2.81"
Flow Length=207' Tc=20.1 min CN=58 Runoff=4.33 cfs 0.476 af

Subcatchment EX-5: Subcat EX-5 Runoff Area=3.788 ac 0.00% Impervious Runoff Depth=4.33"
Flow Length=433' Tc=6.5 min CN=72 Runoff=18.60 cfs 1.366 af

Subcatchment EX-6: Subcat EX-6 Runoff Area=1.939 ac 0.00% Impervious Runoff Depth=3.78"
Flow Length=212' Tc=5.0 min CN=67 Runoff=8.58 cfs 0.610 af

Subcatchment EX-7: Subcat EX-7 Runoff Area=2.477 ac 0.00% Impervious Runoff Depth=3.34"
Flow Length=153' Tc=5.0 min CN=63 Runoff=9.63 cfs 0.690 af

Subcatchment EX-8: Subcat EX-8 Runoff Area=0.137 ac 0.00% Impervious Runoff Depth=1.43"
Flow Length=41' Slope=0.1960 '/' Tc=7.1 min CN=44 Runoff=0.17 cfs 0.016 af

Pond 1P: EX Depression Peak Elev=25.57' Storage=20,490 cf Inflow=18.60 cfs 1.366 af
Discarded=2.93 cfs 1.366 af Primary=0.00 cfs 0.000 af Outflow=2.93 cfs 1.366 af

Pond 2P: EX Depression Peak Elev=25.63' Storage=9,205 cf Inflow=8.58 cfs 0.610 af
Discarded=1.47 cfs 0.610 af Primary=0.00 cfs 0.000 af Outflow=1.47 cfs 0.610 af

Pond 3P: EX Depression Peak Elev=25.37' Storage=9,144 cf Inflow=9.63 cfs 0.690 af
Discarded=1.71 cfs 0.690 af Primary=0.00 cfs 0.000 af Outflow=1.71 cfs 0.690 af

Link DP-1: Off-Site Inflow=0.13 cfs 0.075 af
Primary=0.13 cfs 0.075 af

Link DP-2: Wetlands Inflow=0.55 cfs 0.258 af
Primary=0.55 cfs 0.258 af

Link DP-3: Existing Bogs Inflow=4.54 cfs 0.596 af
Primary=4.54 cfs 0.596 af

Total Runoff Area = 22.437 ac Runoff Volume = 3.595 af Average Runoff Depth = 1.92"
100.00% Pervious = 22.437 ac 0.00% Impervious = 0.000 ac

Summary for Subcatchment EX-1: Subcat EX-1

Runoff = 0.13 cfs @ 13.76 hrs, Volume= 0.075 af, Depth= 0.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
2.785	30	Woods, Good, HSG A
2.785		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
39.4	50	0.0040	0.02		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
6.0	307	0.0290	0.85		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
45.4	357	Total			

Summary for Subcatchment EX-2: Subcat EX-2

Runoff = 0.52 cfs @ 12.98 hrs, Volume= 0.242 af, Depth= 0.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
0.234	72	Dirt roads, HSG A
7.287	30	Woods, Good, HSG A
7.521	31	Weighted Average
7.521		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
26.3	50	0.0110	0.03		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
10.3	618	0.0400	1.00		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
36.6	668	Total			

Summary for Subcatchment EX-3: Subcat EX-3

Runoff = 0.55 cfs @ 12.62 hrs, Volume= 0.120 af, Depth= 0.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
0.306	72	Dirt roads, HSG A
1.453	30	Woods, Good, HSG A
1.759	37	Weighted Average
1.759		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.7	50	0.0200	0.04		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
10.2	334	0.0120	0.55		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
30.9	384	Total			

Summary for Subcatchment EX-4: Subcat EX-4

Runoff = 4.33 cfs @ 12.30 hrs, Volume= 0.476 af, Depth= 2.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
1.339	72	Dirt roads, HSG A
0.692	30	Woods, Good, HSG A
2.031	58	Weighted Average
2.031		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.8	50	0.0340	0.05		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"
3.3	157	0.0250	0.79		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
20.1	207	Total			

Summary for Subcatchment EX-5: Subcat EX-5

Runoff = 18.60 cfs @ 12.10 hrs, Volume= 1.366 af, Depth= 4.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
3.782	72	Dirt roads, HSG A
0.006	30	Woods, Good, HSG A
3.788	72	Weighted Average
3.788		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	50	0.2040	0.94		**Sheet Flow, Dirt** Fallow n= 0.050 P2= 3.44"
5.6	383	0.0050	1.14		**Shallow Concentrated Flow, Dirt** Unpaved Kv= 16.1 fps
6.5	433	Total			

Summary for Subcatchment EX-6: Subcat EX-6

Runoff = 8.58 cfs @ 12.08 hrs, Volume= 0.610 af, Depth= 3.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
1.698	72	Dirt roads, HSG A
0.241	30	Woods, Good, HSG A
1.939	67	Weighted Average
1.939		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.3	50	0.0200	0.37		**Sheet Flow,** Fallow n= 0.050 P2= 3.44"
2.0	162	0.0070	1.35		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
4.3	212	Total, Increased to minimum Tc = 5.0 min			

Summary for Subcatchment EX-7: Subcat EX-7

Runoff = 9.63 cfs @ 12.08 hrs, Volume= 0.690 af, Depth= 3.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
1.960	72	Dirt roads, HSG A
0.517	30	Woods, Good, HSG A
2.477	63	Weighted Average
2.477		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.7	50	0.3700	1.19		**Sheet Flow,** Fallow n= 0.050 P2= 3.44"
1.1	103	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
1.8	153	Total, Increased to minimum Tc = 5.0 min			

Summary for Subcatchment EX-8: Subcat EX-8

Runoff = 0.17 cfs @ 12.13 hrs, Volume= 0.016 af, Depth= 1.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
0.022	72	Dirt roads, HSG A
0.003	89	Dirt roads, HSG D
0.013	98	Water Surface, 0% imp, HSG A
0.099	30	Woods, Good, HSG A
0.137	44	Weighted Average
0.137		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.1	41	0.1960	0.10		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 3.44"

Summary for Pond 1P: EX Depression

Inflow Area = 3.788 ac, 0.00% Impervious, Inflow Depth = 4.33" for 100-Year event
Inflow = 18.60 cfs @ 12.10 hrs, Volume= 1.366 af
Outflow = 2.93 cfs @ 12.62 hrs, Volume= 1.366 af, Atten= 84%, Lag= 31.4 min
Discarded = 2.93 cfs @ 12.62 hrs, Volume= 1.366 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 25.57' @ 12.62 hrs Surf.Area= 46,927 sf Storage= 20,490 cf

Plug-Flow detention time= 62.2 min calculated for 1.365 af (100% of inflow)
Center-of-Mass det. time= 62.1 min (885.5 - 823.4)

Volume	Invert	Avail.Storage	Storage Description
#1	25.00'	111,585 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
25.00	25,532	0	0
26.00	63,362	44,447	44,447
27.00	70,913	67,138	111,585

Device	Routing	Invert	Outlet Devices
#1	Discarded	25.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 21.40'
#2	Primary	26.50'	**30.0' long x 38.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Discarded OutFlow Max=2.93 cfs @ 12.62 hrs HW=25.57' (Free Discharge)
↑—**1=Exfiltration** (Controls 2.93 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=25.00' (Free Discharge)
↑—**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Pond 2P: EX Depression

Inflow Area = 1.939 ac, 0.00% Impervious, Inflow Depth = 3.78" for 100-Year event
Inflow = 8.58 cfs @ 12.08 hrs, Volume= 0.610 af
Outflow = 1.47 cfs @ 12.57 hrs, Volume= 0.610 af, Atten= 83%, Lag= 29.4 min
Discarded = 1.47 cfs @ 12.57 hrs, Volume= 0.610 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 25.63' @ 12.57 hrs Surf.Area= 23,906 sf Storage= 9,205 cf

Plug-Flow detention time= 62.0 min calculated for 0.610 af (100% of inflow)
Center-of-Mass det. time= 62.0 min (894.7 - 832.7)

Volume	Invert	Avail.Storage	Storage Description
#1	25.00'	57,563 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
25.00	5,405	0	0
26.00	34,860	20,133	20,133
27.00	40,000	37,430	57,563

Device	Routing	Invert	Outlet Devices
#1	Discarded	25.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 21.40'
#2	Primary	26.50'	**68.0' long x 53.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Discarded OutFlow Max=1.47 cfs @ 12.57 hrs HW=25.63' (Free Discharge)
↑—**1=Exfiltration** (Controls 1.47 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=25.00' (Free Discharge)
↑—**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Pond 3P: EX Depression

Inflow Area = 2.477 ac, 0.00% Impervious, Inflow Depth = 3.34" for 100-Year event
Inflow = 9.63 cfs @ 12.08 hrs, Volume= 0.690 af
Outflow = 1.71 cfs @ 12.57 hrs, Volume= 0.690 af, Atten= 82%, Lag= 29.3 min
Discarded = 1.71 cfs @ 12.57 hrs, Volume= 0.690 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 25.37' @ 12.57 hrs Surf.Area= 28,124 sf Storage= 9,144 cf

Plug-Flow detention time= 43.0 min calculated for 0.690 af (100% of inflow)
Center-of-Mass det. time= 42.9 min (884.3 - 841.4)

Volume	Invert	Avail.Storage	Storage Description
#1	25.00'	72,787 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
25.00	20,982	0	0
26.00	40,160	30,571	30,571
27.00	44,271	42,216	72,787

Device	Routing	Invert	Outlet Devices
#1	Discarded	25.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 21.40'
#2	Primary	26.50'	**78.0' long x 16.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Discarded OutFlow Max=1.71 cfs @ 12.57 hrs HW=25.37' (Free Discharge)
↑—**1=Exfiltration** (Controls 1.71 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=25.00' (Free Discharge)
↑—**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link DP-1: Off-Site

Inflow Area = 2.785 ac, 0.00% Impervious, Inflow Depth = 0.32" for 100-Year event
Inflow = 0.13 cfs @ 13.76 hrs, Volume= 0.075 af
Primary = 0.13 cfs @ 13.76 hrs, Volume= 0.075 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: Wetlands

Inflow Area = 7.658 ac, 0.00% Impervious, Inflow Depth = 0.40" for 100-Year event
Inflow = 0.55 cfs @ 12.96 hrs, Volume= 0.258 af
Primary = 0.55 cfs @ 12.96 hrs, Volume= 0.258 af, Atten= 0%, Lag= 0.0 min

