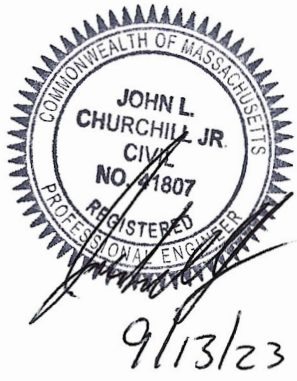
DRAINAGE CALCULATIONS
&
STORMWATER REPORT

at

386 Main Street
Wareham, MA

SEPTEMBER 13, 2023



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386 MAIN STREET

WAREHAM, MA

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1. Project Description

Narrative

This project consists of the construction of 3 residential duplex structures (6 new units), driveway and parking areas, and associated utilities. The site has been developed to meet the Massachusetts Stormwater Standards to the maximum extent practicable and applicable Town of Wareham regulations pertaining to Stormwater Management.

Existing Conditions

The project site is known as 386 Main Street and is shown as Lot 1079 on Wareham Assessors Map 61. The lot consists of 41,005 square feet of land and is currently developed with a residential 4-family structure. The entire property generally slopes towards Main Street where there is an existing retaining wall dividing the front of the property from the roadway.

The property is abutted by residential properties to the northwest and southeast and is abutted by Main Street to the northeast and a right of way leading to High Street to the southwest.

Proposed Conditions

The proposed project includes the construction of 3 duplex structures with associated parking, grading, and utilities. A paved driveway will be provided within the right of way adject to the property to allow access off High Street. Grading has been designed to minimize significant cuts & fills across the site. Utilities will consist of connections to existing water, sewer, gas, and electric facilities located within Main Street & High Street. Stormwater runoff will be directed to a subsurface drainage system. The proposed development will not disturb any new areas that have not already been previously altered.

Soil Description

Existing soil classifications and hydrologic soil groups for the site were obtained from the USDA Soil Conservation Service, Soil Survey of Plymouth County, Massachusetts & The Web Soil Survey. The soil types found within the limits of the drainage analysis are classified as the following:

1.) Canton – Urban Land Complex, 0 to 8 percent slopes (628B)

Canton – Urban Land Complex is considered well drained, exhibits a hydrological classification group “A”, and is the primary soil type over the project site.

2. Hydrologic Analysis & Stormwater Management

Methodology

Stormwater runoff was evaluated for the 2-year, 10-year, 25-year, and 100-year, Type III, 24-hour storm for both pre-development and post-development conditions. Pre-development and post-development conditions were modeled using HydroCAD software, which combines USDA Soil Conservation Service hydrology and hydraulic techniques (commonly known as SCS TR-55 and TR-20) to generate hydrographs (calculations are provided in the supplemental section of this report). The rainfall amounts used for calculating runoff for the 2-year, 10-year, 25-year and 100-year storm events were obtained from the NOAA Atlas 14 Volume 10 Frequency Estimates.

The drainage calculation provided at the end of this report identifies on-site and off-site design points for both existing and proposed conditions. Under existing conditions, runoff is entirely directed offsite. Under proposed conditions, a majority of the runoff is captured and infiltrated onsite, leaving only a small portion runoff directed offsite. **Table 1** compares the pre-development and post-development peak runoff rates and volumes for the 2-year, 10-year, 25-year, and 100-year storm events at two separate design points for the Type III, 24-hour storm events. The design points were evaluated to ensure post-development peak runoff rates and volumes do not exceed pre-development amounts.

Pre-Development Drainage Conditions

The site was modeled into two sub-catchment areas under existing conditions. Subcatchment Area DA-1 comprises the northeast portion of the site and contributes runoff to Main Street. Subcatchment Area DA-2 comprises the southwest portion of the site and the 20’ right of way and contributes runoff to the rear of the lot heading towards High Street.

Refer to the Existing Drainage Areas Plan prepared by this office at the end of this report.

Post-Development Drainage Conditions

Post-development drainage conditions and patterns were maintained to the maximum extent possible. Subcatchment Area DA-1 comprises the outer perimeter of the development area in the northeast portion of the site which contributes runoff to Main Street. Subcatchment Area DA-2 comprises the outer perimeter of the development area in the southwest portion of the site and the section of the right of way that does not get contaminated by the drainage system which contributes runoff to the rear of the lot heading towards High Street. Subcatchment Area DA-3 comprises most of the driveway, structures, and area within the middle of the lot which contributes runoff to 1P, a subsurface drainage system located towards the front of the property and is directly recharged. Subcatchment Area DA-4 comprises the roof runoff of one duplex which contributes to 2P, a roof drywell located at the rear of the property. Subcatchment Area DA-5 comprises the roof runoff of one duplex and the section of pavement within the right of way that does get contained by the drainage system which contributes to 3P, a subsurface drainage system located towards the rear of the property.

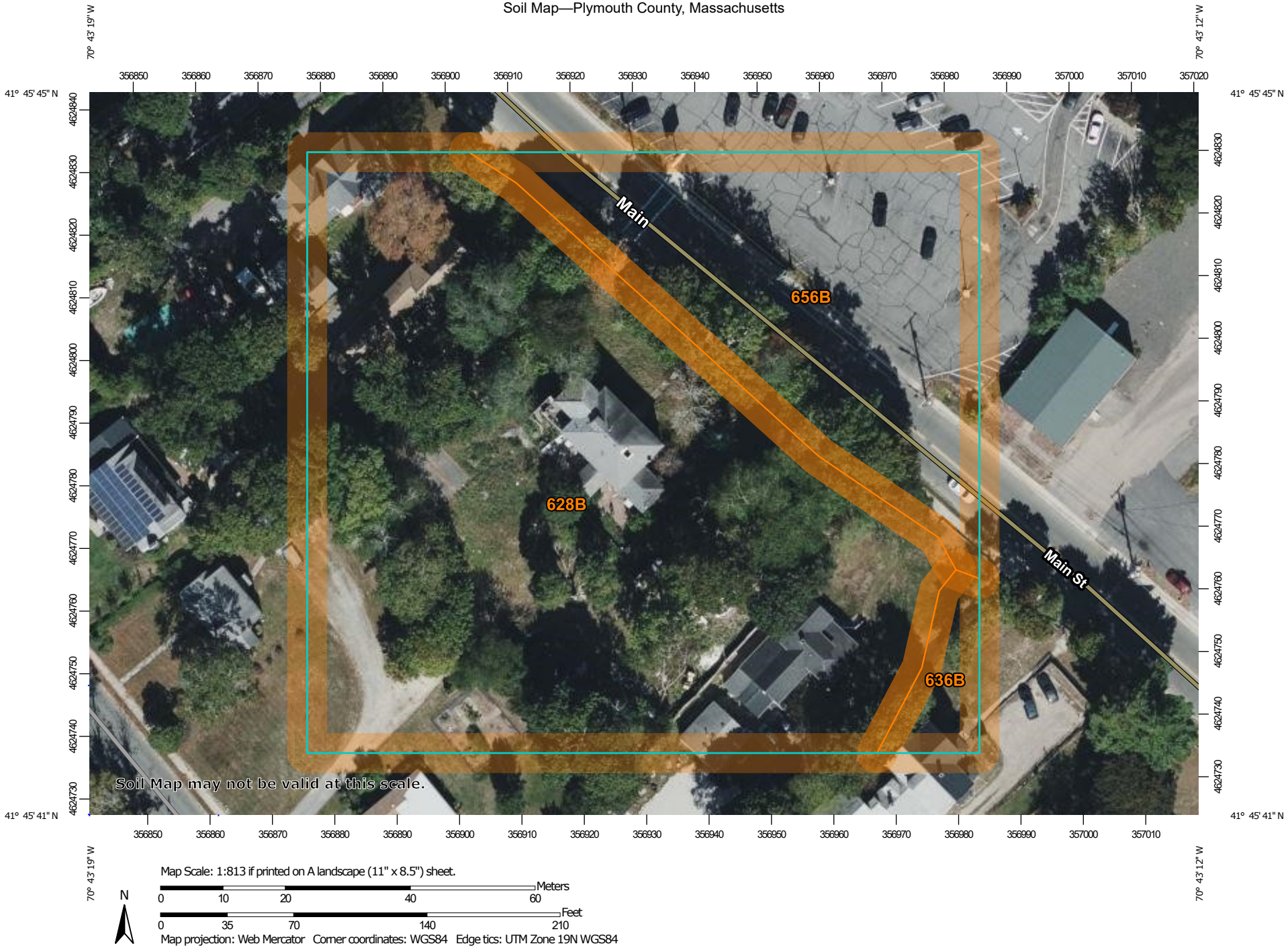
Refer to the Proposed Drainage Areas Plan prepared by this office at the end of this report.

Table 1 compares below the pre-development and post-development peak runoff rates and volumes for the 2-year, 10-year, 25-year, and 100-year storm events at the offsite design points.

	Ex. Flow (cfs)	Prop. Flow (cfs)	Ex. Vol. (af)	Prop. Vol. (cf)
DA-1				
2-Yr Event	0.00	0.00	0.003	0.001
10-Yr Event	0.10	0.05	0.021	0.010
25-Yr Event	0.29	0.14	0.041	0.019
100-Yr Event	0.85	0.80	0.079	0.058
DA-2				
2-Yr Event	0.00	*0.01	0.003	0.003
10-Yr Event	0.09	0.09	0.016	0.010
25-Yr Event	0.24	0.18	0.028	0.017
100-Yr Event	0.58	0.35	0.053	0.029

Table 1 – Comparison of Off-site Stormwater Flows and Volumes

*Minor increase in flow at 2-year storm event due to smaller contributing areas under proposed conditions.



Map Scale: 1:813 if printed on A landscape (11" x 8.5") sheet.

N

Meters: 0 10 20 40 60

Feet: 0 35 70 140 210

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 19N WGS84

MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

Special Point Features

- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Plymouth County, Massachusetts
Survey Area Data: Version 15, Sep 9, 2022

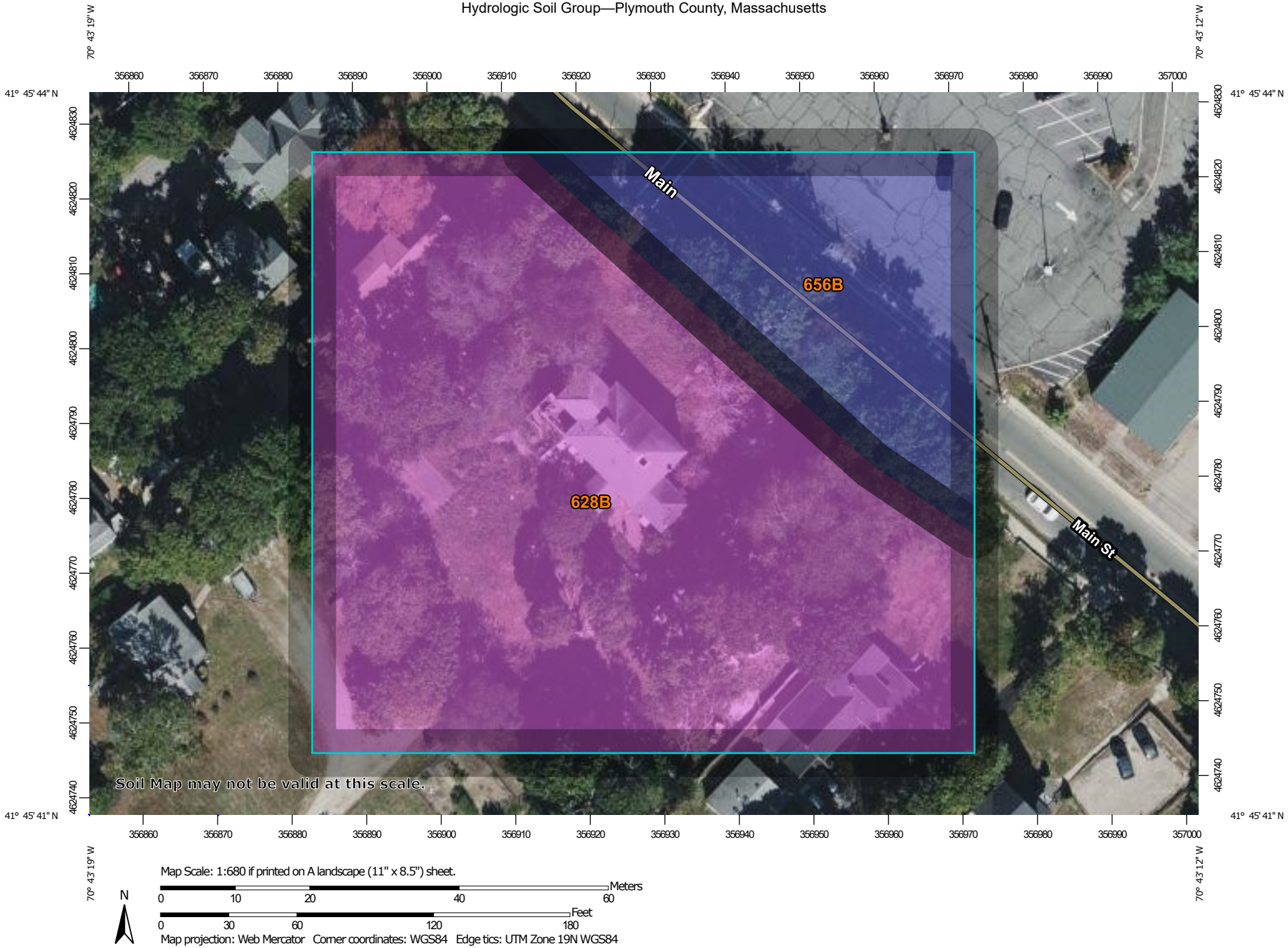
Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 25, 2020—Oct 9, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
628B	Canton - Urban land complex, 0 to 8 percent slopes	1.8	70.1%
636B	Montauk-Urban land complex, 0 to 8 percent slopes	0.1	2.8%
656B	Udorthents - Urban land complex, 0 to 8 percent slopes	0.7	27.1%
Totals for Area of Interest		**2.6**	**100.0%**



Map Scale: 1:680 if printed on A landscape (11" x 8.5") sheet.

N

Meters: 0 10 20 40 60

Feet: 0 30 60 120 180

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 19N WGS84

MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Rating Polygons

A
A/D
B
B/D
C
C/D
D
Not rated or not available

Soil Rating Lines

A
A/D
B
B/D
C
C/D
D
Not rated or not available

Soil Rating Points

A
A/D
B
B/D
C
C/D
D
Not rated or not available

Water Features

Streams and Canals

Transportation

Rails
Interstate Highways
US Routes
Major Roads
Local Roads

Background

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

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Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

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Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
628B	Canton - Urban land complex, 0 to 8 percent slopes	A	1.4	77.5%
656B	Udorthents - Urban land complex, 0 to 8 percent slopes	B	0.4	22.5%
Totals for Area of Interest			**1.8**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

NOAA Atlas 14, Volume 10, Version 3
Location name: Wareham, Massachusetts, USA*
Latitude: 41.762°, Longitude: -70.7211°
Elevation: 32.12 ft**
* source: ESRI Maps
** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)[1]

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	**0.294** (0.237-0.360)	**0.365** (0.294-0.447)	**0.480** (0.385-0.589)	**0.576** (0.460-0.710)	**0.707** (0.549-0.903)	**0.805** (0.613-1.05)	**0.909** (0.677-1.22)	**1.03** (0.724-1.39)	**1.22** (0.823-1.68)	**1.37** (0.909-1.92)
10-min	**0.417** (0.336-0.510)	**0.517** (0.416-0.633)	**0.680** (0.546-0.835)	**0.815** (0.651-1.00)	**1.00** (0.777-1.28)	**1.14** (0.869-1.48)	**1.29** (0.959-1.73)	**1.46** (1.03-1.97)	**1.72** (1.17-2.38)	**1.94** (1.29-2.72)
15-min	**0.491** (0.396-0.601)	**0.608** (0.490-0.745)	**0.800** (0.642-0.982)	**0.959** (0.766-1.18)	**1.18** (0.915-1.51)	**1.34** (1.02-1.74)	**1.52** (1.13-2.03)	**1.72** (1.21-2.32)	**2.03** (1.37-2.80)	**2.29** (1.52-3.20)
30-min	**0.705** (0.569-0.863)	**0.873** (0.703-1.07)	**1.15** (0.922-1.41)	**1.38** (1.10-1.70)	**1.69** (1.31-2.16)	**1.92** (1.47-2.50)	**2.17** (1.62-2.91)	**2.47** (1.73-3.32)	**2.91** (1.97-4.01)	**3.28** (2.17-4.59)
60-min	**0.920** (0.742-1.13)	**1.14** (0.917-1.40)	**1.50** (1.20-1.84)	**1.79** (1.43-2.21)	**2.20** (1.71-2.81)	**2.51** (1.91-3.26)	**2.83** (2.11-3.79)	**3.22** (2.26-4.33)	**3.79** (2.56-5.23)	**4.27** (2.83-5.97)
2-hr	**1.24** (1.00-1.50)	**1.54** (1.25-1.87)	**2.03** (1.64-2.48)	**2.44** (1.96-2.99)	**3.00** (2.35-3.82)	**3.42** (2.63-4.42)	**3.87** (2.91-5.17)	**4.42** (3.12-5.89)	**5.24** (3.58-7.17)	**5.95** (3.97-8.25)
3-hr	**1.46** (1.19-1.77)	**1.81** (1.47-2.19)	**2.38** (1.93-2.89)	**2.85** (2.30-3.48)	**3.51** (2.76-4.44)	**3.99** (3.09-5.14)	**4.51** (3.41-6.00)	**5.15** (3.66-6.84)	**6.12** (4.19-8.32)	**6.94** (4.66-9.57)
6-hr	**1.90** (1.56-2.29)	**2.32** (1.90-2.80)	**3.01** (2.46-3.63)	**3.58** (2.91-4.34)	**4.37** (3.45-5.47)	**4.95** (3.85-6.31)	**5.58** (4.24-7.33)	**6.32** (4.53-8.33)	**7.45** (5.15-10.0)	**8.40** (5.69-11.5)
12-hr	**2.41** (1.99-2.88)	**2.88** (2.38-3.45)	**3.66** (3.00-4.38)	**4.30** (3.51-5.17)	**5.18** (4.11-6.42)	**5.84** (4.56-7.35)	**6.53** (4.97-8.45)	**7.33** (5.30-9.56)	**8.49** (5.92-11.3)	**9.44** (6.44-12.8)
24-hr	**2.90** (2.41-3.45)	**3.44** (2.86-4.09)	**4.32** (3.57-5.14)	**5.05** (4.15-6.03)	**6.05** (4.84-7.44)	**6.81** (5.34-8.48)	**7.60** (5.80-9.70)	**8.47** (6.17-10.9)	**9.70** (6.83-12.8)	**10.7** (7.37-14.4)
2-day	**3.34** (2.79-3.94)	**3.97** (3.32-4.69)	**5.01** (4.17-5.92)	**5.86** (4.86-6.96)	**7.04** (5.67-8.59)	**7.93** (6.27-9.81)	**8.86** (6.82-11.2)	**9.89** (7.26-12.7)	**11.3** (8.04-14.9)	**12.5** (8.69-16.6)
3-day	**3.67** (3.08-4.31)	**4.33** (3.64-5.09)	**5.43** (4.54-6.39)	**6.33** (5.27-7.48)	**7.58** (6.12-9.20)	**8.52** (6.76-10.5)	**9.49** (7.34-12.0)	**10.6** (7.80-13.5)	**12.1** (8.61-15.7)	**13.3** (9.27-17.5)
4-day	**3.96** (3.33-4.63)	**4.64** (3.90-5.44)	**5.76** (4.83-6.76)	**6.68** (5.58-7.87)	**7.96** (6.45-9.62)	**8.93** (7.10-10.9)	**9.93** (7.68-12.4)	**11.0** (8.15-14.0)	**12.5** (8.95-16.2)	**13.7** (9.60-18.0)
7-day	**4.72** (3.99-5.49)	**5.43** (4.59-6.33)	**6.59** (5.56-7.70)	**7.56** (6.34-8.85)	**8.88** (7.23-10.7)	**9.90** (7.91-12.0)	**10.9** (8.48-13.5)	**12.0** (8.96-15.1)	**13.5** (9.70-17.3)	**14.6** (10.3-19.0)
10-day	**5.42** (4.61-6.29)	**6.16** (5.23-7.15)	**7.36** (6.23-8.57)	**8.36** (7.04-9.76)	**9.73** (7.95-11.6)	**10.8** (8.64-13.0)	**11.9** (9.21-14.6)	**12.9** (9.68-16.2)	**14.3** (10.4-18.3)	**15.4** (10.9-20.0)
20-day	**7.52** (6.43-8.67)	**8.34** (7.13-9.62)	**9.68** (8.24-11.2)	**10.8** (9.14-12.5)	**12.3** (10.1-14.5)	**13.5** (10.9-16.1)	**14.7** (11.4-17.8)	**15.8** (11.9-19.6)	**17.1** (12.5-21.7)	**18.1** (12.9-23.3)
30-day	**9.28** (7.97-10.7)	**10.2** (8.73-11.7)	**11.6** (9.95-13.4)	**12.8** (10.9-14.8)	**14.5** (12.0-17.0)	**15.8** (12.8-18.7)	**17.1** (13.3-20.5)	**18.2** (13.8-22.5)	**19.6** (14.4-24.7)	**20.5** (14.7-26.2)
45-day	**11.5** (9.91-13.1)	**12.5** (10.7-14.3)	**14.1** (12.1-16.1)	**15.4** (13.2-17.7)	**17.3** (14.3-20.2)	**18.7** (15.2-22.0)	**20.1** (15.8-24.0)	**21.3** (16.3-26.1)	**22.7** (16.8-28.4)	**23.6** (17.0-30.0)
60-day	**13.4** (11.6-15.2)	**14.4** (12.5-16.5)	**16.2** (13.9-18.5)	**17.6** (15.1-20.2)	**19.6** (16.3-22.8)	**21.2** (17.2-24.9)	**22.7** (17.8-26.9)	**23.9** (18.3-29.2)	**25.3** (18.8-31.6)	**26.2** (19.0-33.2)

[1] Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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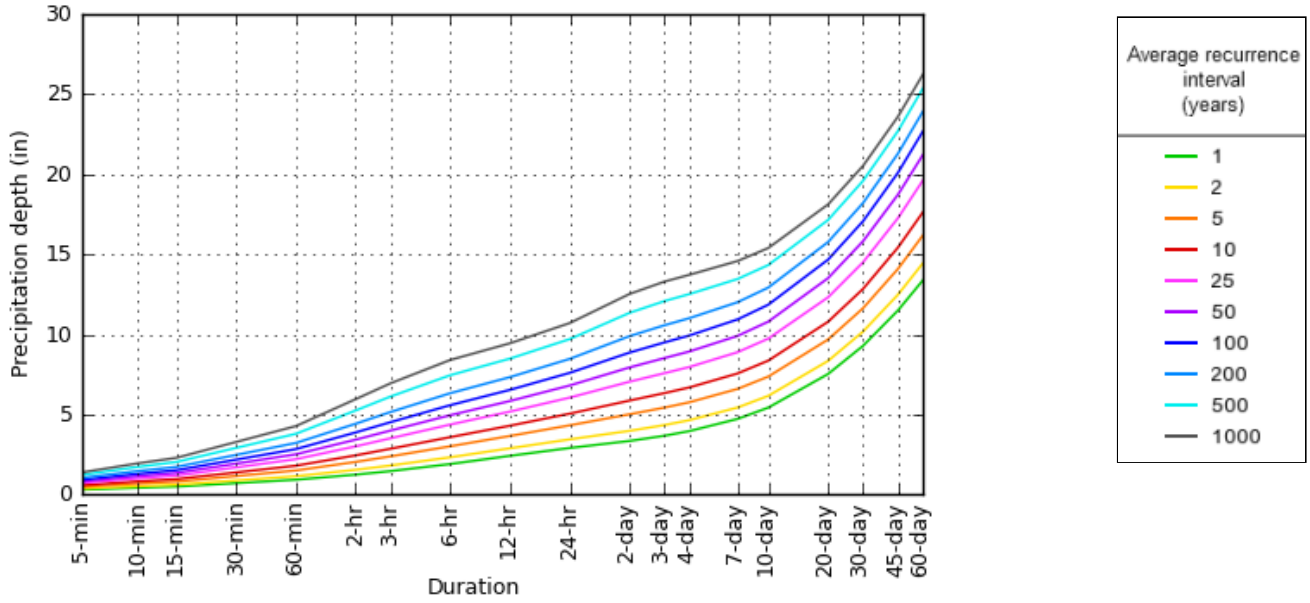
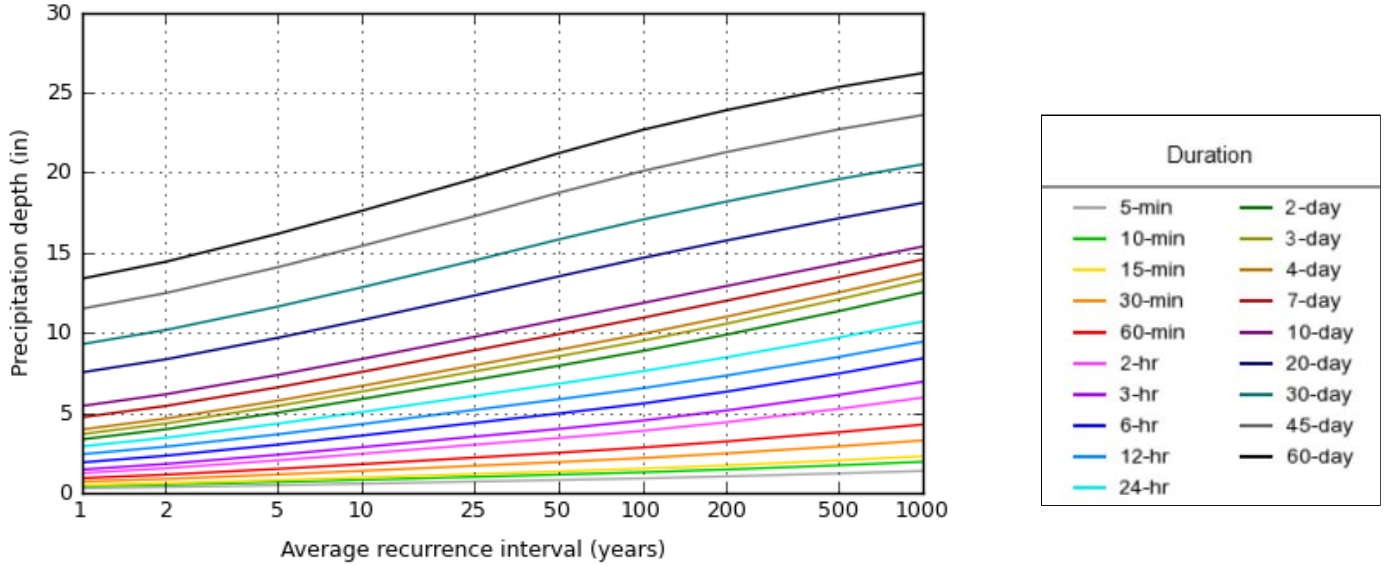
PF graphical

PDS-based depth-duration-frequency (DDF) curves
Latitude: 41.7620°, Longitude: -70.7211°





NOAA Atlas 14, Volume 10, Version 3

Created (GMT): Mon Nov 14 20:27:25 2022

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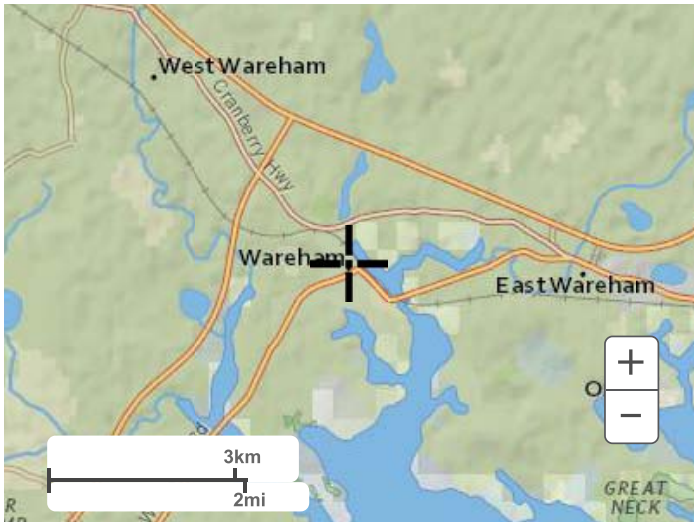
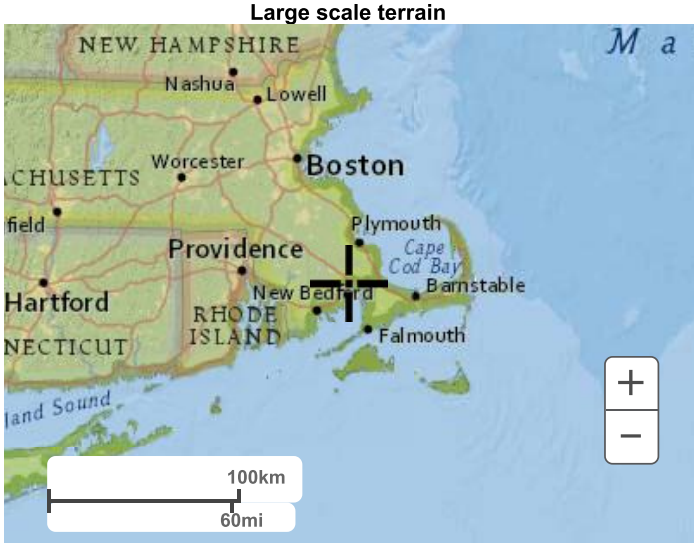
Maps & aerials