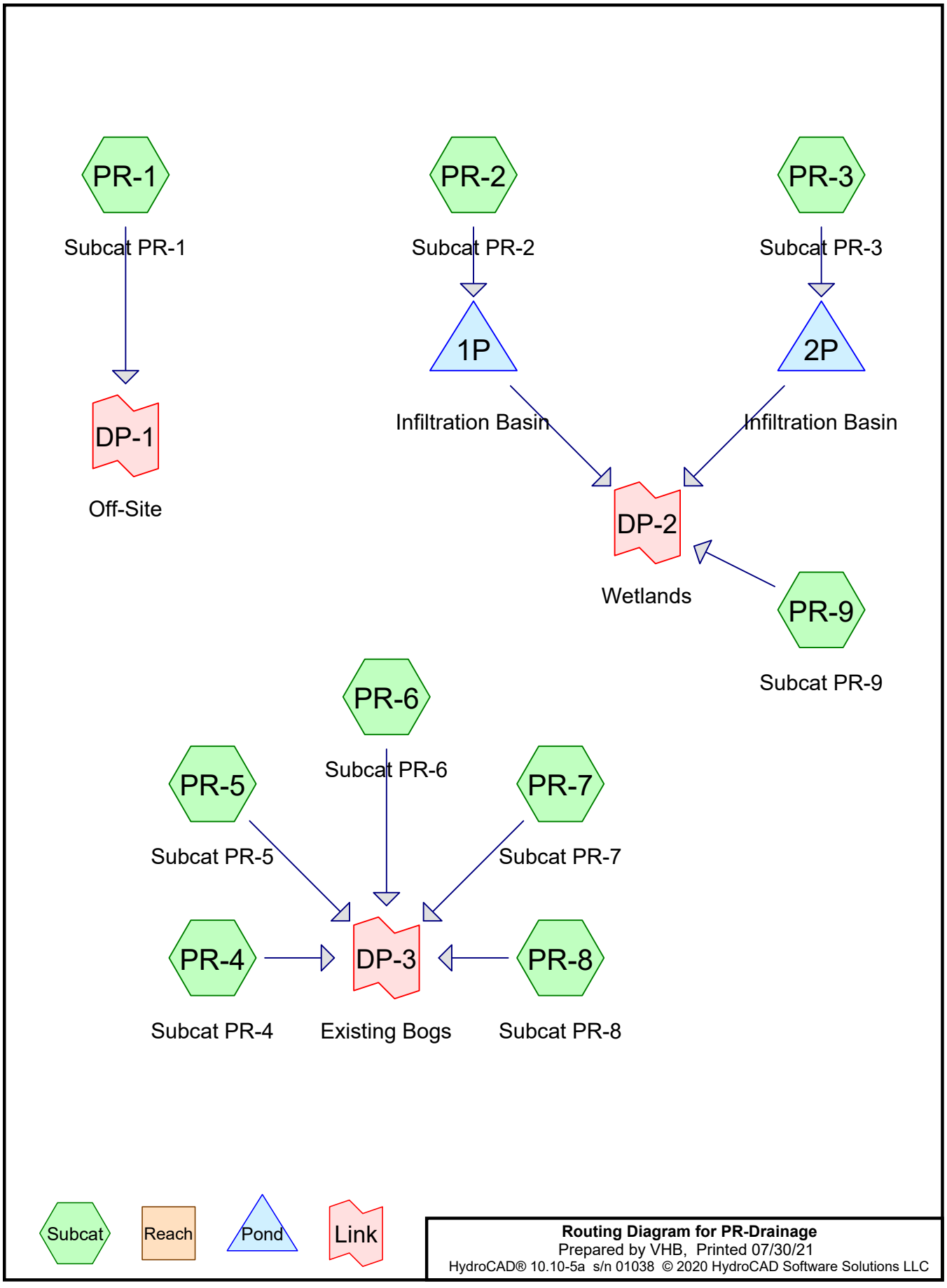
Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-3: Existing Bogs

Inflow Area = 11.994 ac, 0.00% Impervious, Inflow Depth = 0.60" for 100-Year event
Inflow = 4.54 cfs @ 12.31 hrs, Volume= 0.596 af
Primary = 4.54 cfs @ 12.31 hrs, Volume= 0.596 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

HydroCAD Analysis: Proposed Conditions



Routing Diagram for PR-Drainage
Prepared by VHB, Printed 07/30/21

Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-Year	Type III 24-hr		Default	24.00	1	3.44	2
2	10-Year	Type III 24-hr		Default	24.00	1	5.04	2
3	25-Year	Type III 24-hr		Default	24.00	1	6.04	2
4	100-Year	Type III 24-hr		Default	24.00	1	7.58	2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.008	98	Concrete pads, HSG A (PR-2)
0.057	72	Dirt roads, HSG A (PR-7, PR-9)
0.002	89	Dirt roads, HSG D (PR-9)
1.776	76	Gravel roads, HSG A (PR-2, PR-3, PR-5, PR-6, PR-7, PR-8)
19.218	30	Meadow, non-grazed, HSG A (PR-1, PR-2, PR-3, PR-4, PR-5, PR-6, PR-7, PR-8)
0.014	98	Water Surface, 0% imp, HSG A (PR-9)
1.362	30	Woods, Good, HSG A (PR-2, PR-6, PR-7, PR-8, PR-9)
22.437	**34**	**TOTAL AREA**

2-Year Storm Event – Proposed

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PR-1: Subcat PR-1 Runoff Area=2.465 ac 0.00% Impervious Runoff Depth=0.00"
Flow Length=357' Tc=14.4 min CN=30 Runoff=0.00 cfs 0.000 af

Subcatchment PR-2: Subcat PR-2 Runoff Area=7.176 ac 0.11% Impervious Runoff Depth=0.00"
Flow Length=610' Tc=19.0 min CN=33 Runoff=0.00 cfs 0.000 af

Subcatchment PR-3: Subcat PR-3 Runoff Area=1.491 ac 0.00% Impervious Runoff Depth=0.00"
Flow Length=450' Tc=17.3 min CN=38 Runoff=0.00 cfs 0.000 af

Subcatchment PR-4: Subcat PR-4 Runoff Area=0.928 ac 0.00% Impervious Runoff Depth=0.00"
Flow Length=202' Tc=10.6 min CN=30 Runoff=0.00 cfs 0.000 af

Subcatchment PR-5: Subcat PR-5 Runoff Area=1.369 ac 0.00% Impervious Runoff Depth=0.00"
Flow Length=222' Tc=12.8 min CN=35 Runoff=0.00 cfs 0.000 af

Subcatchment PR-6: Subcat PR-6 Runoff Area=4.441 ac 0.00% Impervious Runoff Depth=0.00"
Flow Length=433' Tc=22.0 min CN=36 Runoff=0.00 cfs 0.000 af

Subcatchment PR-7: Subcat PR-7 Runoff Area=1.939 ac 0.00% Impervious Runoff Depth=0.00"
Flow Length=199' Tc=17.1 min CN=34 Runoff=0.00 cfs 0.000 af

Subcatchment PR-8: Subcat PR-8 Runoff Area=2.485 ac 0.00% Impervious Runoff Depth=0.00"
Flow Length=153' Tc=8.4 min CN=33 Runoff=0.00 cfs 0.000 af

Subcatchment PR-9: Subcat PR-9 Runoff Area=0.143 ac 0.00% Impervious Runoff Depth=0.09"
Flow Length=230' Tc=11.5 min CN=46 Runoff=0.00 cfs 0.001 af

Pond 1P: Infiltration Basin Peak Elev=21.50' Storage=0 cf Inflow=0.00 cfs 0.000 af
Discarded=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af

Pond 2P: Infiltration Basin Peak Elev=21.50' Storage=0 cf Inflow=0.00 cfs 0.000 af
Discarded=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af

Link DP-1: Off-Site Inflow=0.00 cfs 0.000 af
Primary=0.00 cfs 0.000 af

Link DP-2: Wetlands Inflow=0.00 cfs 0.001 af
Primary=0.00 cfs 0.001 af

Link DP-3: Existing Bogs Inflow=0.00 cfs 0.000 af
Primary=0.00 cfs 0.000 af

Total Runoff Area = 22.437 ac Runoff Volume = 0.001 af Average Runoff Depth = 0.00"
99.96% Pervious = 22.429 ac 0.04% Impervious = 0.008 ac

Summary for Subcatchment PR-1: Subcat PR-1

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
2.465	30	Meadow, non-grazed, HSG A
2.465		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.5	50	0.0400	0.11		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
6.6	293	0.0400	0.74		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
0.3	14	0.0430	0.77		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
14.4	357	Total			

Summary for Subcatchment PR-2: Subcat PR-2

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

	Area (ac)	CN	Description
	0.463	76	Gravel roads, HSG A
	6.249	30	Meadow, non-grazed, HSG A
*	0.008	98	Concrete pads, HSG A
	0.456	30	Woods, Good, HSG A
	7.176	33	Weighted Average
	7.168		99.89% Pervious Area
	0.008		0.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.9	50	0.1200	0.17		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
14.1	560	0.0320	0.66		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
19.0	610	Total			

Summary for Subcatchment PR-3: Subcat PR-3

Runoff = 0.00 cfs @ 23.99 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
0.270	76	Gravel roads, HSG A
1.221	30	Meadow, non-grazed, HSG A
1.491	38	Weighted Average
1.491		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.6	50	0.0300	0.18		**Sheet Flow,** Grass: Short n= 0.150 P2= 3.44"
12.7	400	0.0200	0.52		**Shallow Concentrated Flow,** Kv= 3.7 fps
17.3	450	Total			

Summary for Subcatchment PR-4: Subcat PR-4

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
0.928	30	Meadow, non-grazed, HSG A
0.928		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.5	50	0.0400	0.11		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
3.1	152	0.0492	0.82		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
10.6	202	Total			

Summary for Subcatchment PR-5: Subcat PR-5

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
0.141	76	Gravel roads, HSG A
1.228	30	Meadow, non-grazed, HSG A
1.369	35	Weighted Average
1.369		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.5	50	0.0300	0.10		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
4.3	172	0.0319	0.66		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
12.8	222	Total			

Summary for Subcatchment PR-6: Subcat PR-6

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
0.559	76	Gravel roads, HSG A
3.877	30	Meadow, non-grazed, HSG A
0.005	30	Woods, Good, HSG A
4.441	36	Weighted Average
4.441		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	50	0.1840	0.90		**Sheet Flow, Dirt** Fallow n= 0.050 P2= 3.44"
0.1	22	0.0450	3.42		**Shallow Concentrated Flow, Dirt** Unpaved Kv= 16.1 fps
21.0	361	0.0060	0.29		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
22.0	433	Total			

Summary for Subcatchment PR-7: Subcat PR-7

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
0.028	72	Dirt roads, HSG A
0.157	76	Gravel roads, HSG A
1.505	30	Meadow, non-grazed, HSG A
0.249	30	Woods, Good, HSG A
1.939	34	Weighted Average
1.939		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.5	50	0.0140	0.07		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
5.2	122	0.0110	0.39		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
0.4	27	0.0040	1.02		**Shallow Concentrated Flow, Gravel** Unpaved Kv= 16.1 fps
17.1	199	Total			

Summary for Subcatchment PR-8: Subcat PR-8

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
0.186	76	Gravel roads, HSG A
1.745	30	Meadow, non-grazed, HSG A
0.554	30	Woods, Good, HSG A
2.485	33	Weighted Average
2.485		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	38	0.4870	1.26		**Sheet Flow, Dirt** Fallow n= 0.050 P2= 3.44"
3.5	12	0.0160	0.06		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
4.4	103	0.0110	0.39		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
8.4	153	Total			

Summary for Subcatchment PR-9: Subcat PR-9

Runoff = 0.00 cfs @ 14.69 hrs, Volume= 0.001 af, Depth= 0.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.44"

Area (ac)	CN	Description
0.029	72	Dirt roads, HSG A
0.002	89	Dirt roads, HSG D
0.014	98	Water Surface, 0% imp, HSG A
0.098	30	Woods, Good, HSG A
0.143	46	Weighted Average
0.143		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.9	50	0.0500	0.12		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
4.6	180	0.0306	0.65		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
11.5	230	Total			

Summary for Pond 1P: Infiltration Basin

Inflow Area = 7.176 ac, 0.11% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min
Discarded = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 21.50' @ 0.00 hrs Surf.Area= 5,083 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert	Avail.Storage	Storage Description
#1	21.50'	7,968 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
21.50	5,083	0	0
22.00	5,570	2,663	2,663
22.90	6,218	5,305	7,968

Device	Routing	Invert	Outlet Devices
#1	Discarded	21.50'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 17.30'
#2	Primary	20.60'	**10.0" Round Culvert** L= 41.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 20.60' / 20.40' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.55 sf
#3	Device 2	22.30'	**48.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#4	Primary	22.80'	**10.0' long Sharp-Crested Rectangular Weir** 2 End Contraction(s) 1.3' Crest Height

Discarded OutFlow Max=0.00 cfs @ 0.00 hrs HW=21.50' (Free Discharge)
└**1=Exfiltration** (Passes 0.00 cfs of 0.28 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=21.50' (Free Discharge)
└**2=Culvert** (Passes 0.00 cfs of 1.49 cfs potential flow)
└**3=Orifice/Grate** (Controls 0.00 cfs)
└**4=Sharp-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Pond 2P: Infiltration Basin

Inflow Area = 1.491 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.00 cfs @ 23.99 hrs, Volume= 0.000 af
Outflow = 0.00 cfs @ 24.03 hrs, Volume= 0.000 af, Atten= 0%, Lag= 2.1 min
Discarded = 0.00 cfs @ 24.03 hrs, Volume= 0.000 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 21.50' @ 24.03 hrs Surf.Area= 972 sf Storage= 0 cf

Plug-Flow detention time= 4.4 min calculated for 0.000 af (100% of inflow)
Center-of-Mass det. time= 4.4 min (1,356.3 - 1,351.8)

Volume	Invert	Avail.Storage	Storage Description
#1	21.50'	2,016 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
21.50	972	0	0
22.00	1,295	567	567
23.00	1,604	1,450	2,016

Device	Routing	Invert	Outlet Devices
#1	Discarded	21.50'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 17.30'
#2	Primary	21.50'	**6.0" Round Culvert** L= 50.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 21.50' / 21.00' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	22.40'	**48.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#4	Primary	22.90'	**10.0' long Sharp-Crested Rectangular Weir** 2 End Contraction(s) 1.3' Crest Height

Discarded OutFlow Max=0.05 cfs @ 24.03 hrs HW=21.50' (Free Discharge)
└**1=Exfiltration** (Controls 0.05 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=21.50' (Free Discharge)
└**2=Culvert** (Controls 0.00 cfs)
└**3=Orifice/Grate** (Controls 0.00 cfs)
└**4=Sharp-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link DP-1: Off-Site

Inflow Area = 2.465 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: Wetlands

Inflow Area = 8.810 ac, 0.09% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.00 cfs @ 14.69 hrs, Volume= 0.001 af
Primary = 0.00 cfs @ 14.69 hrs, Volume= 0.001 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-3: Existing Bogs

Inflow Area = 11.162 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

10-Year Storm Event – Proposed

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PR-1: Subcat PR-1 Runoff Area=2.465 ac 0.00% Impervious Runoff Depth=0.01"
Flow Length=357' Tc=14.4 min CN=30 Runoff=0.00 cfs 0.001 af

Subcatchment PR-2: Subcat PR-2 Runoff Area=7.176 ac 0.11% Impervious Runoff Depth=0.05"
Flow Length=610' Tc=19.0 min CN=33 Runoff=0.04 cfs 0.027 af

Subcatchment PR-3: Subcat PR-3 Runoff Area=1.491 ac 0.00% Impervious Runoff Depth=0.17"
Flow Length=450' Tc=17.3 min CN=38 Runoff=0.04 cfs 0.022 af

Subcatchment PR-4: Subcat PR-4 Runoff Area=0.928 ac 0.00% Impervious Runoff Depth=0.01"
Flow Length=202' Tc=10.6 min CN=30 Runoff=0.00 cfs 0.000 af

Subcatchment PR-5: Subcat PR-5 Runoff Area=1.369 ac 0.00% Impervious Runoff Depth=0.09"
Flow Length=222' Tc=12.8 min CN=35 Runoff=0.02 cfs 0.010 af

Subcatchment PR-6: Subcat PR-6 Runoff Area=4.441 ac 0.00% Impervious Runoff Depth=0.11"
Flow Length=433' Tc=22.0 min CN=36 Runoff=0.07 cfs 0.042 af

Subcatchment PR-7: Subcat PR-7 Runoff Area=1.939 ac 0.00% Impervious Runoff Depth=0.07"
Flow Length=199' Tc=17.1 min CN=34 Runoff=0.02 cfs 0.011 af

Subcatchment PR-8: Subcat PR-8 Runoff Area=2.485 ac 0.00% Impervious Runoff Depth=0.05"
Flow Length=153' Tc=8.4 min CN=33 Runoff=0.01 cfs 0.009 af

Subcatchment PR-9: Subcat PR-9 Runoff Area=0.143 ac 0.00% Impervious Runoff Depth=0.50"
Flow Length=230' Tc=11.5 min CN=46 Runoff=0.03 cfs 0.006 af

Pond 1P: Infiltration Basin Peak Elev=21.50' Storage=10 cf Inflow=0.04 cfs 0.027 af
Discarded=0.04 cfs 0.027 af Primary=0.00 cfs 0.000 af Outflow=0.04 cfs 0.027 af

Pond 2P: Infiltration Basin Peak Elev=21.51' Storage=10 cf Inflow=0.04 cfs 0.022 af
Discarded=0.04 cfs 0.022 af Primary=0.00 cfs 0.000 af Outflow=0.04 cfs 0.022 af

Link DP-1: Off-Site Inflow=0.00 cfs 0.001 af
Primary=0.00 cfs 0.001 af

Link DP-2: Wetlands Inflow=0.03 cfs 0.006 af
Primary=0.03 cfs 0.006 af

Link DP-3: Existing Bogs Inflow=0.11 cfs 0.073 af
Primary=0.11 cfs 0.073 af

Total Runoff Area = 22.437 ac Runoff Volume = 0.129 af Average Runoff Depth = 0.07"
99.96% Pervious = 22.429 ac 0.04% Impervious = 0.008 ac

Summary for Subcatchment PR-1: Subcat PR-1

Runoff = 0.00 cfs @ 23.77 hrs, Volume= 0.001 af, Depth= 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
2.465	30	Meadow, non-grazed, HSG A
2.465		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.5	50	0.0400	0.11		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
6.6	293	0.0400	0.74		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
0.3	14	0.0430	0.77		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
14.4	357	Total			

Summary for Subcatchment PR-2: Subcat PR-2

Runoff = 0.04 cfs @ 17.01 hrs, Volume= 0.027 af, Depth= 0.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

	Area (ac)	CN	Description
	0.463	76	Gravel roads, HSG A
	6.249	30	Meadow, non-grazed, HSG A
*	0.008	98	Concrete pads, HSG A
	0.456	30	Woods, Good, HSG A
	7.176	33	Weighted Average
	7.168		99.89% Pervious Area
	0.008		0.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.9	50	0.1200	0.17		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
14.1	560	0.0320	0.66		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
19.0	610	Total			

Summary for Subcatchment PR-3: Subcat PR-3

Runoff = 0.04 cfs @ 13.82 hrs, Volume= 0.022 af, Depth= 0.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
0.270	76	Gravel roads, HSG A
1.221	30	Meadow, non-grazed, HSG A
1.491	38	Weighted Average
1.491		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.6	50	0.0300	0.18		**Sheet Flow,** Grass: Short n= 0.150 P2= 3.44"
12.7	400	0.0200	0.52		**Shallow Concentrated Flow,** Kv= 3.7 fps
17.3	450	Total			

Summary for Subcatchment PR-4: Subcat PR-4

Runoff = 0.00 cfs @ 23.74 hrs, Volume= 0.000 af, Depth= 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
0.928	30	Meadow, non-grazed, HSG A
0.928		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.5	50	0.0400	0.11		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
3.1	152	0.0492	0.82		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
10.6	202	Total			

Summary for Subcatchment PR-5: Subcat PR-5

Runoff = 0.02 cfs @ 15.20 hrs, Volume= 0.010 af, Depth= 0.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
0.141	76	Gravel roads, HSG A
1.228	30	Meadow, non-grazed, HSG A
1.369	35	Weighted Average
1.369		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.5	50	0.0300	0.10		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
4.3	172	0.0319	0.66		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
12.8	222	Total			

Summary for Subcatchment PR-6: Subcat PR-6

Runoff = 0.07 cfs @ 15.05 hrs, Volume= 0.042 af, Depth= 0.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
0.559	76	Gravel roads, HSG A
3.877	30	Meadow, non-grazed, HSG A
0.005	30	Woods, Good, HSG A
4.441	36	Weighted Average
4.441		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	50	0.1840	0.90		**Sheet Flow, Dirt** Fallow n= 0.050 P2= 3.44"
0.1	22	0.0450	3.42		**Shallow Concentrated Flow, Dirt** Unpaved Kv= 16.1 fps
21.0	361	0.0060	0.29		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
22.0	433	Total			

Summary for Subcatchment PR-7: Subcat PR-7

Runoff = 0.02 cfs @ 15.59 hrs, Volume= 0.011 af, Depth= 0.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
0.028	72	Dirt roads, HSG A
0.157	76	Gravel roads, HSG A
1.505	30	Meadow, non-grazed, HSG A
0.249	30	Woods, Good, HSG A
1.939	34	Weighted Average
1.939		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.5	50	0.0140	0.07		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
5.2	122	0.0110	0.39		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
0.4	27	0.0040	1.02		**Shallow Concentrated Flow, Gravel** Unpaved Kv= 16.1 fps
17.1	199	Total			

Summary for Subcatchment PR-8: Subcat PR-8

Runoff = 0.01 cfs @ 16.84 hrs, Volume= 0.009 af, Depth= 0.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
0.186	76	Gravel roads, HSG A
1.745	30	Meadow, non-grazed, HSG A
0.554	30	Woods, Good, HSG A
2.485	33	Weighted Average
2.485		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	38	0.4870	1.26		**Sheet Flow, Dirt** Fallow n= 0.050 P2= 3.44"
3.5	12	0.0160	0.06		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
4.4	103	0.0110	0.39		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
8.4	153	Total			

Summary for Subcatchment PR-9: Subcat PR-9

Runoff = 0.03 cfs @ 12.36 hrs, Volume= 0.006 af, Depth= 0.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=5.04"

Area (ac)	CN	Description
0.029	72	Dirt roads, HSG A
0.002	89	Dirt roads, HSG D
0.014	98	Water Surface, 0% imp, HSG A
0.098	30	Woods, Good, HSG A
0.143	46	Weighted Average
0.143		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.9	50	0.0500	0.12		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
4.6	180	0.0306	0.65		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
11.5	230	Total			

Summary for Pond 1P: Infiltration Basin

Inflow Area = 7.176 ac, 0.11% Impervious, Inflow Depth = 0.05" for 10-Year event
Inflow = 0.04 cfs @ 17.01 hrs, Volume= 0.027 af
Outflow = 0.04 cfs @ 17.08 hrs, Volume= 0.027 af, Atten= 0%, Lag= 3.9 min
Discarded = 0.04 cfs @ 17.08 hrs, Volume= 0.027 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 21.50' @ 17.08 hrs Surf.Area= 5,085 sf Storage= 10 cf

Plug-Flow detention time= 4.2 min calculated for 0.027 af (100% of inflow)
Center-of-Mass det. time= 4.2 min (1,162.3 - 1,158.1)

Volume	Invert	Avail.Storage	Storage Description
#1	21.50'	7,968 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
21.50	5,083	0	0
22.00	5,570	2,663	2,663
22.90	6,218	5,305	7,968

Device	Routing	Invert	Outlet Devices
#1	Discarded	21.50'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 17.30'
#2	Primary	20.60'	**10.0" Round Culvert** L= 41.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 20.60' / 20.40' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.55 sf
#3	Device 2	22.30'	**48.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#4	Primary	22.80'	**10.0' long Sharp-Crested Rectangular Weir** 2 End Contraction(s) 1.3' Crest Height

Discarded OutFlow Max=0.28 cfs @ 17.08 hrs HW=21.50' (Free Discharge)
└**1=Exfiltration** (Controls 0.28 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=21.50' (Free Discharge)
└**2=Culvert** (Passes 0.00 cfs of 1.49 cfs potential flow)
└**3=Orifice/Grate** (Controls 0.00 cfs)
└**4=Sharp-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Pond 2P: Infiltration Basin