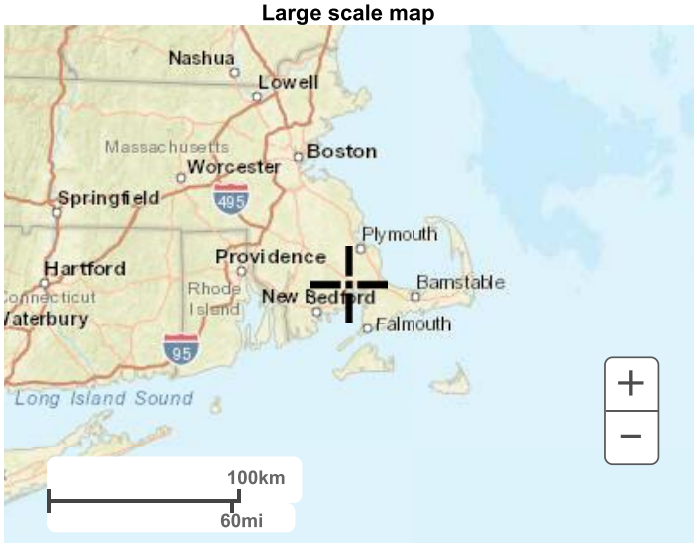
Small scale terrain



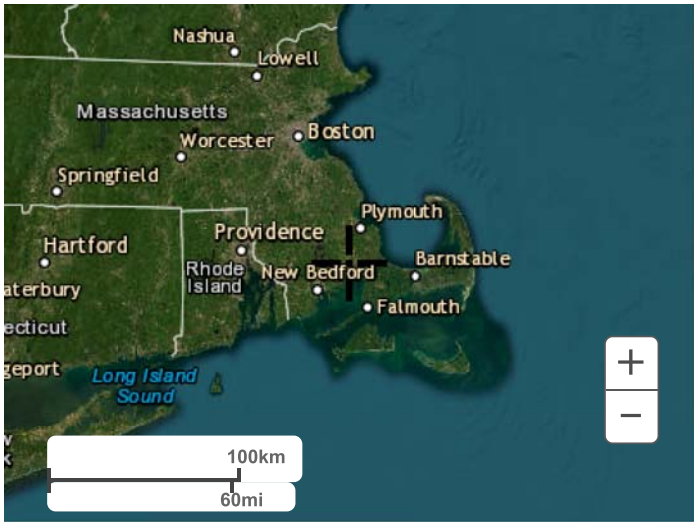
Large scale terrain



Large scale map



Large scale aerial



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US Department of Commerce
National Oceanic and Atmospheric Administration
National Weather Service
National Water Center
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

Disclaimer

GROUNDWATER RECHARGE VOLUME CALCULATIONS

386 MAIN STREET

WAREHAM, MASSACHUSETTS

Total Proposed Impervious Area

- Impervious Area = 20,489 s.f.

Recharge Factor

Hydrologic Group A Soils = 0.60 inches of runoff

Groundwater Recharge Volume Required

20,489 s.f. x (0.60 inches x 1/12) = **1,024** c.f. required

Groundwater Recharge Volume Provided in Leaching Pits

- The storage volume is **5,087** c.f. within leaching pits and void space in stone (refer to HydroCAD output of "Pond 3P" in drainage report)

Conclusion: Total recharge volume of **5,087** c.f. provided is greater than the required recharge volume of **1,024** c.f.; therefore **OK**.

WATER QUALITY VOLUME CALCULATIONS

386 MAIN STREET

WAREHAM, MASSACHUSETTS

Total Proposed Impervious Area

- Impervious Area (I) = 20,489 s.f.

Water Quality Volume (WQV) Required to be Treated (1" of runoff)

- WQV = 1.0" x I (s.f.)
- WQV = 1.0"/(12 in/ft) x 20,489 s.f. = **1,707 c.f.** required

Water Quality Volume Provided

Total recharge volume of **5,087** c.f. provided from proposed leaching basins and void space in stone (see Groundwater Recharge Volume Calculations – previous pages)

Conclusion: Proposed water quality volume of **5,087** c.f. provided is greater than **1,707** c.f. required; therefore OK.

INFILTRATION DRAIN-DOWN TIME CALCULATIONS

386 MAIN STREET

WAREHAM, MASSACHUSETTS

Pond 1P:

Maximum Drain Time = 72 hours

Provided Drain Time = Storage Volume / (K x Basin Bottom Area)

= 5,087 c.f. / [(2.41 in/hr) (1ft/12 inches) x 1,449 s.f.]

= **17.5 hours**, which is less than max. drain time of 72 hours, therefore OK.

OIL/GRIT SEPARATOR SIZING CALCULATIONS

386 MAIN STREET

WAREHAM, MASSACHUSETTS

Oil/Grit Separator- DA-3:

Contributing Impervious Area (I) = 16,576 s.f. or 0.381 acres

Sediment Chamber Vol. Req'd = 400 c.f. per Impervious Acre (I)
= 400 c.f. x 0.381 = 152.4 c.f.
= 152.4 c.f. x 7.48 gal/c.f. = **1,140** gallons required

Sediment Chamber Vol. Prov'd
2,500 Gallon tank is 5.83' wide inside and 11.33' long inside, Eff. height is 5.00',
Therefore, utilize 5.25' for first chamber = 5.83' x 5.25' x 5.0' = 153.0 c.f.
= 153.0 c.f. x 7.48 gal/c.f. = **1,145** gallons prov'd

Conclusion: Proposed **1,145** gallons provided is greater than **1,140** gallons required; therefore OK.

Oil/Grit Separator- DA-5:

Contributing Impervious Area (I) = 1,875 s.f. or 0.043 acres

Sediment Chamber Vol. Req'd = 400 c.f. per Impervious Acre (I)
= 400 c.f. x 0.043 = 17.2 c.f.
= 17.2 c.f. x 7.48 gal/c.f. = **129** gallons required

Sediment Chamber Vol. Prov'd
1,500 Gallon tank is 5.0' wide inside and 9.83' long inside, Eff. height is 4.00',
Therefore, utilize 3.00' for first chamber = 5.0' x 3.00' x 4.0' = 60 c.f.
= 60 c.f. x 7.48 gal/c.f. = **449** gallons prov'd

Conclusion: Proposed **449** gallons provided is greater than **129** gallons required; therefore OK.

TSS REMOVAL CALCULATIONS

INSTRUCTIONS:
1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location: 386 Main Street (Pre-Treatment)

TSS Removal Calculation Worksheet

B BMP[1]	C TSS Removal Rate[1]	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
Deep Sump and Hooded Catch Basin	**0.25**	**1.00**	**0.25**	**0.75**
Oil Grit Separator	**0.25**	**0.75**	**0.19**	**0.56**
	0.00	**0.56**	**0.00**	**0.56**
	0.00	**0.56**	**0.00**	**0.56**
	0.00	**0.56**	**0.00**	**0.56**

Total TSS Removal = **44%**

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project:
Prepared By: **JC Engineering**
Date: **9/13/2023**

*Equals remaining load from previous BMP (E) which enters the BMP

Non-automated TSS Calculation Sheet must be used if Proprietary BMP Proposed

1. From MassDEP Stormwater Handbook Vol. 1

INSTRUCTIONS:
1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location: 386 Main Street

TSS Removal Calculation Worksheet

B BMP[1]	C TSS Removal Rate[1]	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
Deep Sump and Hooded Catch Basin	**0.25**	**1.00**	**0.25**	**0.75**
Oil Grit Separator	**0.25**	**0.75**	**0.19**	**0.56**
Subsurface Infiltration Structure	**0.80**	**0.56**	**0.45**	**0.11**
	0.00	**0.11**	**0.00**	**0.11**
	0.00	**0.11**	**0.00**	**0.11**

Total TSS Removal = **89%**

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project:
Prepared By: **JC Engineering**
Date: **9/13/2023**

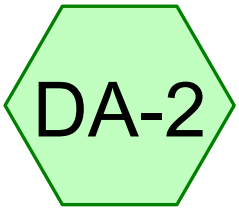
*Equals remaining load from previous BMP (E) which enters the BMP

Non-automated TSS Calculation Sheet must be used if Proprietary BMP Proposed

1. From MassDEP Stormwater Handbook Vol. 1

EXISTING CONDITIONS DRAINAGE CALCULATIONS

Offsite Runoff to Main Street



Offstie Runoff to Rear of Property

Subcat Reach Pond Link

Routing Diagram for Existing Conditions
Prepared by JC Engineering Inc., Printed 9/13/2023

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.955	39	>75% Grass cover, Good, HSG A (DA-1, DA-2)
0.035	72	Dirt roads, HSG A (DA-2)
0.129	98	Unconnected pavement, HSG A (DA-1, DA-2)
1.120	**47**	**TOTAL AREA**

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
1.120	HSG A	DA-1, DA-2
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.000	Other	
1.120		**TOTAL AREA**

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.955	0.000	0.000	0.000	0.000	0.955	>75% Grass cover, Good	DA-1, DA-2
0.035	0.000	0.000	0.000	0.000	0.035	Dirt roads	DA-2
0.129	0.000	0.000	0.000	0.000	0.129	Unconnected pavement	DA-1, DA-2
1.120	**0.000**	**0.000**	**0.000**	**0.000**	**1.120**	**TOTAL AREA**	

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment DA-1: Offsite Runoff to Runoff Area=30,635 sf 12.37% Impervious Runoff Depth>0.04"
Flow Length=165' Tc=5.6 min UI Adjusted CN=43 Runoff=0.00 cfs 0.003 af

Subcatchment DA-2: Offstie Runoff to Rear Runoff Area=18,143 sf 10.20% Impervious Runoff Depth>0.07"
Flow Length=235' Tc=7.0 min UI Adjusted CN=45 Runoff=0.00 cfs 0.003 af

Total Runoff Area = 1.120 ac Runoff Volume = 0.005 af Average Runoff Depth = 0.06"
88.44% Pervious = 0.990 ac 11.56% Impervious = 0.129 ac

Summary for Subcatchment DA-1: Offsite Runoff to Main Street

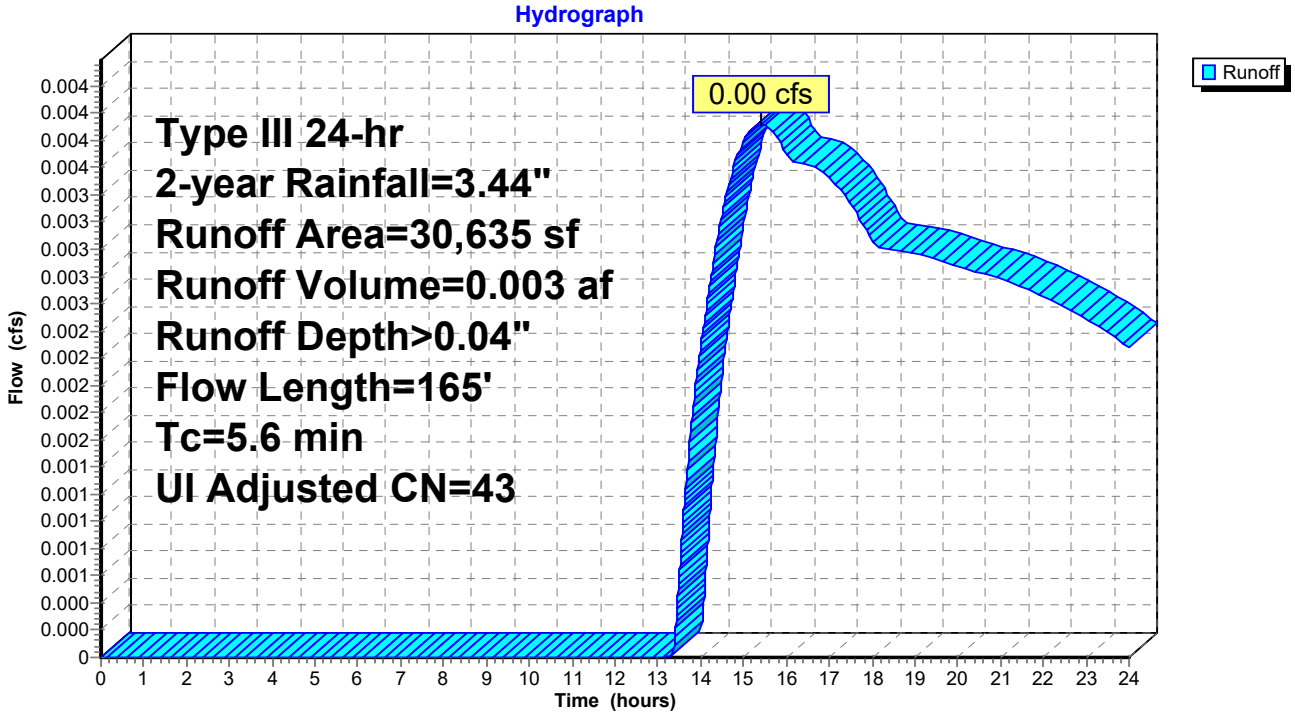
Runoff = 0.00 cfs @ 15.40 hrs, Volume= 0.003 af, Depth> 0.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.44"

Area (sf)	CN	Adj	Description
3,789	98		Unconnected pavement, HSG A
26,846	39		>75% Grass cover, Good, HSG A
30,635	46	43	Weighted Average, UI Adjusted
26,846			87.63% Pervious Area
3,789			12.37% Impervious Area
3,789			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.6	50	0.0300	0.18		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
1.0	115	0.0800	1.98		**Shallow Concentrated Flow, B-C** Short Grass Pasture Kv= 7.0 fps
5.6	165	Total			

Subcatchment DA-1: Offsite Runoff to Main Street



Summary for Subcatchment DA-2: Offstie Runoff to Rear of Property

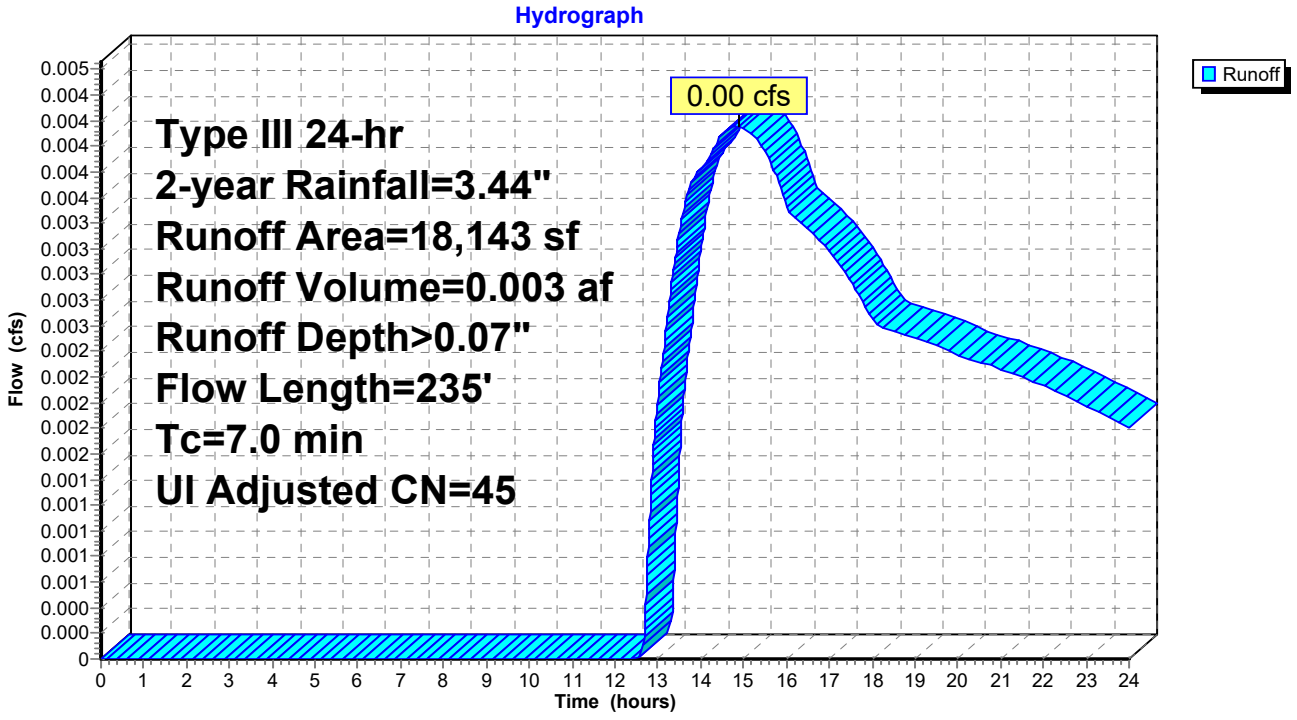
Runoff = 0.00 cfs @ 14.88 hrs, Volume= 0.003 af, Depth> 0.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.44"

Area (sf)	CN	Adj	Description
1,850	98		Unconnected pavement, HSG A
14,767	39		>75% Grass cover, Good, HSG A
1,526	72		Dirt roads, HSG A
18,143	48	45	Weighted Average, UI Adjusted
16,293			89.80% Pervious Area
1,850			10.20% Impervious Area
1,850			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.9	50	0.0160	0.14		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.9	125	0.0220	2.39		**Shallow Concentrated Flow, B-C** Unpaved Kv= 16.1 fps
0.2	60	0.0470	4.40		**Shallow Concentrated Flow, C-D** Paved Kv= 20.3 fps
7.0	235	Total			

Subcatchment DA-2: Offstie Runoff to Rear of Property



Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment DA-1: Offsite Runoff to Runoff Area=30,635 sf 12.37% Impervious Runoff Depth>0.37"
Flow Length=165' Tc=5.6 min UI Adjusted CN=43 Runoff=0.10 cfs 0.021 af

Subcatchment DA-2: Offstie Runoff to Rear Runoff Area=18,143 sf 10.20% Impervious Runoff Depth>0.46"
Flow Length=235' Tc=7.0 min UI Adjusted CN=45 Runoff=0.09 cfs 0.016 af

Total Runoff Area = 1.120 ac Runoff Volume = 0.037 af Average Runoff Depth = 0.40"
88.44% Pervious = 0.990 ac 11.56% Impervious = 0.129 ac

Summary for Subcatchment DA-1: Offsite Runoff to Main Street

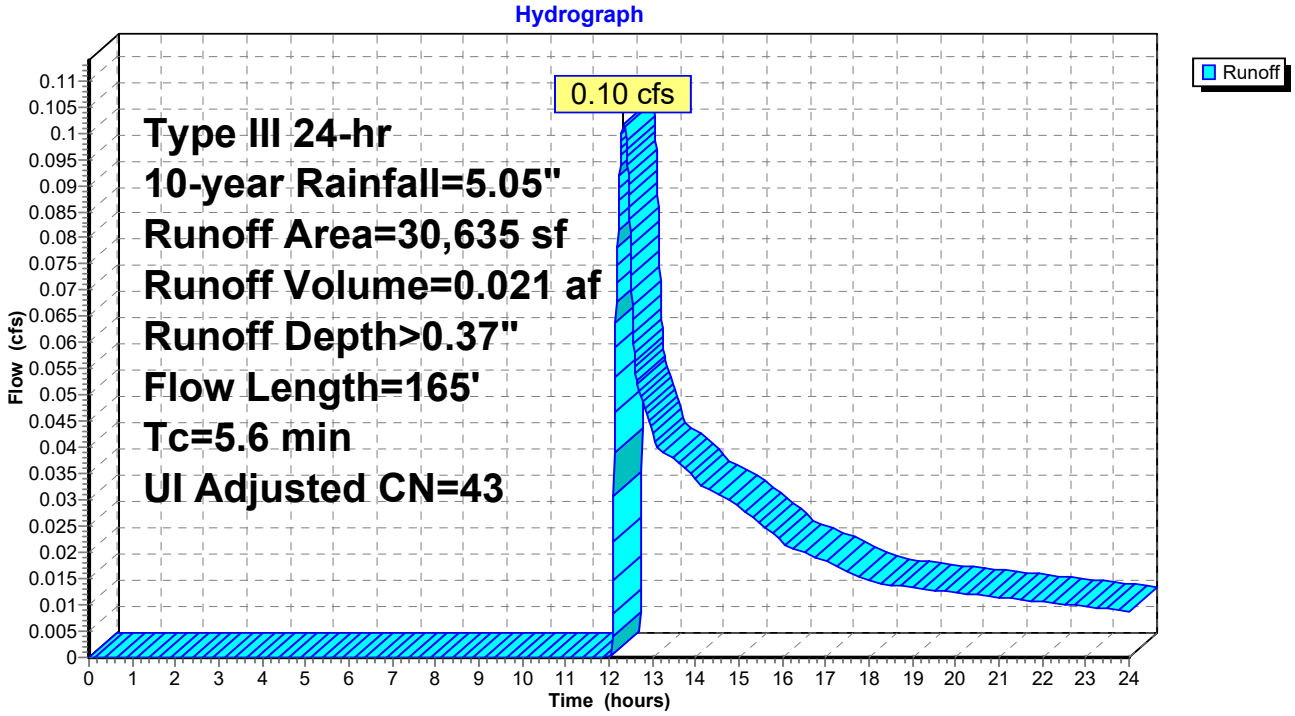
Runoff = 0.10 cfs @ 12.34 hrs, Volume= 0.021 af, Depth> 0.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.05"

Area (sf)	CN	Adj	Description
3,789	98		Unconnected pavement, HSG A
26,846	39		>75% Grass cover, Good, HSG A
30,635	46	43	Weighted Average, UI Adjusted
26,846			87.63% Pervious Area
3,789			12.37% Impervious Area
3,789			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.6	50	0.0300	0.18		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
1.0	115	0.0800	1.98		**Shallow Concentrated Flow, B-C** Short Grass Pasture Kv= 7.0 fps
5.6	165	Total			

Subcatchment DA-1: Offsite Runoff to Main Street



Summary for Subcatchment DA-2: Offstie Runoff to Rear of Property

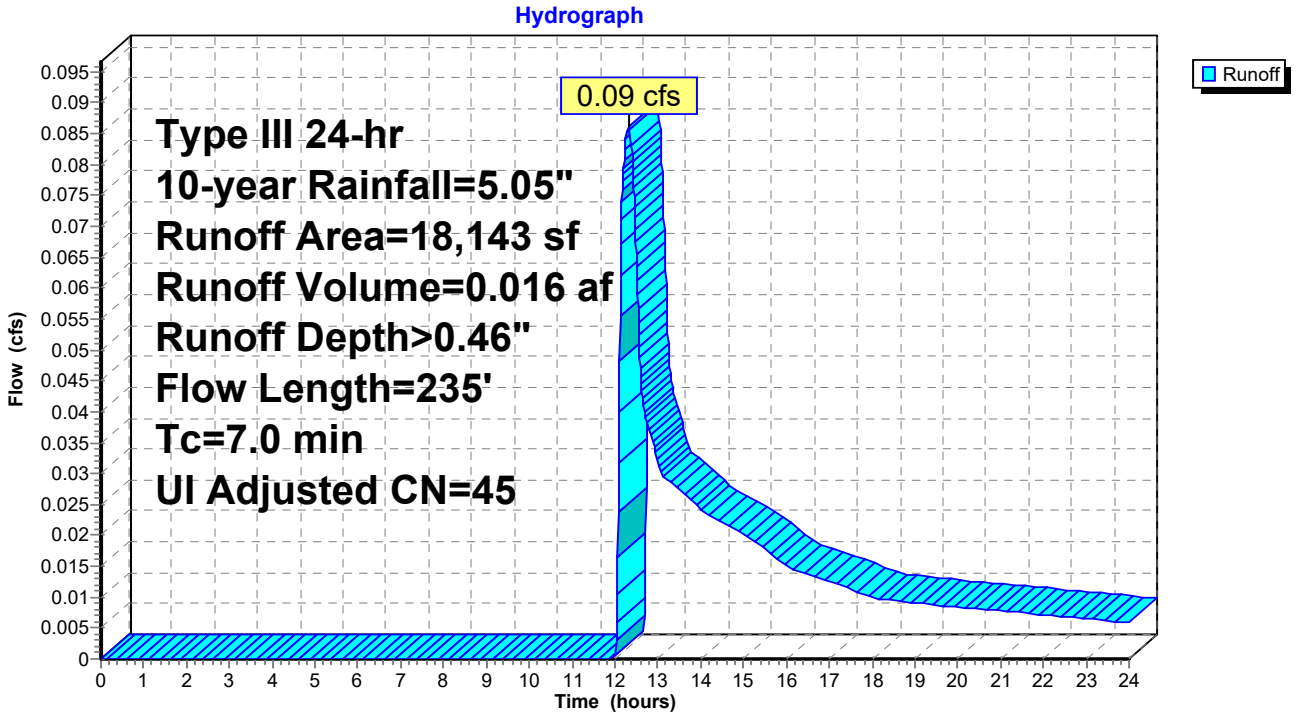
Runoff = 0.09 cfs @ 12.31 hrs, Volume= 0.016 af, Depth> 0.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.05"

Area (sf)	CN	Adj	Description
1,850	98		Unconnected pavement, HSG A
14,767	39		>75% Grass cover, Good, HSG A
1,526	72		Dirt roads, HSG A
18,143	48	45	Weighted Average, UI Adjusted
16,293			89.80% Pervious Area
1,850			10.20% Impervious Area
1,850			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.9	50	0.0160	0.14		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.9	125	0.0220	2.39		**Shallow Concentrated Flow, B-C** Unpaved Kv= 16.1 fps
0.2	60	0.0470	4.40		**Shallow Concentrated Flow, C-D** Paved Kv= 20.3 fps
7.0	235	Total			

Subcatchment DA-2: Offstie Runoff to Rear of Property



Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment DA-1: Offsite Runoff to Runoff Area=30,635 sf 12.37% Impervious Runoff Depth>0.69"
Flow Length=165' Tc=5.6 min UI Adjusted CN=43 Runoff=0.29 cfs 0.041 af

Subcatchment DA-2: Offstie Runoff to Rear Runoff Area=18,143 sf 10.20% Impervious Runoff Depth>0.82"
Flow Length=235' Tc=7.0 min UI Adjusted CN=45 Runoff=0.24 cfs 0.028 af

Total Runoff Area = 1.120 ac Runoff Volume = 0.069 af Average Runoff Depth = 0.74"
88.44% Pervious = 0.990 ac 11.56% Impervious = 0.129 ac

Summary for Subcatchment DA-1: Offsite Runoff to Main Street

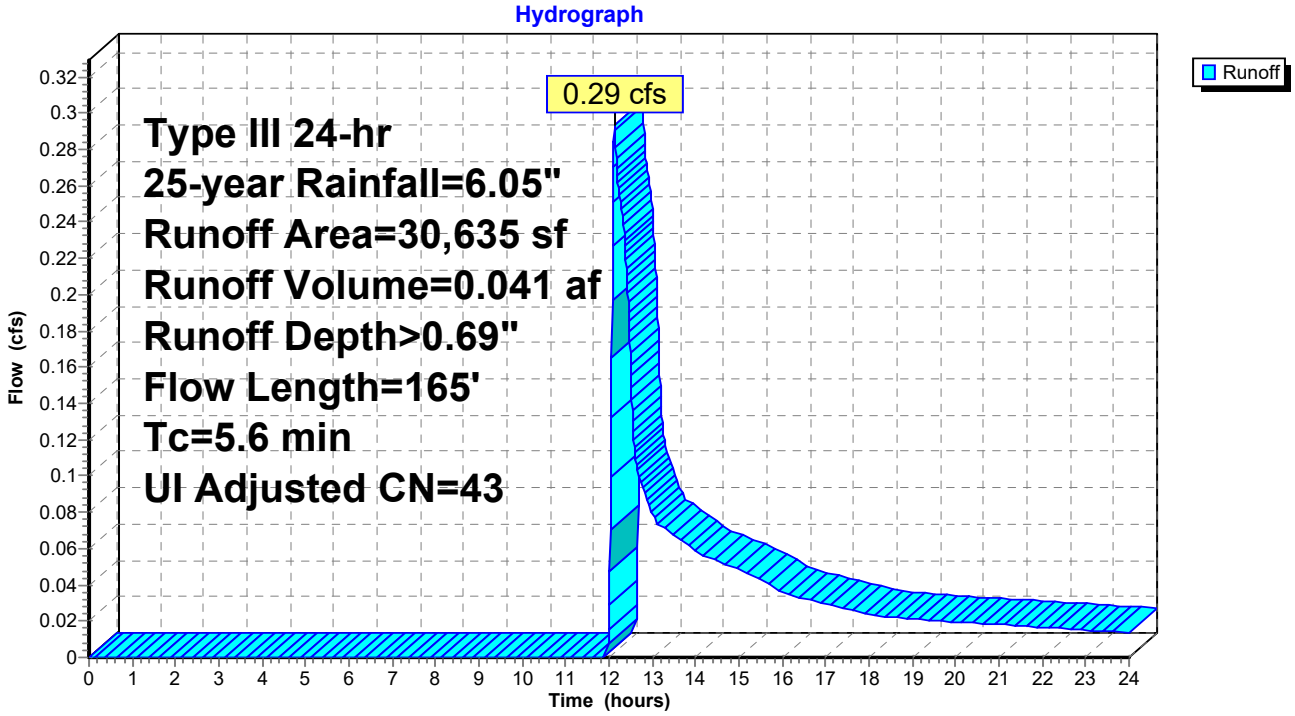
Runoff = 0.29 cfs @ 12.13 hrs, Volume= 0.041 af, Depth> 0.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-year Rainfall=6.05"

Area (sf)	CN	Adj	Description
3,789	98		Unconnected pavement, HSG A
26,846	39		>75% Grass cover, Good, HSG A
30,635	46	43	Weighted Average, UI Adjusted
26,846			87.63% Pervious Area
3,789			12.37% Impervious Area
3,789			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.6	50	0.0300	0.18		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
1.0	115	0.0800	1.98		**Shallow Concentrated Flow, B-C** Short Grass Pasture Kv= 7.0 fps
5.6	165	Total			