Inflow Area = 1.491 ac, 0.00% Impervious, Inflow Depth = 0.17" for 10-Year event
Inflow = 0.04 cfs @ 13.82 hrs, Volume= 0.022 af
Outflow = 0.04 cfs @ 13.89 hrs, Volume= 0.022 af, Atten= 0%, Lag= 4.6 min
Discarded = 0.04 cfs @ 13.89 hrs, Volume= 0.022 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 21.51' @ 13.89 hrs Surf.Area= 978 sf Storage= 10 cf

Plug-Flow detention time= 4.4 min calculated for 0.022 af (100% of inflow)
Center-of-Mass det. time= 4.4 min (1,037.9 - 1,033.5)

Volume	Invert	Avail.Storage	Storage Description
#1	21.50'	2,016 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
21.50	972	0	0
22.00	1,295	567	567
23.00	1,604	1,450	2,016

Device	Routing	Invert	Outlet Devices
#1	Discarded	21.50'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 17.30'
#2	Primary	21.50'	**6.0" Round Culvert** L= 50.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 21.50' / 21.00' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	22.40'	**48.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#4	Primary	22.90'	**10.0' long Sharp-Crested Rectangular Weir** 2 End Contraction(s) 1.3' Crest Height

Discarded OutFlow Max=0.05 cfs @ 13.89 hrs HW=21.51' (Free Discharge)
└**1=Exfiltration** (Controls 0.05 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=21.50' (Free Discharge)
└**2=Culvert** (Controls 0.00 cfs)
└**3=Orifice/Grate** (Controls 0.00 cfs)
└**4=Sharp-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link DP-1: Off-Site

Inflow Area = 2.465 ac, 0.00% Impervious, Inflow Depth = 0.01" for 10-Year event
Inflow = 0.00 cfs @ 23.77 hrs, Volume= 0.001 af
Primary = 0.00 cfs @ 23.77 hrs, Volume= 0.001 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: Wetlands

Inflow Area = 8.810 ac, 0.09% Impervious, Inflow Depth = 0.01" for 10-Year event
Inflow = 0.03 cfs @ 12.36 hrs, Volume= 0.006 af
Primary = 0.03 cfs @ 12.36 hrs, Volume= 0.006 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-3: Existing Bogs

Inflow Area = 11.162 ac, 0.00% Impervious, Inflow Depth = 0.08" for 10-Year event
Inflow = 0.11 cfs @ 15.33 hrs, Volume= 0.073 af
Primary = 0.11 cfs @ 15.33 hrs, Volume= 0.073 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

25-Year Storm Event – Proposed

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PR-1: Subcat PR-1 Runoff Area=2.465 ac 0.00% Impervious Runoff Depth=0.08"
Flow Length=357' Tc=14.4 min CN=30 Runoff=0.02 cfs 0.016 af

Subcatchment PR-2: Subcat PR-2 Runoff Area=7.176 ac 0.11% Impervious Runoff Depth=0.18"
Flow Length=610' Tc=19.0 min CN=33 Runoff=0.17 cfs 0.105 af

Subcatchment PR-3: Subcat PR-3 Runoff Area=1.491 ac 0.00% Impervious Runoff Depth=0.40"
Flow Length=450' Tc=17.3 min CN=38 Runoff=0.19 cfs 0.050 af

Subcatchment PR-4: Subcat PR-4 Runoff Area=0.928 ac 0.00% Impervious Runoff Depth=0.08"
Flow Length=202' Tc=10.6 min CN=30 Runoff=0.01 cfs 0.006 af

Subcatchment PR-5: Subcat PR-5 Runoff Area=1.369 ac 0.00% Impervious Runoff Depth=0.26"
Flow Length=222' Tc=12.8 min CN=35 Runoff=0.07 cfs 0.030 af

Subcatchment PR-6: Subcat PR-6 Runoff Area=4.441 ac 0.00% Impervious Runoff Depth=0.30"
Flow Length=433' Tc=22.0 min CN=36 Runoff=0.28 cfs 0.113 af

Subcatchment PR-7: Subcat PR-7 Runoff Area=1.939 ac 0.00% Impervious Runoff Depth=0.22"
Flow Length=199' Tc=17.1 min CN=34 Runoff=0.06 cfs 0.035 af

Subcatchment PR-8: Subcat PR-8 Runoff Area=2.485 ac 0.00% Impervious Runoff Depth=0.18"
Flow Length=153' Tc=8.4 min CN=33 Runoff=0.06 cfs 0.036 af

Subcatchment PR-9: Subcat PR-9 Runoff Area=0.143 ac 0.00% Impervious Runoff Depth=0.88"
Flow Length=230' Tc=11.5 min CN=46 Runoff=0.08 cfs 0.011 af

Pond 1P: Infiltration Basin Peak Elev=21.51' Storage=42 cf Inflow=0.17 cfs 0.105 af
Discarded=0.17 cfs 0.105 af Primary=0.00 cfs 0.000 af Outflow=0.17 cfs 0.105 af

Pond 2P: Infiltration Basin Peak Elev=21.82' Storage=345 cf Inflow=0.19 cfs 0.050 af
Discarded=0.07 cfs 0.050 af Primary=0.00 cfs 0.000 af Outflow=0.07 cfs 0.050 af

Link DP-1: Off-Site Inflow=0.02 cfs 0.016 af
Primary=0.02 cfs 0.016 af

Link DP-2: Wetlands Inflow=0.08 cfs 0.011 af
Primary=0.08 cfs 0.011 af

Link DP-3: Existing Bogs Inflow=0.43 cfs 0.219 af
Primary=0.43 cfs 0.219 af

Total Runoff Area = 22.437 ac Runoff Volume = 0.401 af Average Runoff Depth = 0.21"
99.96% Pervious = 22.429 ac 0.04% Impervious = 0.008 ac

Summary for Subcatchment PR-1: Subcat PR-1

Runoff = 0.02 cfs @ 15.57 hrs, Volume= 0.016 af, Depth= 0.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
2.465	30	Meadow, non-grazed, HSG A
2.465		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.5	50	0.0400	0.11		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
6.6	293	0.0400	0.74		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
0.3	14	0.0430	0.77		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
14.4	357	Total			

Summary for Subcatchment PR-2: Subcat PR-2

Runoff = 0.17 cfs @ 14.69 hrs, Volume= 0.105 af, Depth= 0.18"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

	Area (ac)	CN	Description
	0.463	76	Gravel roads, HSG A
	6.249	30	Meadow, non-grazed, HSG A
*	0.008	98	Concrete pads, HSG A
	0.456	30	Woods, Good, HSG A
	7.176	33	Weighted Average
	7.168		99.89% Pervious Area
	0.008		0.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.9	50	0.1200	0.17		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
14.1	560	0.0320	0.66		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
19.0	610	Total			

Summary for Subcatchment PR-3: Subcat PR-3

Runoff = 0.19 cfs @ 12.54 hrs, Volume= 0.050 af, Depth= 0.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
0.270	76	Gravel roads, HSG A
1.221	30	Meadow, non-grazed, HSG A
1.491	38	Weighted Average
1.491		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.6	50	0.0300	0.18		**Sheet Flow,** Grass: Short n= 0.150 P2= 3.44"
12.7	400	0.0200	0.52		**Shallow Concentrated Flow,** Kv= 3.7 fps
17.3	450	Total			

Summary for Subcatchment PR-4: Subcat PR-4

Runoff = 0.01 cfs @ 15.52 hrs, Volume= 0.006 af, Depth= 0.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
0.928	30	Meadow, non-grazed, HSG A
0.928		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.5	50	0.0400	0.11		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
3.1	152	0.0492	0.82		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
10.6	202	Total			

Summary for Subcatchment PR-5: Subcat PR-5

Runoff = 0.07 cfs @ 12.57 hrs, Volume= 0.030 af, Depth= 0.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
0.141	76	Gravel roads, HSG A
1.228	30	Meadow, non-grazed, HSG A
1.369	35	Weighted Average
1.369		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.5	50	0.0300	0.10		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
4.3	172	0.0319	0.66		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
12.8	222	Total			

Summary for Subcatchment PR-6: Subcat PR-6

Runoff = 0.28 cfs @ 12.68 hrs, Volume= 0.113 af, Depth= 0.30"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
0.559	76	Gravel roads, HSG A
3.877	30	Meadow, non-grazed, HSG A
0.005	30	Woods, Good, HSG A
4.441	36	Weighted Average
4.441		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	50	0.1840	0.90		**Sheet Flow, Dirt** Fallow n= 0.050 P2= 3.44"
0.1	22	0.0450	3.42		**Shallow Concentrated Flow, Dirt** Unpaved Kv= 16.1 fps
21.0	361	0.0060	0.29		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
22.0	433	Total			

Summary for Subcatchment PR-7: Subcat PR-7

Runoff = 0.06 cfs @ 13.79 hrs, Volume= 0.035 af, Depth= 0.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
0.028	72	Dirt roads, HSG A
0.157	76	Gravel roads, HSG A
1.505	30	Meadow, non-grazed, HSG A
0.249	30	Woods, Good, HSG A
1.939	34	Weighted Average
1.939		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.5	50	0.0140	0.07		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
5.2	122	0.0110	0.39		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
0.4	27	0.0040	1.02		**Shallow Concentrated Flow, Gravel** Unpaved Kv= 16.1 fps
17.1	199	Total			

Summary for Subcatchment PR-8: Subcat PR-8

Runoff = 0.06 cfs @ 13.84 hrs, Volume= 0.036 af, Depth= 0.18"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
0.186	76	Gravel roads, HSG A
1.745	30	Meadow, non-grazed, HSG A
0.554	30	Woods, Good, HSG A
2.485	33	Weighted Average
2.485		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	38	0.4870	1.26		**Sheet Flow, Dirt** Fallow n= 0.050 P2= 3.44"
3.5	12	0.0160	0.06		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
4.4	103	0.0110	0.39		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
8.4	153	Total			

Summary for Subcatchment PR-9: Subcat PR-9

Runoff = 0.08 cfs @ 12.22 hrs, Volume= 0.011 af, Depth= 0.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.04"

Area (ac)	CN	Description
0.029	72	Dirt roads, HSG A
0.002	89	Dirt roads, HSG D
0.014	98	Water Surface, 0% imp, HSG A
0.098	30	Woods, Good, HSG A
0.143	46	Weighted Average
0.143		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.9	50	0.0500	0.12		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
4.6	180	0.0306	0.65		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
11.5	230	Total			

Summary for Pond 1P: Infiltration Basin

Inflow Area = 7.176 ac, 0.11% Impervious, Inflow Depth = 0.18" for 25-Year event
Inflow = 0.17 cfs @ 14.69 hrs, Volume= 0.105 af
Outflow = 0.17 cfs @ 14.76 hrs, Volume= 0.105 af, Atten= 0%, Lag= 4.1 min
Discarded = 0.17 cfs @ 14.76 hrs, Volume= 0.105 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 21.51' @ 14.76 hrs Surf.Area= 5,091 sf Storage= 42 cf

Plug-Flow detention time= 4.2 min calculated for 0.105 af (100% of inflow)
Center-of-Mass det. time= 4.2 min (1,054.7 - 1,050.5)

Volume	Invert	Avail.Storage	Storage Description
#1	21.50'	7,968 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
21.50	5,083	0	0
22.00	5,570	2,663	2,663
22.90	6,218	5,305	7,968

Device	Routing	Invert	Outlet Devices
#1	Discarded	21.50'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 17.30'
#2	Primary	20.60'	**10.0" Round Culvert** L= 41.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 20.60' / 20.40' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.55 sf
#3	Device 2	22.30'	**48.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#4	Primary	22.80'	**10.0' long Sharp-Crested Rectangular Weir** 2 End Contraction(s) 1.3' Crest Height

Discarded OutFlow Max=0.28 cfs @ 14.76 hrs HW=21.51' (Free Discharge)
└**1=Exfiltration** (Controls 0.28 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=21.50' (Free Discharge)
└**2=Culvert** (Passes 0.00 cfs of 1.49 cfs potential flow)
└**3=Orifice/Grate** (Controls 0.00 cfs)
└**4=Sharp-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Pond 2P: Infiltration Basin

Inflow Area = 1.491 ac, 0.00% Impervious, Inflow Depth = 0.40" for 25-Year event
Inflow = 0.19 cfs @ 12.54 hrs, Volume= 0.050 af
Outflow = 0.07 cfs @ 15.02 hrs, Volume= 0.050 af, Atten= 63%, Lag= 149.1 min
Discarded = 0.07 cfs @ 15.02 hrs, Volume= 0.050 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 21.82' @ 15.02 hrs Surf.Area= 1,179 sf Storage= 345 cf

Plug-Flow detention time= 48.0 min calculated for 0.050 af (100% of inflow)
Center-of-Mass det. time= 47.9 min (1,026.1 - 978.2)

Volume	Invert	Avail.Storage	Storage Description
#1	21.50'	2,016 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
21.50	972	0	0
22.00	1,295	567	567
23.00	1,604	1,450	2,016

Device	Routing	Invert	Outlet Devices
#1	Discarded	21.50'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 17.30'
#2	Primary	21.50'	**6.0" Round Culvert** L= 50.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 21.50' / 21.00' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	22.40'	**48.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#4	Primary	22.90'	**10.0' long Sharp-Crested Rectangular Weir** 2 End Contraction(s) 1.3' Crest Height

Discarded OutFlow Max=0.07 cfs @ 15.02 hrs HW=21.82' (Free Discharge)
└**1=Exfiltration** (Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=21.50' (Free Discharge)
└**2=Culvert** (Controls 0.00 cfs)
└**3=Orifice/Grate** (Controls 0.00 cfs)
└**4=Sharp-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link DP-1: Off-Site

Inflow Area = 2.465 ac, 0.00% Impervious, Inflow Depth = 0.08" for 25-Year event
Inflow = 0.02 cfs @ 15.57 hrs, Volume= 0.016 af
Primary = 0.02 cfs @ 15.57 hrs, Volume= 0.016 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: Wetlands

Inflow Area = 8.810 ac, 0.09% Impervious, Inflow Depth = 0.01" for 25-Year event
Inflow = 0.08 cfs @ 12.22 hrs, Volume= 0.011 af
Primary = 0.08 cfs @ 12.22 hrs, Volume= 0.011 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-3: Existing Bogs

Inflow Area = 11.162 ac, 0.00% Impervious, Inflow Depth = 0.24" for 25-Year event
Inflow = 0.43 cfs @ 12.70 hrs, Volume= 0.219 af
Primary = 0.43 cfs @ 12.70 hrs, Volume= 0.219 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

100-Year Storm Event – Proposed

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PR-1: Subcat PR-1 Runoff Area=2.465 ac 0.00% Impervious Runoff Depth=0.32"
Flow Length=357' Tc=14.4 min CN=30 Runoff=0.14 cfs 0.066 af

Subcatchment PR-2: Subcat PR-2 Runoff Area=7.176 ac 0.11% Impervious Runoff Depth=0.52"
Flow Length=610' Tc=19.0 min CN=33 Runoff=1.17 cfs 0.311 af

Subcatchment PR-3: Subcat PR-3 Runoff Area=1.491 ac 0.00% Impervious Runoff Depth=0.90"
Flow Length=450' Tc=17.3 min CN=38 Runoff=0.65 cfs 0.112 af

Subcatchment PR-4: Subcat PR-4 Runoff Area=0.928 ac 0.00% Impervious Runoff Depth=0.32"
Flow Length=202' Tc=10.6 min CN=30 Runoff=0.06 cfs 0.025 af

Subcatchment PR-5: Subcat PR-5 Runoff Area=1.369 ac 0.00% Impervious Runoff Depth=0.67"
Flow Length=222' Tc=12.8 min CN=35 Runoff=0.38 cfs 0.076 af

Subcatchment PR-6: Subcat PR-6 Runoff Area=4.441 ac 0.00% Impervious Runoff Depth=0.74"
Flow Length=433' Tc=22.0 min CN=36 Runoff=1.32 cfs 0.275 af

Subcatchment PR-7: Subcat PR-7 Runoff Area=1.939 ac 0.00% Impervious Runoff Depth=0.59"
Flow Length=199' Tc=17.1 min CN=34 Runoff=0.41 cfs 0.096 af

Subcatchment PR-8: Subcat PR-8 Runoff Area=2.485 ac 0.00% Impervious Runoff Depth=0.52"
Flow Length=153' Tc=8.4 min CN=33 Runoff=0.47 cfs 0.108 af

Subcatchment PR-9: Subcat PR-9 Runoff Area=0.143 ac 0.00% Impervious Runoff Depth=1.61"
Flow Length=230' Tc=11.5 min CN=46 Runoff=0.18 cfs 0.019 af

Pond 1P: Infiltration Basin Peak Elev=22.08' Storage=3,104 cf Inflow=1.17 cfs 0.311 af
Discarded=0.35 cfs 0.311 af Primary=0.00 cfs 0.000 af Outflow=0.35 cfs 0.311 af

Pond 2P: Infiltration Basin Peak Elev=22.43' Storage=1,145 cf Inflow=0.65 cfs 0.112 af
Discarded=0.09 cfs 0.094 af Primary=0.17 cfs 0.018 af Outflow=0.26 cfs 0.112 af

Link DP-1: Off-Site Inflow=0.14 cfs 0.066 af
Primary=0.14 cfs 0.066 af

Link DP-2: Wetlands Inflow=0.21 cfs 0.037 af
Primary=0.21 cfs 0.037 af

Link DP-3: Existing Bogs Inflow=2.58 cfs 0.579 af
Primary=2.58 cfs 0.579 af

Total Runoff Area = 22.437 ac Runoff Volume = 1.088 af Average Runoff Depth = 0.58"
99.96% Pervious = 22.429 ac 0.04% Impervious = 0.008 ac

Summary for Subcatchment PR-1: Subcat PR-1

Runoff = 0.14 cfs @ 12.60 hrs, Volume= 0.066 af, Depth= 0.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
2.465	30	Meadow, non-grazed, HSG A
2.465		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.5	50	0.0400	0.11		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
6.6	293	0.0400	0.74		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
0.3	14	0.0430	0.77		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
14.4	357	Total			

Summary for Subcatchment PR-2: Subcat PR-2

Runoff = 1.17 cfs @ 12.56 hrs, Volume= 0.311 af, Depth= 0.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

	Area (ac)	CN	Description
	0.463	76	Gravel roads, HSG A
	6.249	30	Meadow, non-grazed, HSG A
*	0.008	98	Concrete pads, HSG A
	0.456	30	Woods, Good, HSG A
	7.176	33	Weighted Average
	7.168		99.89% Pervious Area
	0.008		0.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.9	50	0.1200	0.17		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
14.1	560	0.0320	0.66		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
19.0	610	Total			

Summary for Subcatchment PR-3: Subcat PR-3

Runoff = 0.65 cfs @ 12.40 hrs, Volume= 0.112 af, Depth= 0.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
0.270	76	Gravel roads, HSG A
1.221	30	Meadow, non-grazed, HSG A
1.491	38	Weighted Average
1.491		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.6	50	0.0300	0.18		**Sheet Flow,** Grass: Short n= 0.150 P2= 3.44"
12.7	400	0.0200	0.52		**Shallow Concentrated Flow,** Kv= 3.7 fps
17.3	450	Total			

Summary for Subcatchment PR-4: Subcat PR-4

Runoff = 0.06 cfs @ 12.54 hrs, Volume= 0.025 af, Depth= 0.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
0.928	30	Meadow, non-grazed, HSG A
0.928		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.5	50	0.0400	0.11		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
3.1	152	0.0492	0.82		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
10.6	202	Total			

Summary for Subcatchment PR-5: Subcat PR-5

Runoff = 0.38 cfs @ 12.41 hrs, Volume= 0.076 af, Depth= 0.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
0.141	76	Gravel roads, HSG A
1.228	30	Meadow, non-grazed, HSG A
1.369	35	Weighted Average
1.369		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.5	50	0.0300	0.10		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
4.3	172	0.0319	0.66		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
12.8	222	Total			

Summary for Subcatchment PR-6: Subcat PR-6

Runoff = 1.32 cfs @ 12.52 hrs, Volume= 0.275 af, Depth= 0.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
0.559	76	Gravel roads, HSG A
3.877	30	Meadow, non-grazed, HSG A
0.005	30	Woods, Good, HSG A
4.441	36	Weighted Average
4.441		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	50	0.1840	0.90		**Sheet Flow, Dirt** Fallow n= 0.050 P2= 3.44"
0.1	22	0.0450	3.42		**Shallow Concentrated Flow, Dirt** Unpaved Kv= 16.1 fps
21.0	361	0.0060	0.29		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
22.0	433	Total			

Summary for Subcatchment PR-7: Subcat PR-7

Runoff = 0.41 cfs @ 12.50 hrs, Volume= 0.096 af, Depth= 0.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
0.028	72	Dirt roads, HSG A
0.157	76	Gravel roads, HSG A
1.505	30	Meadow, non-grazed, HSG A
0.249	30	Woods, Good, HSG A
1.939	34	Weighted Average
1.939		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.5	50	0.0140	0.07		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
5.2	122	0.0110	0.39		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
0.4	27	0.0040	1.02		**Shallow Concentrated Flow, Gravel** Unpaved Kv= 16.1 fps
17.1	199	Total			

Summary for Subcatchment PR-8: Subcat PR-8

Runoff = 0.47 cfs @ 12.40 hrs, Volume= 0.108 af, Depth= 0.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
0.186	76	Gravel roads, HSG A
1.745	30	Meadow, non-grazed, HSG A
0.554	30	Woods, Good, HSG A
2.485	33	Weighted Average
2.485		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	38	0.4870	1.26		**Sheet Flow, Dirt** Fallow n= 0.050 P2= 3.44"
3.5	12	0.0160	0.06		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
4.4	103	0.0110	0.39		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
8.4	153	Total			

Summary for Subcatchment PR-9: Subcat PR-9

Runoff = 0.18 cfs @ 12.19 hrs, Volume= 0.019 af, Depth= 1.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.58"

Area (ac)	CN	Description
0.029	72	Dirt roads, HSG A
0.002	89	Dirt roads, HSG D
0.014	98	Water Surface, 0% imp, HSG A
0.098	30	Woods, Good, HSG A
0.143	46	Weighted Average
0.143		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.9	50	0.0500	0.12		**Sheet Flow, Meadow** n= 0.320 P2= 3.44"
4.6	180	0.0306	0.65		**Shallow Concentrated Flow, Meadow** Kv= 3.7 fps
11.5	230	Total			

Summary for Pond 1P: Infiltration Basin

Inflow Area = 7.176 ac, 0.11% Impervious, Inflow Depth = 0.52" for 100-Year event
Inflow = 1.17 cfs @ 12.56 hrs, Volume= 0.311 af
Outflow = 0.35 cfs @ 15.90 hrs, Volume= 0.311 af, Atten= 70%, Lag= 200.5 min
Discarded = 0.35 cfs @ 15.90 hrs, Volume= 0.311 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 22.08' @ 15.90 hrs Surf.Area= 5,627 sf Storage= 3,104 cf

Plug-Flow detention time= 94.6 min calculated for 0.311 af (100% of inflow)
Center-of-Mass det. time= 94.5 min (1,072.3 - 977.8)

Volume	Invert	Avail.Storage	Storage Description
#1	21.50'	7,968 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
21.50	5,083	0	0
22.00	5,570	2,663	2,663
22.90	6,218	5,305	7,968