Subcatchment DA-1: Offsite Runoff to Main Street



Summary for Subcatchment DA-2: Offstie Runoff to Rear of Property

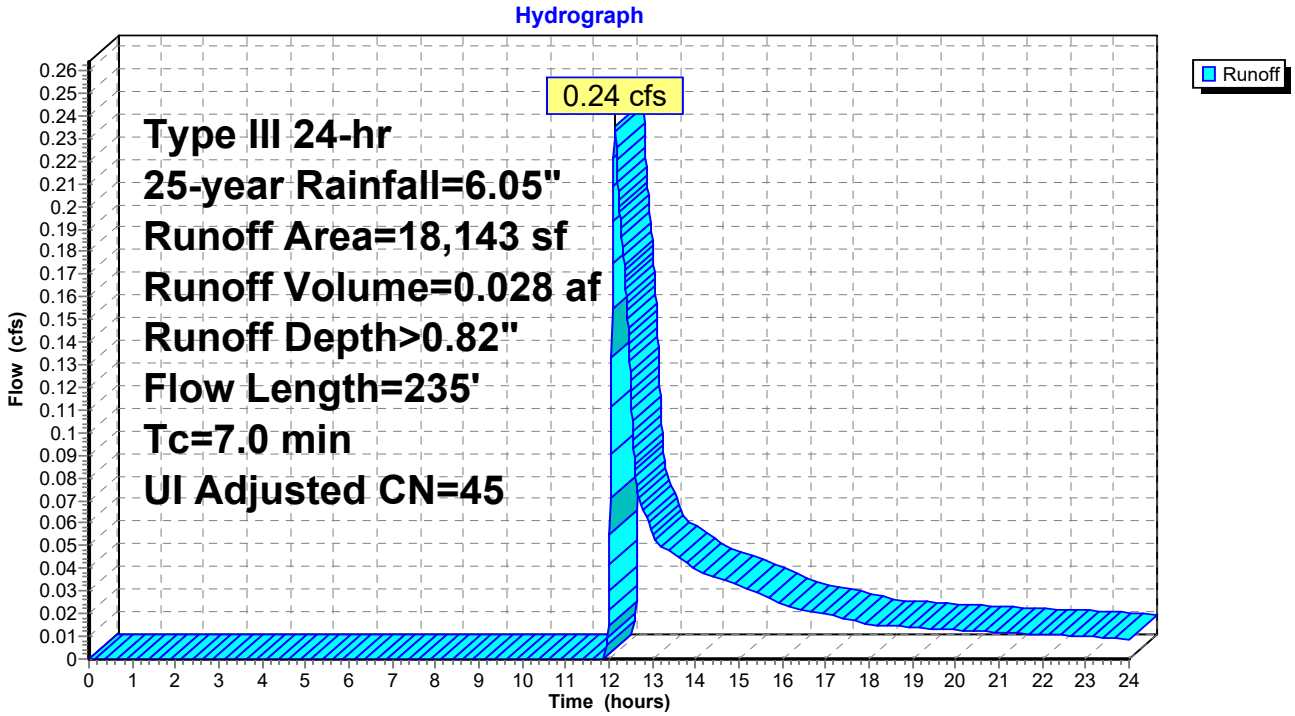
Runoff = 0.24 cfs @ 12.14 hrs, Volume= 0.028 af, Depth> 0.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-year Rainfall=6.05"

Area (sf)	CN	Adj	Description
1,850	98		Unconnected pavement, HSG A
14,767	39		>75% Grass cover, Good, HSG A
1,526	72		Dirt roads, HSG A
18,143	48	45	Weighted Average, UI Adjusted
16,293			89.80% Pervious Area
1,850			10.20% Impervious Area
1,850			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.9	50	0.0160	0.14		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.9	125	0.0220	2.39		**Shallow Concentrated Flow, B-C** Unpaved Kv= 16.1 fps
0.2	60	0.0470	4.40		**Shallow Concentrated Flow, C-D** Paved Kv= 20.3 fps
7.0	235	Total			

Subcatchment DA-2: Offstie Runoff to Rear of Property



Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment DA-1: Offsite Runoff to Runoff Area=30,635 sf 12.37% Impervious Runoff Depth>1.34"
Flow Length=165' Tc=5.6 min UI Adjusted CN=43 Runoff=0.85 cfs 0.079 af

Subcatchment DA-2: Offstie Runoff to Rear Runoff Area=18,143 sf 10.20% Impervious Runoff Depth>1.53"
Flow Length=235' Tc=7.0 min UI Adjusted CN=45 Runoff=0.58 cfs 0.053 af

Total Runoff Area = 1.120 ac Runoff Volume = 0.132 af Average Runoff Depth = 1.41"
88.44% Pervious = 0.990 ac 11.56% Impervious = 0.129 ac

Summary for Subcatchment DA-1: Offsite Runoff to Main Street

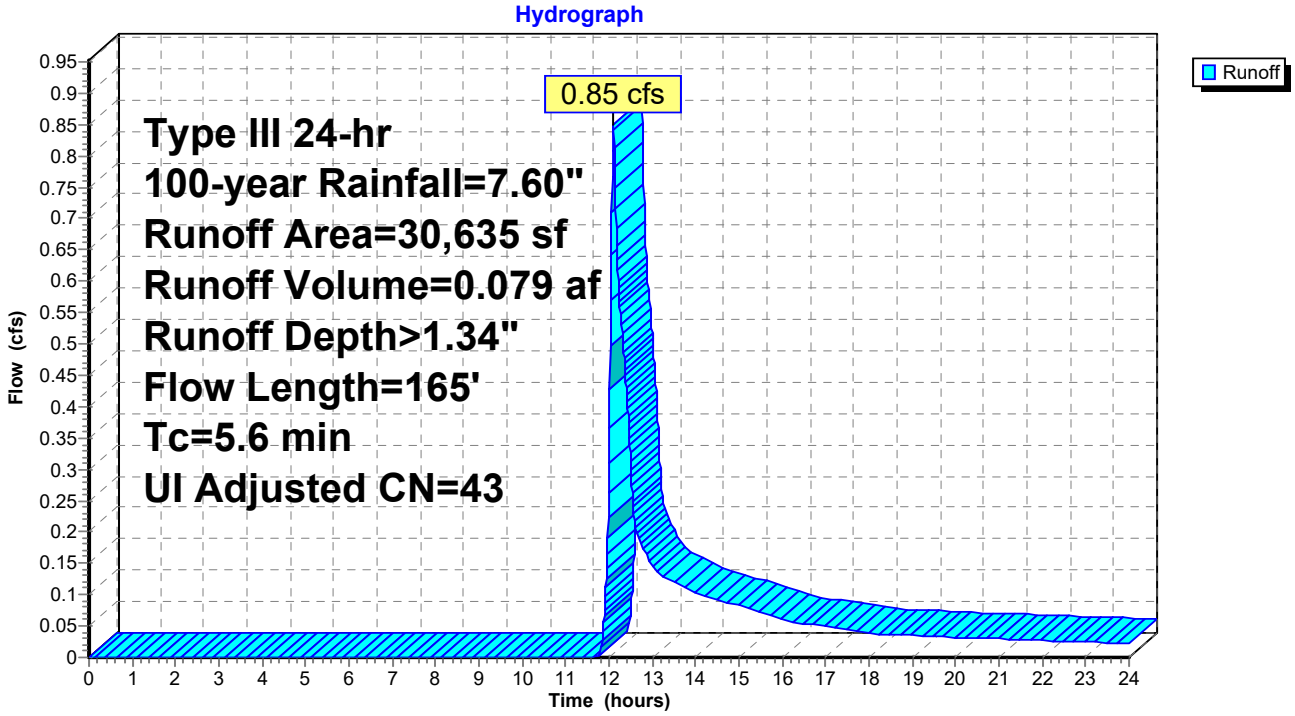
Runoff = 0.85 cfs @ 12.10 hrs, Volume= 0.079 af, Depth> 1.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.60"

Area (sf)	CN	Adj	Description
3,789	98		Unconnected pavement, HSG A
26,846	39		>75% Grass cover, Good, HSG A
30,635	46	43	Weighted Average, UI Adjusted
26,846			87.63% Pervious Area
3,789			12.37% Impervious Area
3,789			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.6	50	0.0300	0.18		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
1.0	115	0.0800	1.98		**Shallow Concentrated Flow, B-C** Short Grass Pasture Kv= 7.0 fps
5.6	165	Total			

Subcatchment DA-1: Offsite Runoff to Main Street



Summary for Subcatchment DA-2: Offstie Runoff to Rear of Property

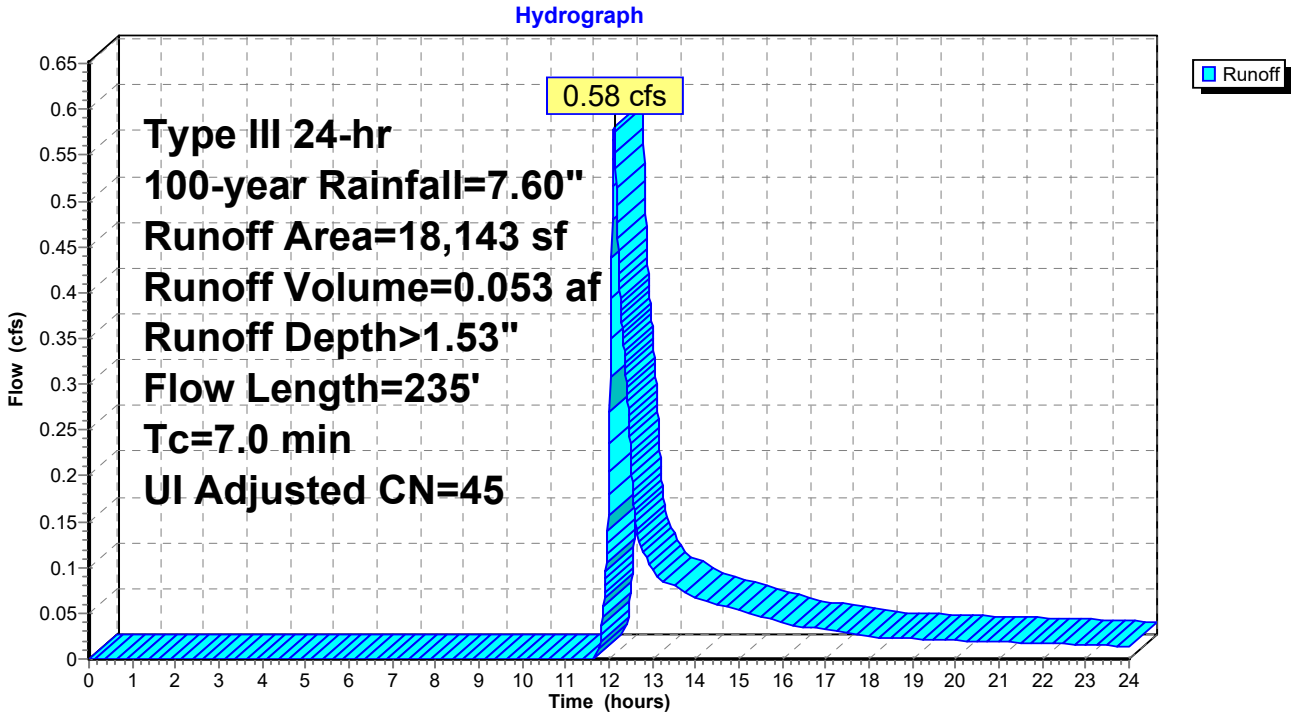
Runoff = 0.58 cfs @ 12.12 hrs, Volume= 0.053 af, Depth> 1.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.60"

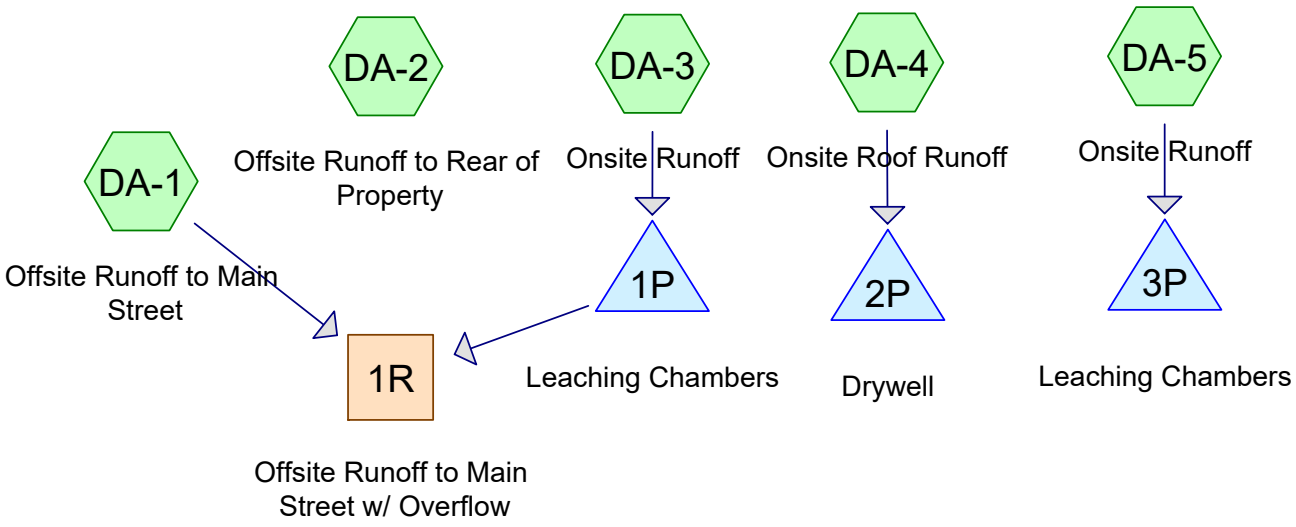
Area (sf)	CN	Adj	Description
1,850	98		Unconnected pavement, HSG A
14,767	39		>75% Grass cover, Good, HSG A
1,526	72		Dirt roads, HSG A
18,143	48	45	Weighted Average, UI Adjusted
16,293			89.80% Pervious Area
1,850			10.20% Impervious Area
1,850			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.9	50	0.0160	0.14		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.9	125	0.0220	2.39		**Shallow Concentrated Flow, B-C** Unpaved Kv= 16.1 fps
0.2	60	0.0470	4.40		**Shallow Concentrated Flow, C-D** Paved Kv= 20.3 fps
7.0	235	Total			

Subcatchment DA-2: Offstie Runoff to Rear of Property



PROPOSED CONDITIONS DRAINAGE CALCULATIONS



Subcat Reach Pond Link

Routing Diagram for Proposed Conditions
Prepared by JC Engineering Inc., Printed 9/13/2023

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.640	39	>75% Grass cover, Good, HSG A (DA-1, DA-2, DA-3, DA-5)
0.479	98	Paved parking, HSG A (DA-1, DA-2, DA-3, DA-5)
0.066	98	Roofs, HSG A (DA-4, DA-5)
1.185	**66**	**TOTAL AREA**

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
1.185	HSG A	DA-1, DA-2, DA-3, DA-4, DA-5
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.000	Other	
1.185		**TOTAL AREA**

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.640	0.000	0.000	0.000	0.000	0.640	>75% Grass cover, Good	DA-1, DA-2, DA-3, DA-5
0.479	0.000	0.000	0.000	0.000	0.479	Paved parking	DA-1, DA-2, DA-3, DA-5
0.066	0.000	0.000	0.000	0.000	0.066	Roofs	DA-4, DA-5
1.185	**0.000**	**0.000**	**0.000**	**0.000**	**1.185**	**TOTAL AREA**	

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment DA-1: Offsite Runoff to Main Runoff Area=14,327 sf 6.55% Impervious Runoff Depth>0.04"
Flow Length=140' Tc=5.3 min CN=43 Runoff=0.00 cfs 0.001 af