Device	Routing	Invert	Outlet Devices
#1	Discarded	21.50'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 17.30'
#2	Primary	20.60'	**10.0" Round Culvert** L= 41.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 20.60' / 20.40' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.55 sf
#3	Device 2	22.30'	**48.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#4	Primary	22.80'	**10.0' long Sharp-Crested Rectangular Weir** 2 End Contraction(s) 1.3' Crest Height

Discarded OutFlow Max=0.35 cfs @ 15.90 hrs HW=22.08' (Free Discharge)
└**1=Exfiltration** (Controls 0.35 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=21.50' (Free Discharge)
└**2=Culvert** (Passes 0.00 cfs of 1.49 cfs potential flow)
└**3=Orifice/Grate** (Controls 0.00 cfs)
└**4=Sharp-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Pond 2P: Infiltration Basin

Inflow Area = 1.491 ac, 0.00% Impervious, Inflow Depth = 0.90" for 100-Year event
Inflow = 0.65 cfs @ 12.40 hrs, Volume= 0.112 af
Outflow = 0.26 cfs @ 13.00 hrs, Volume= 0.112 af, Atten= 60%, Lag= 36.1 min
Discarded = 0.09 cfs @ 13.00 hrs, Volume= 0.094 af
Primary = 0.17 cfs @ 13.00 hrs, Volume= 0.018 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 22.43' @ 13.00 hrs Surf.Area= 1,426 sf Storage= 1,145 cf

Plug-Flow detention time= 130.3 min calculated for 0.112 af (100% of inflow)
Center-of-Mass det. time= 130.3 min (1,065.8 - 935.5)

Volume	Invert	Avail.Storage	Storage Description
#1	21.50'	2,016 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
21.50	972	0	0
22.00	1,295	567	567
23.00	1,604	1,450	2,016

Device	Routing	Invert	Outlet Devices
#1	Discarded	21.50'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 17.30'
#2	Primary	21.50'	**6.0" Round Culvert** L= 50.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 21.50' / 21.00' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	22.40'	**48.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#4	Primary	22.90'	**10.0' long Sharp-Crested Rectangular Weir** 2 End Contraction(s) 1.3' Crest Height

Discarded OutFlow Max=0.09 cfs @ 13.00 hrs HW=22.43' (Free Discharge)
└**1=Exfiltration** (Controls 0.09 cfs)

Primary OutFlow Max=0.16 cfs @ 13.00 hrs HW=22.43' (Free Discharge)
└**2=Culvert** (Passes 0.16 cfs of 0.65 cfs potential flow)
└**3=Orifice/Grate** (Weir Controls 0.16 cfs @ 0.52 fps)
└**4=Sharp-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link DP-1: Off-Site

Inflow Area = 2.465 ac, 0.00% Impervious, Inflow Depth = 0.32" for 100-Year event
Inflow = 0.14 cfs @ 12.60 hrs, Volume= 0.066 af
Primary = 0.14 cfs @ 12.60 hrs, Volume= 0.066 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: Wetlands

Inflow Area = 8.810 ac, 0.09% Impervious, Inflow Depth = 0.05" for 100-Year event
Inflow = 0.21 cfs @ 13.00 hrs, Volume= 0.037 af
Primary = 0.21 cfs @ 13.00 hrs, Volume= 0.037 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Summary for Link DP-3: Existing Bogs

Inflow Area = 11.162 ac, 0.00% Impervious, Inflow Depth = 0.62" for 100-Year event
Inflow = 2.58 cfs @ 12.48 hrs, Volume= 0.579 af
Primary = 2.58 cfs @ 12.48 hrs, Volume= 0.579 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Appendix C: Standard 3 Computations and Supporting Documentation

- Preliminary Test Pit Data
- Soil Evaluation in accordance with Volume 3, Chapter 1 of the Handbook
- 72-hour drawdown analysis

Preliminary Test Pit Data

Memorandum

To: FILE

Date: July 21, 2021

Project #: 15225.02

From: Kenneth S. Staffier, PE
Soil Evaluator #2322

Re: Preliminary Test Pits
Proposed Solar Array
0 Route 25
Wareham, Massachusetts

Preliminary Test Pits

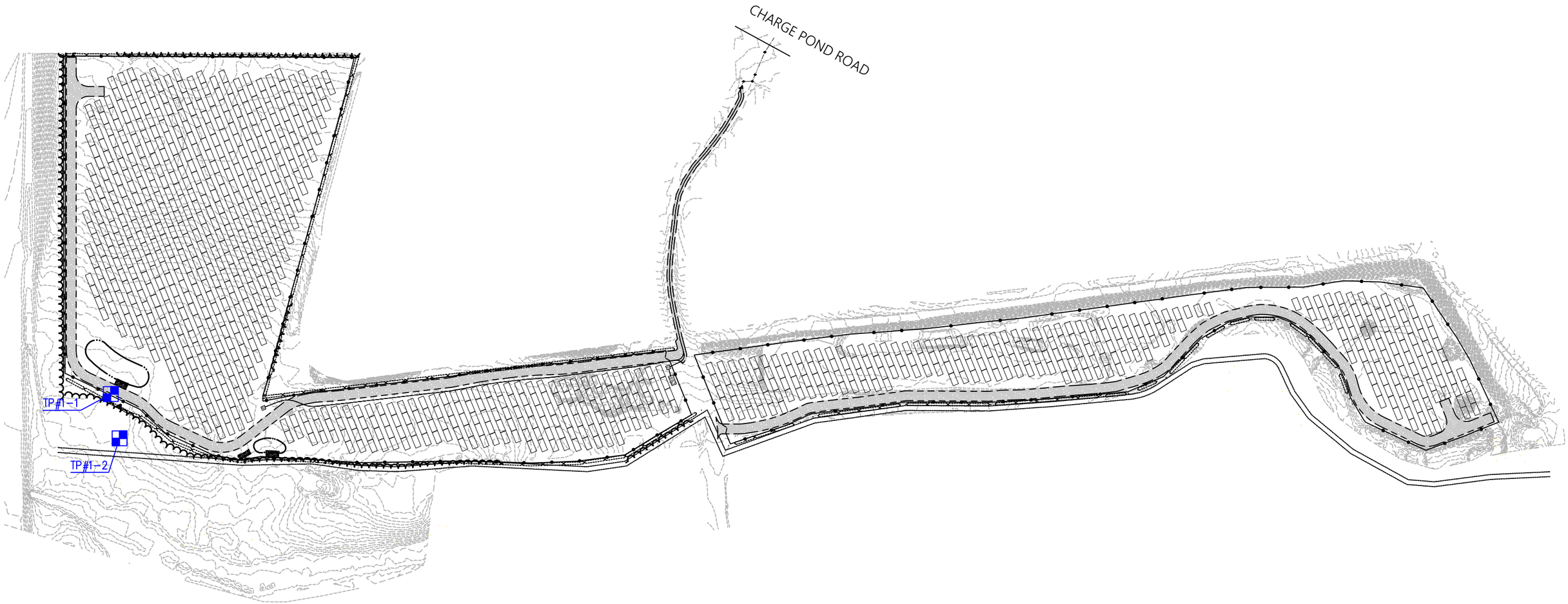
On April 15, 2021 VHB completed 6 hand-dug test pits at 0 Route 20 in Wareham, MA to determine the depth to estimated seasonal and general soil characteristics within the location of proposed stormwater management practices (SMPs). Two test pits were excavated to a depth ranging from 55" – 59". The soil profile was generally consistent with an A1 horizon (forest mat) ranging from 4" – 5" thick; an A2 Horizon (Loamy Sand) ranging from 5" – 11" thick, a B horizon (Loamy Sand) ranging from 11" – 15" thick; and a C horizon (Fine Sand – Loamy Sand) ranging from 31" – 32" thick. Groundwater was encountered at depths ranging from 44" – 50".

The test pit logs are outlined in Table 1 below:

Table 1 Test Pit Data

Test Pit	A_1 Horizon	A_2 Horizon	B Horizon	C Horizon	Depth to Estimated Seasonal High Groundwater
1-1	0"-4" Forest Mat	4"-9" Loamy Sand	9"-24" Loamy Sand	24" -55" Fine Sand- Loamy Sand	44"
1-2	0"-5" Forest Mat	5"-16" Loamy Sand	16"-27" Loamy Sand	27"-59" Fine Sand – Sandy Loam	50"

1 All measurements from ground surface



0 200 400 Feet

Test Pit Figure
0 Route 25
Wareham, MA

Figure 1

07/22/2021

Soil Evaluation and Analysis

United States Department of Agriculture

NRCS

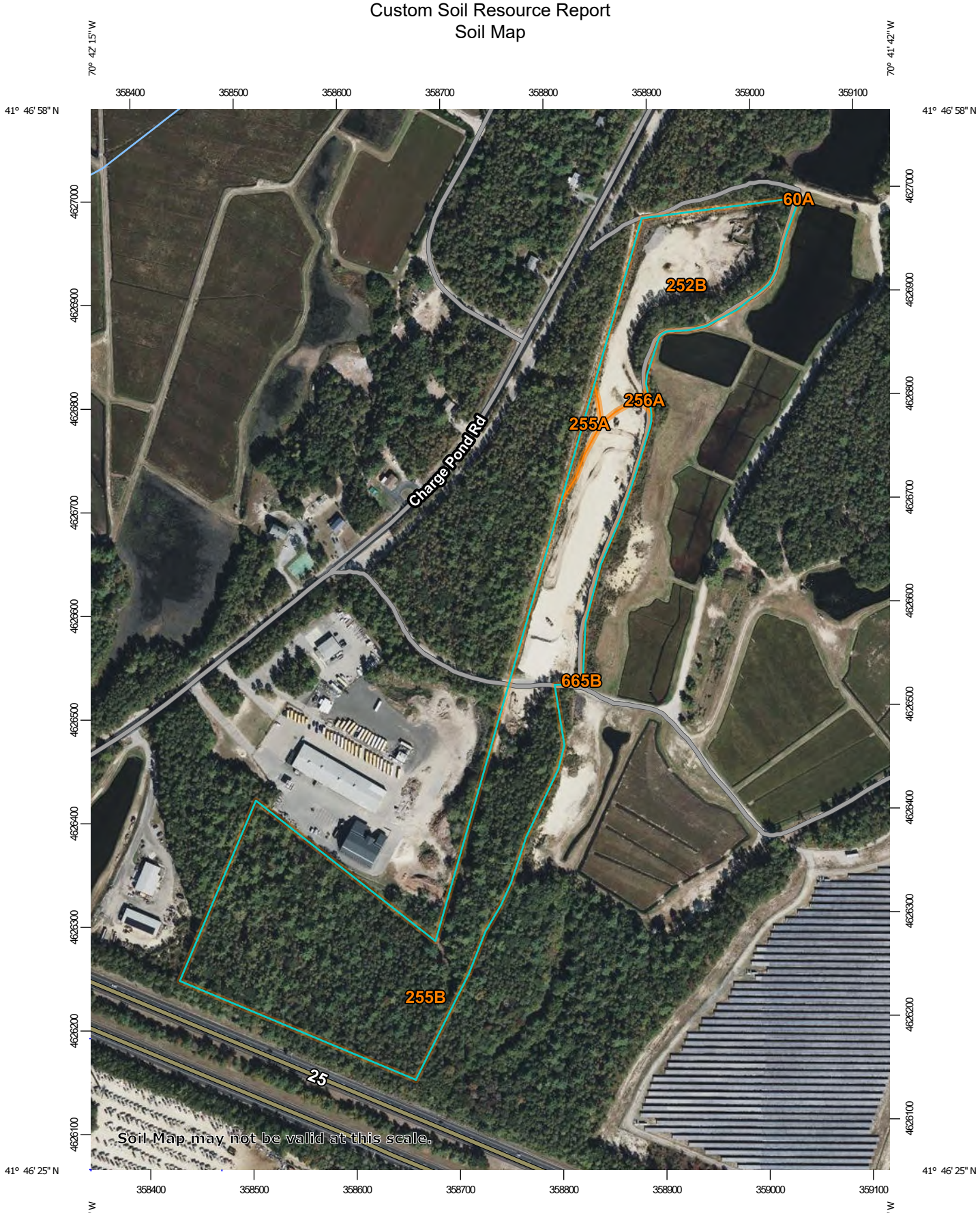
Natural Resources Conservation Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Plymouth County, Massachusetts

June 18, 2021

Custom Soil Resource Report
Soil Map



Map Scale: 1:4,990 if printed on A portrait (8.5" x 11") sheet.

N

Meters
0 50 100 200 300

Feet
0 200 400 800 1200

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 19N WGS84

MAP LEGEND

Area of Interest (AOI)
- Area of Interest (AOI)

Soils
- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

Special Point Features
- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

Water Features
- Streams and Canals

Transportation
- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background
- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Plymouth County, Massachusetts
Survey Area Data: Version 13, Jun 9, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 25, 2020—Oct 9, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
60A	Swansea coarse sand, 0 to 2 percent slopes	0.0	0.0%
252B	Carver coarse sand, 3 to 8 percent slopes	4.8	21.6%
255A	Windsor loamy sand, 0 to 3 percent slopes	0.3	1.2%
255B	Windsor loamy sand, 3 to 8 percent slopes	17.3	77.1%
256A	Deerfield loamy fine sand, 0 to 3 percent slopes	0.0	0.1%
665B	Udipsamments, 0 to 8 percent slopes	0.0	0.0%
Totals for Area of Interest		**22.4**	**100.0%**

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it

was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

72-Hour Drawdown Analysis

Recharge Calculations

Project	Proposed Solar Array	**Project #**	15225.01
Calculated by	SKE	**Date**	06/28/21
Checked by	KSS	**Date**	06/28/21

REQUIRED RECHARGE VOLUME

No increase in impervious area, therefore required charge volume is 0.

PROVIDED RECHARGE VOLUME

BASIN #1

Infiltration Basin

Volumes provided below the lowest outlet at elevation: 22.5

Provided Volume:

Elevation	Area (ft^2)	Cumulative Volume (ft^3)
21.5	5,083	0
22.0	5,570	2,663
22.8	6,218	**7,378**

Drawdown: $(V_{Infiltration}/A_{Bottom})$/Rawl's Rate

Rawls Recharge Rate:	**2.41**	(in/hr)
Drawdown Time:	**7**	(hours)

BASIN #2

Infiltration Basin

Volumes provided below the lowest outlet at elevation: 22.5

Provided Volume:

Elevation	Area (ft^2)	Cumulative Volume (ft^3)
21.5	972	0
22.0	1,295	567
22.9	1,604	**1,871**

Drawdown: $(V_{Infiltration}/A_{Bottom})$/Rawl's Rate

Rawls Recharge Rate:	**2.41**	(in/hr)
Drawdown Time:	**10**	(hours)

RECHARGE VOLUME SUMMARY

Required Recharge Volume:	**0**	(ft^3)
Total Recharge Volume Provided:	**9,250**	(ft^3)

Appendix D: Standard 4 Supporting Information

› Water Quality Volume Calculations

Water Quality Volume Calculations

Water Quality Volume Calculations

Project	Proposed Solar Array	**Project #**	15225.01
Calculated by	SKE	**Date**	07/15/21
Checked by	JRG	**Date**	07/15/21

BASIN #1P

Runoff from subcatchment areas PR-2

Water Quality Storm Runoff Depth	(in)	0.0
Total Impervious Area	(ft^2)	0

BASIN WQV:

Required Volume:	**Runoff Depth to be Treated (in)**	**Required Volume (ft^3)**
	0.0	**0**

Provided Volume:	**Elevation**	**Area (ft^2)**	**Cumulative Volume (ft^3)**
	21.5	5,083	0
	22.0	5,570	2,663
	22.8	6,218	**7,378**

FREEBOARD CHECK:

100-YR Peak Elevation:	22.0
Maximum Basin Elevation:	22.8
Basin Freeboard:	**0.8**

BASIN #2P

Runoff from subcatchment areas PR-3

Water Quality Storm Runoff Depth	(in)	0.0
Total Impervious Area	(ft^2)	0

BASIN WQV:

Required Volume:	**Runoff Depth to be Treated (in)**	**Required Volume (ft^3)**
	0.0	**0**

Provided Volume:	**Elevation**	**Area (ft^2)**	**Cumulative Volume (ft^3)**
	21.5	972	0
	22.0	1,295	567
	22.9	1,604	**1,871**

FREEBOARD CHECK:

100-YR Peak Elevation:	22.4
Maximum Basin Elevation:	22.9
Basin Freeboard:	**0.5**

** Per MassDEP Treatment Requirement*

Appendix E: Standard 8 Supporting Information

› List of Recommended Construction Period BMPs

List of Recommended Construction Period BMPs

Erosion and Sedimentation Control Measures

The Project will disturb approximately 19.3 acres of land and is therefore required to obtain coverage under the Environmental Protection Agency (EPA) National Pollutant Discharge Elimination System (NPDES) Construction General Permit. As required under this permit, a Stormwater Pollution Prevention Plan (SWPPP) will be developed and submitted before land disturbance begins.

The following erosion and sedimentation controls are for use during the earthwork and construction phases of the project. The following controls are provided as recommendations for the site contractor and do not constitute or replace the final Stormwater Pollution Prevention Plan that must be fully implemented by the Contractor and owner in Compliance with EPA NPDES regulations.

Straw Wattles

Straw wattles will be placed to trap sediment transported by runoff before it reaches the drainage system or leaves the construction site. Wattles will be set at least four inches into the existing ground to minimize undercutting by runoff.

Silt Fencing

In areas where high runoff velocities or high sediment loads are expected, straw bale barriers will be backed up with silt fencing. This semi-permeable barrier made of a synthetic porous fabric will provide additional protection. The silt fences and straw bale barrier will be replaced as determined by periodic field inspections.

Gravel and Construction Entrance/Exit

A temporary crushed-stone construction entrance/exit will be constructed. A cross slope will be placed in the entrance to direct runoff to a protected catch basin inlet or settling area. If deemed necessary after construction begins, a wash pad may be included to wash off vehicle wheels before leaving the project site.

Diversion Channels

Diversion channels will be used to collect runoff from construction areas and discharge to either sedimentation basins or protected catch basin inlets.

Temporary Sediment Basins

Temporary sediment basins will be designed either as excavations or bermed stormwater detention structures (depending on grading) that will retain runoff for a sufficient period of time to allow suspended soil particles to settle out prior to discharge. These temporary basins will be located based on construction needs as determined by the contractor and outlet devices will be designed to control velocity and sediment. Points of discharge from sediment basins will be stabilized to minimize erosion.

Vegetative Slope Stabilization

Stabilization of open soil surfaces will be implemented within 14 days after grading or construction activities have temporarily or permanently ceased, unless there is sufficient snow cover to prohibit implementation. Vegetative slope stabilization will be used to minimize erosion on slopes of 3:1 or flatter. Annual grasses, such as annual rye, will be used to ensure rapid germination and production of root mass. Permanent stabilization will be completed with the planting of perennial grasses or legumes. Establishment of temporary and permanent vegetative cover may be established by hydro-seeding or sodding. A suitable topsoil, good seedbed preparation, and adequate lime, fertilizer and water will be provided for effective establishment of these vegetative stabilization methods. Mulch will also be used after permanent seeding to protect soil from the impact of falling rain and to increase the capacity of the soil to absorb water.

Maintenance

- The contractor or subcontractor will be responsible for implementing each control shown on the Sedimentation and Erosion Control Plan. In accordance with EPA regulations, the contractor must sign a copy of a certification to verify that a plan has been prepared and that permit regulations are understood.

- The on-site contractor will inspect all sediment and erosion control structures periodically and after each rainfall event. Records of the inspections will be prepared and maintained on-site by the contractor.

- Silt shall be removed from behind barriers if greater than 6-inches deep or as needed.

- Damaged or deteriorated items will be repaired immediately after identification.

- The underside of straw bales should be kept in close contact with the earth and reset as necessary.

- Sediment that is collected in structures shall be disposed of properly and covered if stored on-site.

- Erosion control structures shall remain in place until all disturbed earth has been securely stabilized. After removal of structures, disturbed areas shall be regraded and stabilized as necessary.

The sedimentation and erosion control plan is included in project plan set.

EMERGENCY NOTIFICATION PHONE NUMBERS

1. **SUPERVISOR/MANAGER - TBD**

   Name: _____ Beeper: _____

   Phone: _____ Home Phone: _____

   ALTERENATE

   Name: _____ Beeper: _____

   Phone: _____ Home Phone: _____

2. **FIRE DEPARTMENT**

   Emergency: **911**

   Business: (781) 270-1925

   POLICE DEPARTMENT

   Emergency: **911**

   Business: _____

3. **CLEANUP CONTRACTOR - TBD**

   Address: _____

   Phone: _____

4. **MASSACHUSETTS DEPARTMENT OF ENVIRONMENTAL PROTECTION**

   Emergency: _____

   Northeast Region – Woburn Office: _____

5. **NATIONAL RESPONSE CENTER**

   Phone: (800) 424-8802

   ALTERNATE: U.S. ENVIRONMENTAL PROTECTION AGENCY

   Emergency: (617) 223-7265

   Business: (617) 860-4300

6. **CONSERVATION COMMISSION**

   Contact: (508) 291-3100 x 6505

   BOARD OF HEALTH

   Contact: (508) 291-3100 x 3197

7. **FACILITY MANAGER - TBD**

   Name: _____

   Phone: _____

Appendix F: Standard 9 Supporting Information

› Operations & Maintenance Plan

Proposed Large-Scale Ground-Mounted Solar Photovoltaic Installation

0 Route 25
Wareham, MA

PREPARED FOR

Wareham PV I, LLC
330 Congress Street, 6th Floor
Boston, MA 02210
617.377.4301

PREPARED BY

101 Walnut Street
PO Box 9151
Watertown, MA 02471
617.924.1770

August 2021

Table of Contents

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Project Information

Site

Proposed Large-Scale Ground-Mounted Solar Photovoltaic Installation
0 Roue 25
Wareham, MA

Developer

Wareham PV I, LLC
330 Congress Street, 6th Floor
Boston, MA 02210
617.377.4301

Site Supervisor - TBD

Site Manager Name
Site Manager Address
Site Manager City, State Zip
Site Manager Phone Number

Site Contact - TBD

Name: _____

Telephone: _____

Cell phone: _____

Email: _____

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Section A: Source Control

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A Source Control

A comprehensive source control program will be implemented at the Project Site, which includes the following components:

- Outlet control structure cleaning
    - Refer to Section D of this manual
- Clearing litter from the access drives, and perimeter landscape areas
    - Refer to Section D & E of this manual
- Spill Prevention training
    - Refer to Section B of this manual

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Section B: Spill Prevention

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B Spill Prevention

Spill prevention equipment and training will be provided by Longroad Energy.

B.1 Initial Notification

In the event of a spill the facility and/or construction manager or supervisor will be notified immediately.

Facility Manager (name):	TBD
Facility Manager (phone):	TBD
Construction Manager (name) :	
Construction Manager (phone):	

The supervisor will first contact the Fire Department and then notify the Police Department, the Public Health Commission and the Conservation Commission. The Fire Department is ultimately responsible for matters of public health and safety and should be notified immediately.

B.2 Further Notification

Based on the assessment from the Fire Chief, additional notification to a cleanup contractor may be made. The Massachusetts Department of Environmental Protection (DEP) and the EPA may be notified depending upon the nature and severity of the spill. The Fire Chief will be responsible for determining the level of cleanup and notification required. The attached list of emergency phone numbers shall be posted in the main construction/facility office and readily accessible to all employees. A hazardous waste spill report shall be completed as necessary using the attached form.