Subcatchment DA-2: Offsite Runoff to Rear Runoff Area=7,644 sf 19.15% Impervious Runoff Depth>0.18"
Flow Length=85' Tc=7.3 min CN=50 Runoff=0.01 cfs 0.003 af

Subcatchment DA-3: Onsite Runoff Runoff Area=20,433 sf 81.12% Impervious Runoff Depth>2.13"
Flow Length=200' Tc=1.4 min CN=87 Runoff=1.38 cfs 0.083 af

Subcatchment DA-4: Onsite Roof Runoff Runoff Area=1,440 sf 100.00% Impervious Runoff Depth>3.20"
Tc=5.0 min CN=98 Runoff=0.11 cfs 0.009 af

Subcatchment DA-5: Onsite Runoff Runoff Area=7,785 sf 42.58% Impervious Runoff Depth>0.67"
Flow Length=125' Tc=4.9 min CN=64 Runoff=0.12 cfs 0.010 af

Reach 1R: Offsite Runoff to Main Street w/ Overflow Inflow=0.00 cfs 0.001 af
Outflow=0.00 cfs 0.001 af

Pond 1P: Leaching Chambers Peak Elev=18.41' Storage=1,499 cf Inflow=1.38 cfs 0.083 af
Discarded=0.10 cfs 0.083 af Primary=0.00 cfs 0.000 af Outflow=0.10 cfs 0.083 af

Pond 2P: Drywell Peak Elev=24.52' Storage=150 cf Inflow=0.11 cfs 0.009 af
Outflow=0.01 cfs 0.009 af

Pond 3P: Leaching Chambers Peak Elev=22.97' Storage=103 cf Inflow=0.12 cfs 0.010 af
Outflow=0.02 cfs 0.010 af

Total Runoff Area = 1.185 ac Runoff Volume = 0.106 af Average Runoff Depth = 1.07"
54.03% Pervious = 0.640 ac 45.97% Impervious = 0.545 ac

Summary for Subcatchment DA-1: Offsite Runoff to Main Street

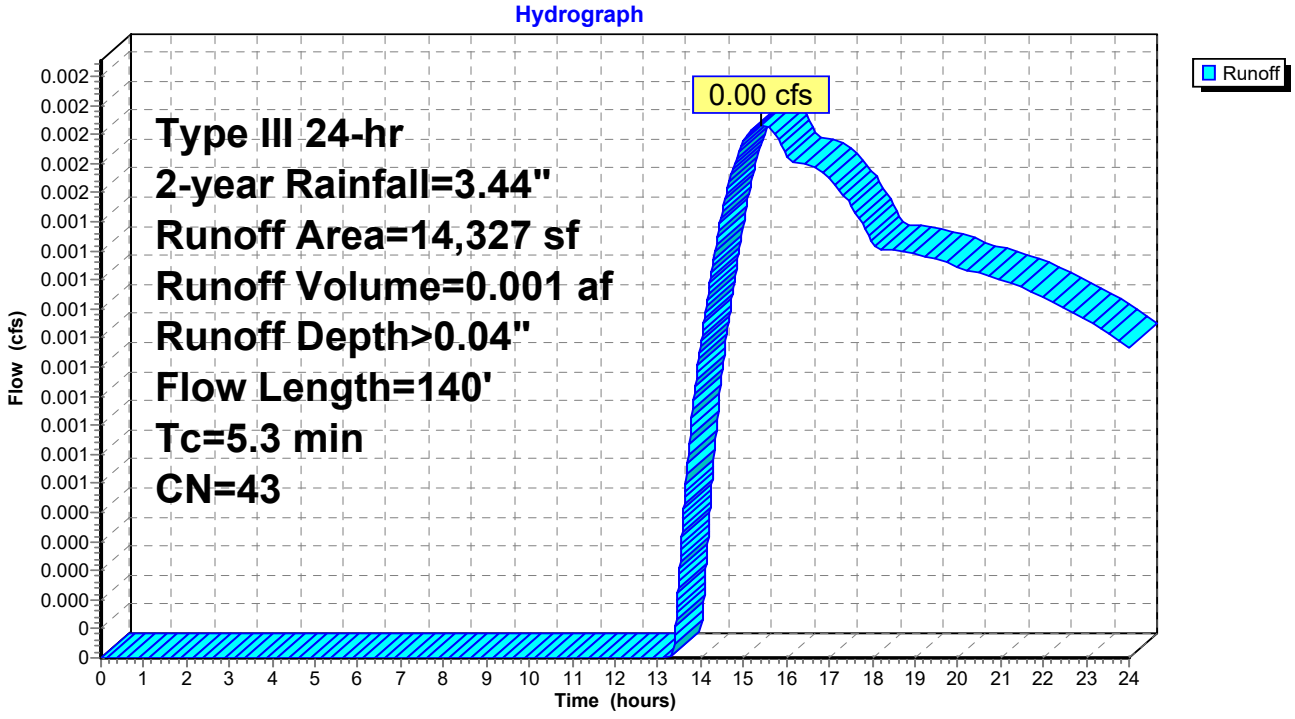
Runoff = 0.00 cfs @ 15.40 hrs, Volume= 0.001 af, Depth> 0.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.44"

Area (sf)	CN	Description
939	98	Paved parking, HSG A
13,388	39	>75% Grass cover, Good, HSG A
14,327	43	Weighted Average
13,388		93.45% Pervious Area
939		6.55% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.6	50	0.0300	0.18		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.7	90	0.0900	2.10		**Shallow Concentrated Flow, B-C** Short Grass Pasture Kv= 7.0 fps
5.3	140	Total			

Subcatchment DA-1: Offsite Runoff to Main Street



Summary for Subcatchment DA-2: Offsite Runoff to Rear of Property

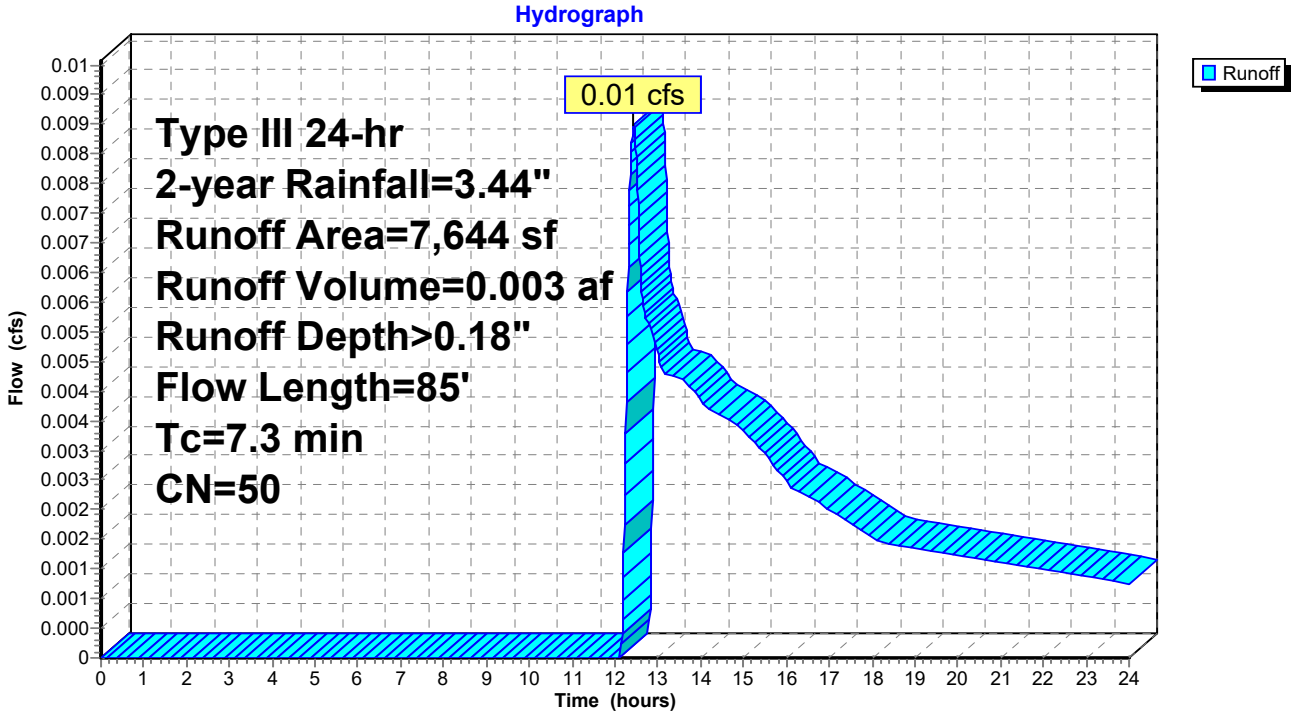
Runoff = 0.01 cfs @ 12.44 hrs, Volume= 0.003 af, Depth> 0.18"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.44"

Area (sf)	CN	Description
6,180	39	>75% Grass cover, Good, HSG A
1,464	98	Paved parking, HSG A
7,644	50	Weighted Average
6,180		80.85% Pervious Area
1,464		19.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.7	50	0.0120	0.13		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.6	35	0.0170	0.91		**Shallow Concentrated Flow, B-C** Short Grass Pasture Kv= 7.0 fps
7.3	85	Total			

Subcatchment DA-2: Offsite Runoff to Rear of Property



Summary for Subcatchment DA-3: Onsite Runoff

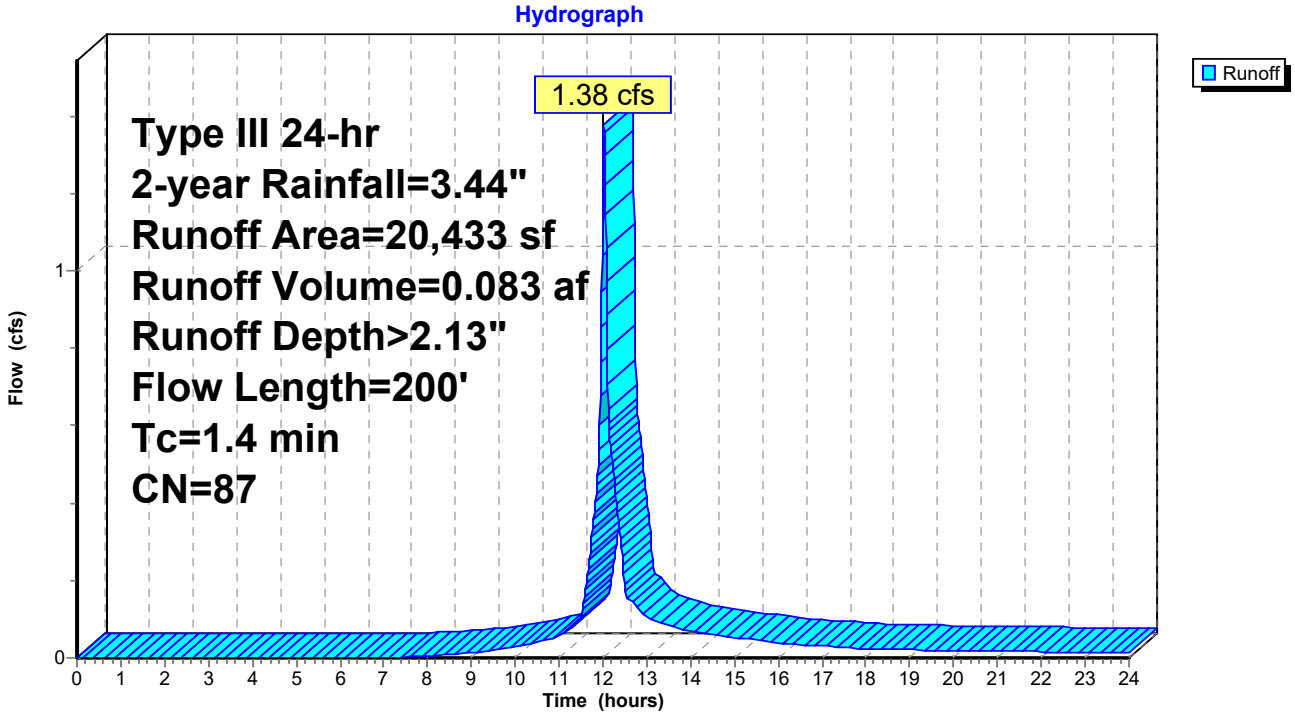
Runoff = 1.38 cfs @ 12.02 hrs, Volume= 0.083 af, Depth> 2.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.44"

Area (sf)	CN	Description
16,576	98	Paved parking, HSG A
3,857	39	>75% Grass cover, Good, HSG A
20,433	87	Weighted Average
3,857		18.88% Pervious Area
16,576		81.12% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.7	50	0.0200	1.24		**Sheet Flow, A-B** Smooth surfaces n= 0.011 P2= 3.44"
0.7	150	0.0300	3.52		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
1.4	200	Total			

Subcatchment DA-3: Onsite Runoff



Summary for Subcatchment DA-4: Onsite Roof Runoff

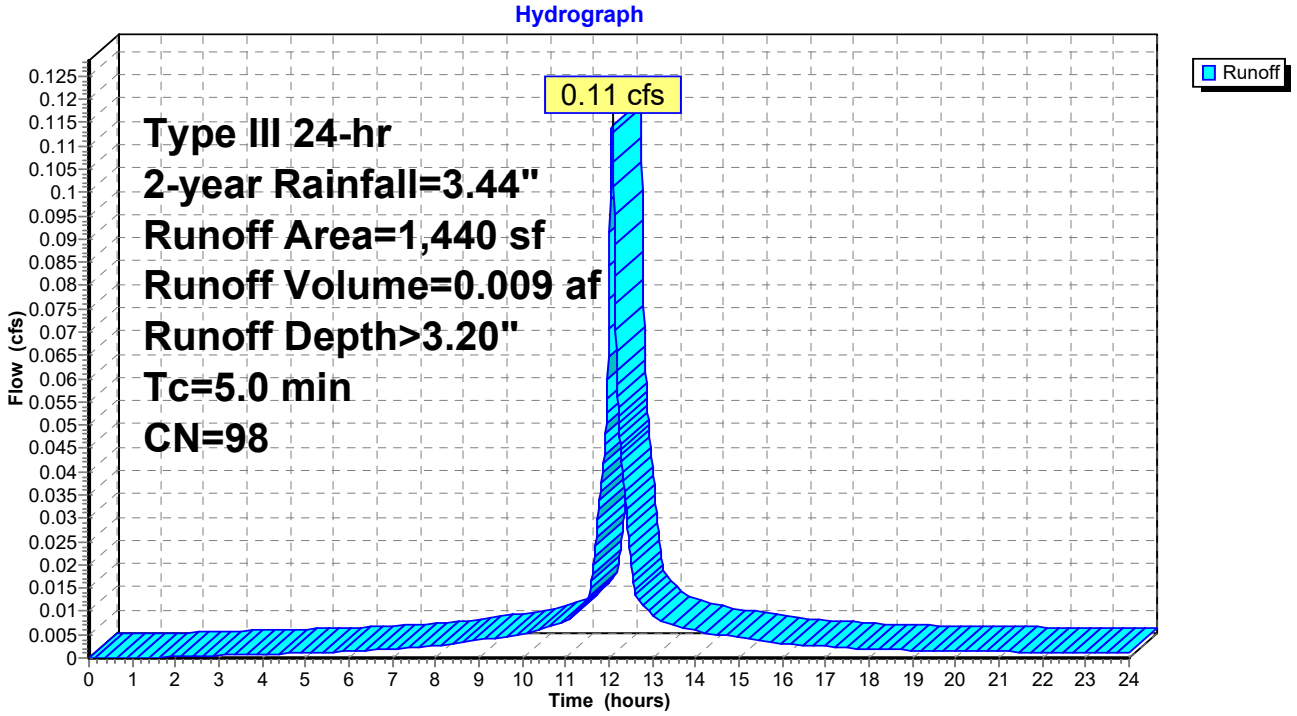
Runoff = 0.11 cfs @ 12.07 hrs, Volume= 0.009 af, Depth> 3.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.44"

Area (sf)	CN	Description
1,440	98	Roofs, HSG A
1,440		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry, Roof Runoff**

Subcatchment DA-4: Onsite Roof Runoff



Summary for Subcatchment DA-5: Onsite Runoff

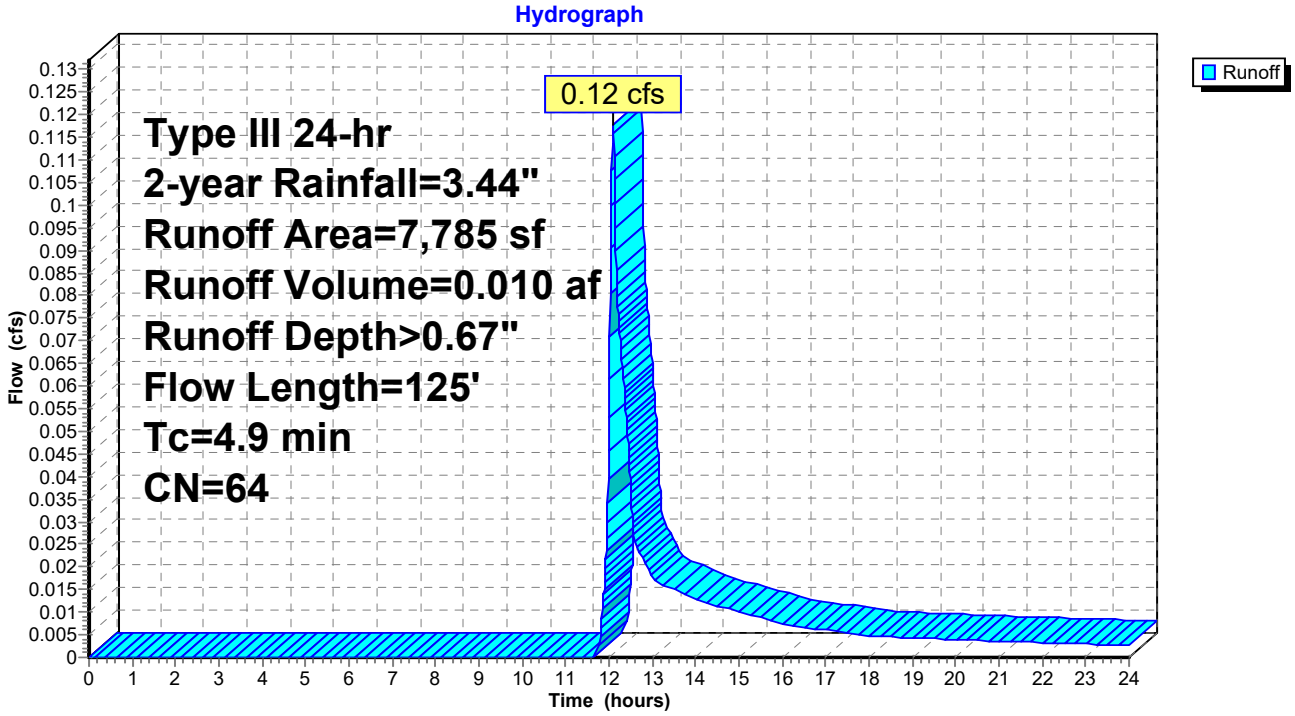
Runoff = 0.12 cfs @ 12.09 hrs, Volume= 0.010 af, Depth> 0.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.44"

Area (sf)	CN	Description
1,440	98	Roofs, HSG A
1,875	98	Paved parking, HSG A
4,470	39	>75% Grass cover, Good, HSG A
7,785	64	Weighted Average
4,470		57.42% Pervious Area
3,315		42.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.5	50	0.0320	0.19		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.4	75	0.0270	3.34		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
4.9	125	Total			

Subcatchment DA-5: Onsite Runoff



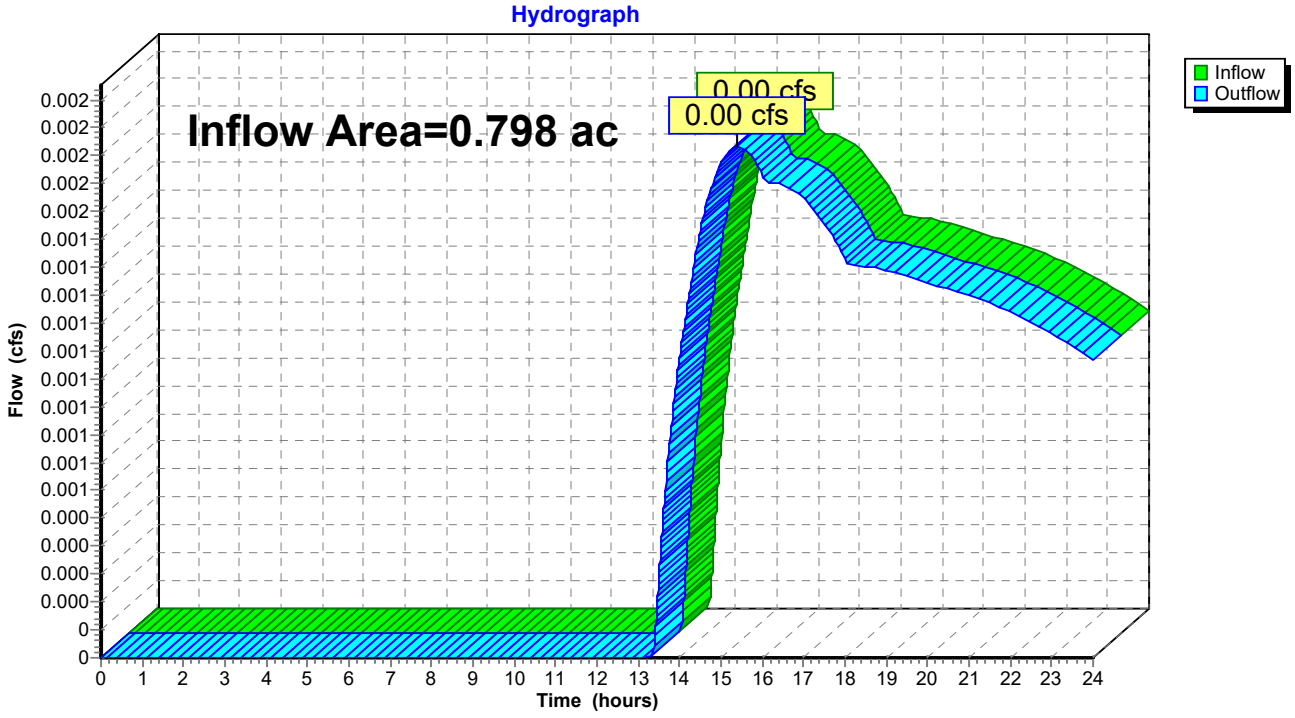
Summary for Reach 1R: Offsite Runoff to Main Street w/ Overflow

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.798 ac, 50.39% Impervious, Inflow Depth > 0.02" for 2-year event
Inflow = 0.00 cfs @ 15.40 hrs, Volume= 0.001 af
Outflow = 0.00 cfs @ 15.40 hrs, Volume= 0.001 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Reach 1R: Offsite Runoff to Main Street w/ Overflow



Summary for Pond 1P: Leaching Chambers

Inflow Area = 0.469 ac, 81.12% Impervious, Inflow Depth > 2.13" for 2-year event
Inflow = 1.38 cfs @ 12.02 hrs, Volume= 0.083 af
Outflow = 0.10 cfs @ 13.05 hrs, Volume= 0.083 af, Atten= 93%, Lag= 61.9 min
Discarded = 0.10 cfs @ 13.05 hrs, Volume= 0.083 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Peak Elev= 18.41' @ 13.05 hrs Surf.Area= 1,449 sf Storage= 1,499 cf

Plug-Flow detention time= 140.5 min calculated for 0.083 af (100% of inflow)
Center-of-Mass det. time= 139.1 min (950.5 - 811.4)

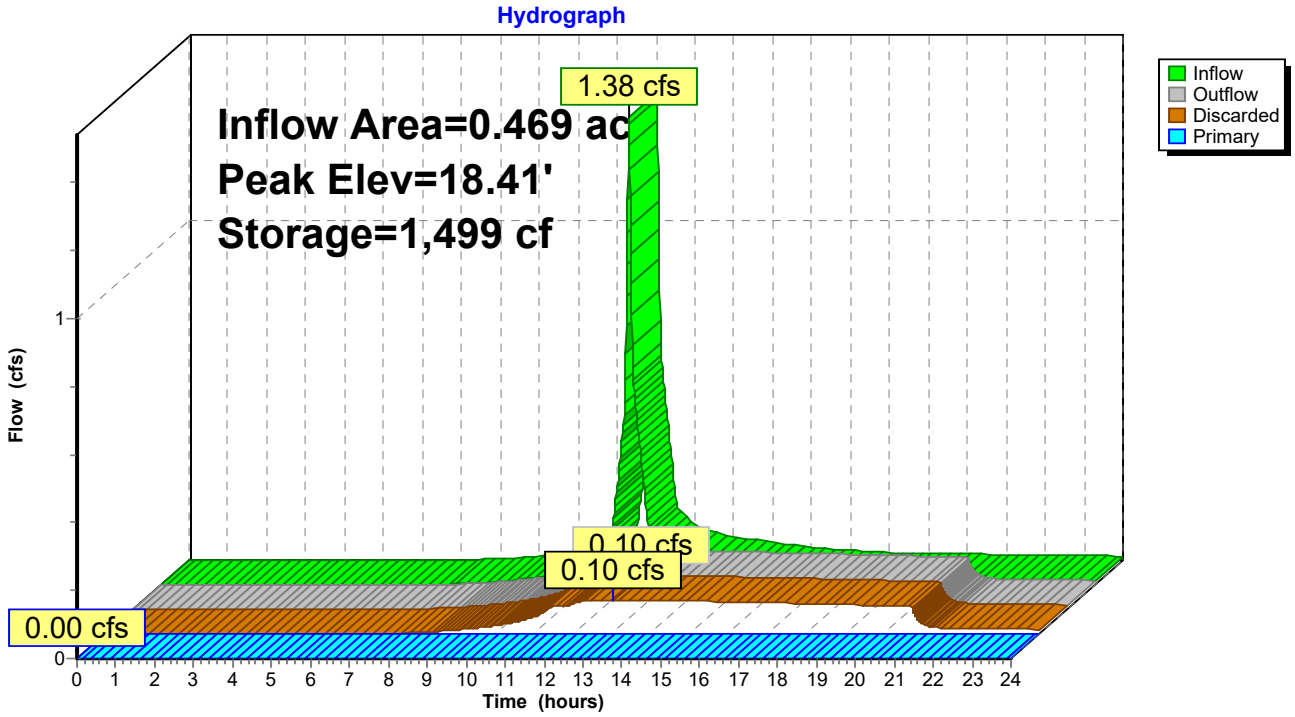
Volume	Invert	Avail.Storage	Storage Description
#1	16.50'	3,052 cf	**23.00'W x 63.00'L x 6.67'H Prismatoid** 9,665 cf Overall - 2,036 cf Embedded = 7,629 cf x 40.0% Voids
#2	16.50'	2,036 cf	**6.00'D x 6.00'H Vertical Cone/Cylinder** x 12 Inside #1
		5,087 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	16.50'	**2.410 in/hr Exfiltration over Wetted area**
#2	Primary	22.51'	**24.0" x 24.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.10 cfs @ 13.05 hrs HW=18.41' (Free Discharge)
↑—**1=Exfiltration** (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=16.50' (Free Discharge)
↑—**2=Orifice/Grate** (Controls 0.00 cfs)

Pond 1P: Leaching Chambers



Summary for Pond 2P: Drywell

Inflow Area = 0.033 ac,100.00% Impervious, Inflow Depth > 3.20" for 2-year event
Inflow = 0.11 cfs @ 12.07 hrs, Volume= 0.009 af
Outflow = 0.01 cfs @ 11.18 hrs, Volume= 0.009 af, Atten= 93%, Lag= 0.0 min
Discarded = 0.01 cfs @ 11.18 hrs, Volume= 0.009 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Peak Elev= 24.52' @ 13.19 hrs Surf.Area= 144 sf Storage= 150 cf

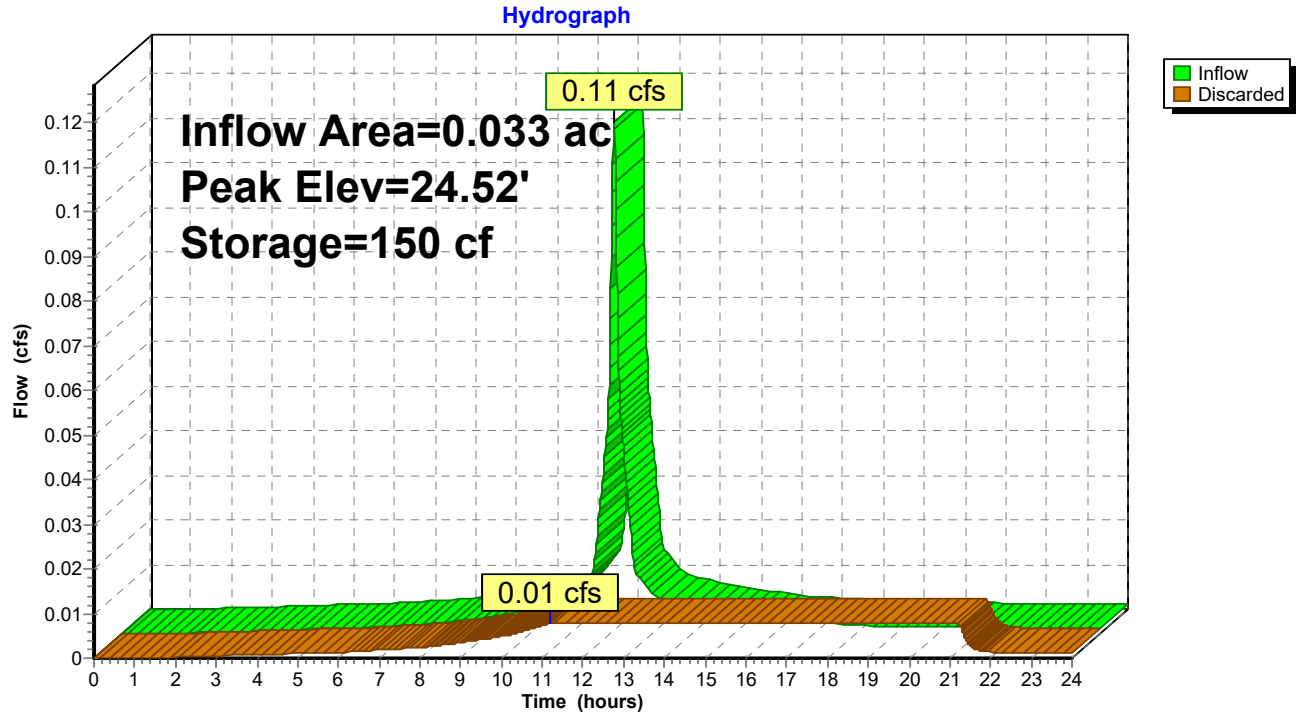
Plug-Flow detention time= 144.0 min calculated for 0.009 af (100% of inflow)
Center-of-Mass det. time= 142.9 min (896.5 - 753.5)