Emergency Notification Phone Numbers

1. FACILITY MANAGER - TBD

Name: _____ Phone: _____

Beeper/Cell: _____

Home Phone: _____

Alternate Contact: _____ Phone: _____

Beeper/Cell: _____

Home Phone: _____

2. FIRE & POLICE DEPARTMENT Emergency: 911

3. CLEANUP CONTRACTOR

Address: _____ Phone: _____

4. MASSACHUSETTS DEPARTMENT OF ENVIRONMENTAL PROTECTION (DEP) Emergency: (888) 304-1133

5. NATIONAL RESPONSE CENTER Phone: (800) 424-8802

Alternate: U.S. Environmental Protection Agency Business: (888) 372-7341

6. WAREHAM HEALTH DEPARTMENT Phone: (508) 291-3100 x 3197

WAREHAM CONSERVATION COMMISSION: Phone: (508) 291-3100 x 6505

Hazardous Waste & Oil Spill Report

Date: _____ **Time:** _____ **AM / PM**

Exact location (Transformer #): _____

Type of equipment: _____ **Make:** _____ **Size:** _____

S / N: _____ **Weather Conditions:** _____

On or near water? ☐ Yes ☐ No **If yes, name of body of water:** _____

Type of chemical / oil spilled: _____

Amount of chemical / oil spilled: _____

Cause of spill: _____

Measures taken to contain or clean up spill: _____

Amount of chemical / oil recovered: _____ **Method:** _____

Material collected as a result of cleanup:

_____ drums containing _____

_____ drums containing _____

_____ drums containing _____

Location and method of debris disposal: _____

Name and address of any person, firm, or corporation suffering charges: _____

Procedures, method, and precautions instituted to prevent a similar occurrence from recurring: _____

Spill reported by General Office by: _____ **Time:** _____ **AM / PM**

Spill reported to DEP / National Response Center by: _____

DEP Date: _____ **Time:** _____ **AM / PM** **Inspector:** _____

NRC Date: _____ **Time:** _____ **AM / PM** **Inspector:** _____

Additional comments: _____

B.3 Assessment – Initial Containment

The supervisor or manager will assess the incident and initiate containment control measures with the appropriate spill containment equipment included in the spill kit kept on-site. A list of recommended spill equipment to be kept on site is included on the following page.

Fire / Police Department: 911

Wareham Health Department (508) 291-3100 x 3197

Wareham Conservation Commission: (508) 291-3100 x 6505

Emergency Response Equipment

The following equipment and materials shall be maintained at all times and stored in a secure area for long-term emergency response need.

Supplies	Quantity	Recommended Suppliers
› Sorbent Pillows/"Pigs"	2	http://www.newpig.com Item # KIT276 — mobile container with two pigs
› Sorbent Boom/Sock	25 feet	http://www.forestry-suppliers.com
› Sorbent Pads	50	
› Lite-Dri® Absorbent	5 pounds	
› Shovel	1	Item # 33934 — Shovel (or equivalent)
› Pry Bar	1	Item # 43210 — Manhole cover pick (or equivalent)
› Goggles	1 pair	Item # 23334 — Goggles (or equivalent)
› Gloves – Heavy	1 pair	Item # 90926 — Gloves (or equivalent)

Section C: Snow Management

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C Snow Management

- Access roads on site will be plowed and snow will be pushed to the shoulder of the gravel road. Snow will be managed to prevent blockage of stormwater drainage swales and stormwater management features. Snow combined with sand and debris may block a storm drainage system, diminishing the infiltration capacity of the system and causing localized flooding.
- Sand and debris deposited on vegetated or paved areas shall be cleared from the site and properly disposed of at the end of the snow season, no later than May 15.
- Snow shall not be dumped into any waterbody, pond, or wetland resource area.

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Section D: Maintenance of Stormwater Management Systems

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D Maintenance of Stormwater Management Systems

D.1 Structural Stormwater Management Devices

D.1.1 Basin Outlet Control Structures

The proper removal of sediments and associated pollutants and trash occurs only when catch basin inlets and sumps are cleaned out regularly. The more frequent the cleaning, the less likely sediments will be re-suspended and subsequently discharged. In addition, frequent cleaning also results in more volume available for future deposition and enhances the overall performance. As noted in the pavement Operation and Maintenance (O&M) section, more frequent sweeping of paved surfaces will result in less accumulation in catch basins, less cleaning of subsurface structures, and less disposal costs.

There are two (2) outlet control structures on site. Disposal of all sediments must be in accordance with applicable local, state, and federal guidelines. A map of the outlet control structure locations is included in Section E.5 Maintenance Checklists and Device Location Maps.

Inspections and Cleaning

- All outlet control structures shall be inspected at least four times per year and cleaned a minimum of at least once per year.
- Sediment (if more than six inches deep) and/or floatable pollutants shall be pumped from the basin and disposed of at an approved offsite facility in accordance with all applicable regulations.
- Any structural damage or other indication of malfunction will be reported to the site manager and repaired as necessary
- During colder periods, the grates must be kept free of snow and ice.
- During warmer periods, the grates must be kept free of leaves, litter, sand, and debris.

D.1.2 Stormwater Outfalls

The stormwater drainage system at Proposed Solar Array has two (2) outfall locations where treated stormwater is discharged to surface wetlands.

- Inspect outfall locations monthly for the first three months after construction to ensure proper functioning and correct any areas that have settled or experienced washouts.
- Inspect outfalls annually after initial three month period.
- Annual inspections should be supplemented after large storms, when washouts may occur.
- Maintain vegetation around outfalls to prevent blockages at the outfall.
- Maintain rip rap pad below each outfall and replace any washouts.
- Remove and dispose of any trash or debris at the outfall.

D.2 Vegetated Stormwater Management Devices

D.2.1 Surface Infiltration Basins

There are two (2) surface infiltration ponds on site. The infiltration ponds are completely vegetated basins that are designed to detain, clean and infiltrate roadway runoff. The maintenance of the infiltration basins may affect the functioning of stormwater management practices. This includes the condition of the side slope vegetation and the sediment deposits in the bottom of the ponds.

Initial Post-construction Inspection

- Infiltration basins should be inspected after every major storm for the first few months to ensure proper stabilization and function.

Long-term Maintenance

- The grass on the sideslopes and in the buffer areas should be mowed, and grass clippings, organic matter, and accumulated trash and debris removed, at least twice during the growing season.
- Eroded or barren spots should be reseeded immediately after inspection to prevent additional erosion and accumulation of sediment.
- Deep tilling can be used to break up a clogged surface area.
- Sediment should be removed from the basin as necessary. Removal procedures should not take place until the floor of the basin is thoroughly dry.

Inspections and Cleaning

- Infiltration basins should be inspected at least twice a year to ensure proper stabilization and function.

- Light equipment, which will not compact the underlying soil, should be used to remove the top layer.

D.2.2 Vegetated Areas Maintenance

Although not a structural component of the drainage system, the maintenance of vegetated areas may affect the functioning of the stormwater management system. This includes the health/density of vegetative cover and activities such as the application and disposal of lawn and garden care products, disposal of leaves and yard trimmings and proper aeration of soils.

- Inspect planted areas on a semi-annual basis and remove any litter.
- Maintain planted areas adjacent to pavement to prevent soil washout.
- Immediately clean any soil deposited on pavement.
- Re-seed bare areas; install appropriate erosion control measures when native soil is exposed or erosion channels are forming.
- Plant alternative mixture of grass species in the event of unsuccessful establishment.
- The grass vegetation should be cut to a height between three and four inches.
- Pesticide/Herbicide Usage – No pesticides are to be used unless a single spot treatment is required for a specific control application.
- Fertilizer usage should be avoided. If deemed necessary, slow release fertilizer should be used. Fertilizer may be used to begin the establishment of vegetation in bare or damaged areas, but should not be applied on a regular basis unless necessary.
- Annual application of compost amendments and aeration are recommended.

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Section E: Operations and Maintenance Plan Summary

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E Operations and Maintenance Plan Summary

This Operation and Maintenance Plan has been prepared in accordance with the Stormwater Management Policy developed by the DEP. It specifies operational practices and drainage system maintenance requirements for the Solar Array located at 0 Route 25 in Wareham, Massachusetts. Requirements should be adjusted by the site manager as necessary to ensure successful functioning of system components.

E.1 Routine Maintenance Checklists

Routine required maintenance is described in Sections A – D. The following checklists are to be used by the property manager to implement and document the required maintenance and inspection tasks.

E.2 Reporting and Documentation

The site supervisor shall be responsible for ensuring that the scheduled tasks as described in this plan are appropriately completed and recorded in the Maintenance Log. Accurate records of all inspections, routine maintenance and repairs shall be documented and these records shall be available for inspection by members of the Wareham Conservation Commission, or their designated agent, upon request.

The Maintenance Log shall:

- Document the completion of required maintenance tasks.
- Identify the person responsible for the completion of tasks.
- Identify any outstanding problems, malfunctions or inconsistencies identified during the course of routine maintenance.
- Document specific repairs or replacements.

E.3 Construction Practices Maintenance/ Evaluation Checklist

Proposed Large-Scale Ground-Mounted Solar Photovoltaic Installation Wareham, MA

Best Management Practice	Inspection Frequency	Date Inspected	Inspector Initials	Minimum Maintenance and Key Items to Check	Cleaning or Repair Needed ☐Yes/No (List Items)	Date of Cleaning or Repair	Performed by:
Straw Wattles/ Silt Fencing	Weekly and after any rainfall			Sediment build up, broken bales or stakes			
Gravel Construction Entrance	Weekly and after any rainfall			Filled voids, runoff/sediments into street			
Diversion Channels	Weekly and after any rainfall			Maintained, moved as necessary to correct locations, Check for erosion or breakout			
Temporary Sedimentation Basins	Weekly and after any rainfall			Cracking, erosion, breakout, sediment buildup, contaminants			

Stormwater Control Manager (TBD): _____

E.4 Long-term Maintenance/Evaluation Checklist

Proposed Large-Scale Ground-Mounted Solar Photovoltaic Installation Wareham, MA

Best Management Practice	Minimum Maintenance and Key Items to Check	Inspection Frequency	Date Inspected	Inspector Initials	Cleaning Frequency	Cleaning or Repair Needed ☐Yes/No	Date of Cleaning or Repair	Performed by:
Outfall Structures	Remove debris and excess vegetation, replace any dislodged riprap	1X per year			1X per year			
Infiltration Basins	Remove sediment 1X per year or if >6 inches	1X per year			1X per year			

Stormwater Control Manager: _____

E.5 Maintenance Checklists and Device Location Maps

These checklists are provided for the maintenance crew to photocopy and use when conducting inspections and cleaning activities to the stormwater management systems.

Outlet Control Structures – Inspect 4 times per year, clean when sediment depth >6 inches or at least once per year.

OCS	Inspected (Y/N)	Sediment Depth (inches)	Cleaning needed (Y/N)	Date Cleaned	Comments (Trash, Oil, Pet waste, Lawn Debris, Damage)
101				/ /	
201				/ /	

Outfalls – Inspect 4 times per year, replace any dislodged rip-rap, remove excess vegetation, remove any debris.

Outfall	Inspected (Y/N)	Sediment Depth (inches)	Cleaning needed (Y/N)	Date Cleaned	Comments (Trash, Oil, Pet waste, Lawn Debris, Damage)
102				/ /	
103				/ /	

Infiltration/Detention Basins – Inspect once per year, remove sediment if more than 6 inches has accumulated in sediment forebay or sediment collection row.

Basin	Inspected (Y/N)	Sediment Depth (inches)	Cleaning needed (Y/N)	Date Cleaned	Comments (Trash, Oil, Pet waste, Lawn Debris, Damage)
IB 1				/ /	
IB 2				/ /	
				/ /	