Volume	Invert	Avail.Storage	Storage Description
#1	22.50'	316 cf	**12.00'W x 12.00'L x 6.67'H Prismatoid** 960 cf Overall - 170 cf Embedded = 791 cf x 40.0% Voids
#2	22.50'	170 cf	**6.00'D x 6.00'H Vertical Cone/Cylinder** Inside #1
		486 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	22.50'	**2.410 in/hr Exfiltration over Surface area**

Discarded OutFlow Max=0.01 cfs @ 11.18 hrs HW=22.57' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.01 cfs)

Pond 2P: Drywell



Summary for Pond 3P: Leaching Chambers

Inflow Area = 0.179 ac, 42.58% Impervious, Inflow Depth > 0.67" for 2-year event
Inflow = 0.12 cfs @ 12.09 hrs, Volume= 0.010 af
Outflow = 0.02 cfs @ 12.03 hrs, Volume= 0.010 af, Atten= 80%, Lag= 0.0 min
Discarded = 0.02 cfs @ 12.03 hrs, Volume= 0.010 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Peak Elev= 22.97' @ 12.69 hrs Surf.Area= 420 sf Storage= 103 cf

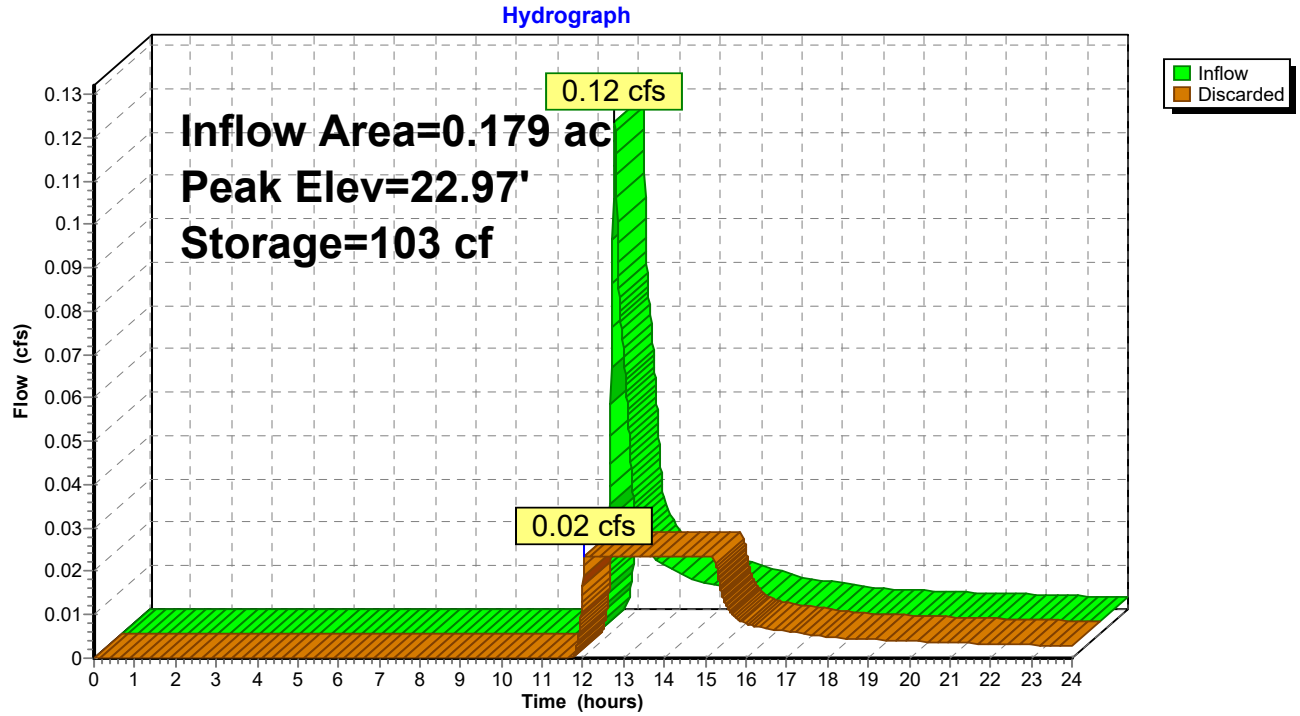
Plug-Flow detention time= 34.8 min calculated for 0.010 af (100% of inflow)
Center-of-Mass det. time= 32.7 min (923.5 - 890.7)

Volume	Invert	Avail.Storage	Storage Description
#1	22.50'	917 cf	**14.00'W x 30.00'L x 6.67'H Prismatoid** 2,801 cf Overall - 509 cf Embedded = 2,292 cf x 40.0% Voids
#2	22.50'	509 cf	**6.00'D x 6.00'H Vertical Cone/Cylinder** x 3 Inside #1
		1,426 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	22.50'	**2.410 in/hr Exfiltration over Surface area**

Discarded OutFlow Max=0.02 cfs @ 12.03 hrs HW=22.57' (Free Discharge)
↑**1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Pond 3P: Leaching Chambers



Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment DA-1: Offsite Runoff to Main Runoff Area=14,327 sf 6.55% Impervious Runoff Depth>0.37"
Flow Length=140' Tc=5.3 min CN=43 Runoff=0.05 cfs 0.010 af

Subcatchment DA-2: Offsite Runoff to Rear Runoff Area=7,644 sf 19.15% Impervious Runoff Depth>0.71"
Flow Length=85' Tc=7.3 min CN=50 Runoff=0.09 cfs 0.010 af

Subcatchment DA-3: Onsite Runoff Runoff Area=20,433 sf 81.12% Impervious Runoff Depth>3.61"
Flow Length=200' Tc=1.4 min CN=87 Runoff=2.30 cfs 0.141 af

Subcatchment DA-4: Onsite Roof Runoff Runoff Area=1,440 sf 100.00% Impervious Runoff Depth>4.81"
Tc=5.0 min CN=98 Runoff=0.17 cfs 0.013 af

Subcatchment DA-5: Onsite Runoff Runoff Area=7,785 sf 42.58% Impervious Runoff Depth>1.61"
Flow Length=125' Tc=4.9 min CN=64 Runoff=0.33 cfs 0.024 af

Reach 1R: Offsite Runoff to Main Street w/ Overflow Inflow=0.05 cfs 0.010 af
Outflow=0.05 cfs 0.010 af

Pond 1P: Leaching Chambers Peak Elev=20.26' Storage=2,945 cf Inflow=2.30 cfs 0.141 af
Discarded=0.12 cfs 0.125 af Primary=0.00 cfs 0.000 af Outflow=0.12 cfs 0.125 af

Pond 2P: Drywell Peak Elev=26.02' Storage=263 cf Inflow=0.17 cfs 0.013 af
Outflow=0.01 cfs 0.011 af

Pond 3P: Leaching Chambers Peak Elev=24.57' Storage=453 cf Inflow=0.33 cfs 0.024 af
Outflow=0.02 cfs 0.024 af

Total Runoff Area = 1.185 ac Runoff Volume = 0.199 af Average Runoff Depth = 2.01"
54.03% Pervious = 0.640 ac 45.97% Impervious = 0.545 ac

Summary for Subcatchment DA-1: Offsite Runoff to Main Street

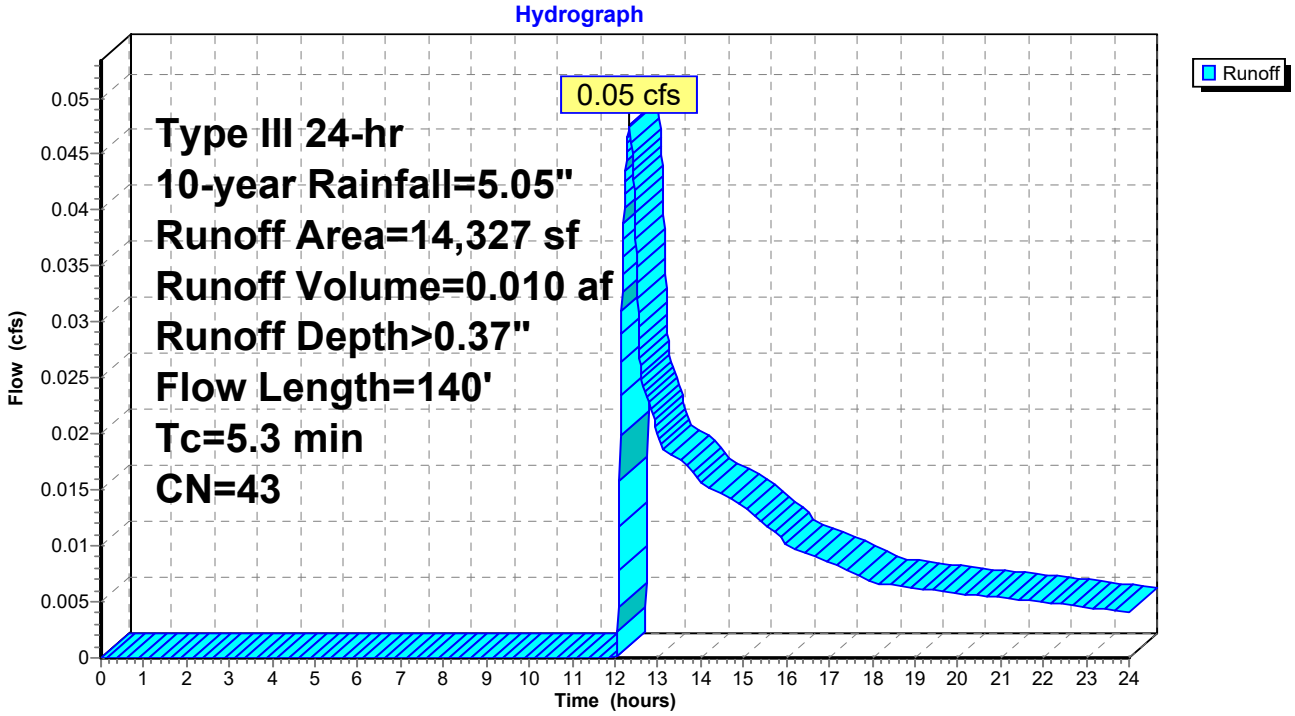
Runoff = 0.05 cfs @ 12.34 hrs, Volume= 0.010 af, Depth> 0.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.05"

Area (sf)	CN	Description
939	98	Paved parking, HSG A
13,388	39	>75% Grass cover, Good, HSG A
14,327	43	Weighted Average
13,388		93.45% Pervious Area
939		6.55% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.6	50	0.0300	0.18		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.7	90	0.0900	2.10		**Shallow Concentrated Flow, B-C** Short Grass Pasture Kv= 7.0 fps
5.3	140	Total			

Subcatchment DA-1: Offsite Runoff to Main Street



Summary for Subcatchment DA-2: Offsite Runoff to Rear of Property

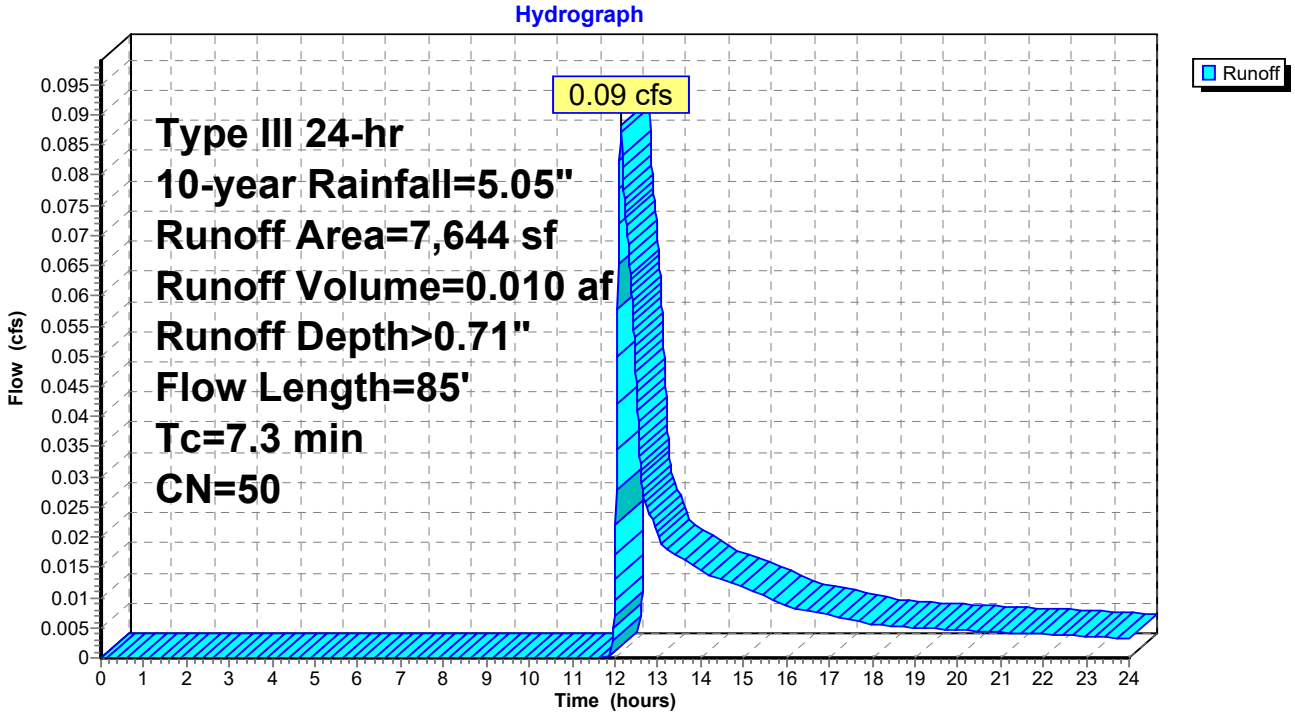
Runoff = 0.09 cfs @ 12.14 hrs, Volume= 0.010 af, Depth> 0.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.05"

Area (sf)	CN	Description
6,180	39	>75% Grass cover, Good, HSG A
1,464	98	Paved parking, HSG A
7,644	50	Weighted Average
6,180		80.85% Pervious Area
1,464		19.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.7	50	0.0120	0.13		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.6	35	0.0170	0.91		**Shallow Concentrated Flow, B-C** Short Grass Pasture Kv= 7.0 fps
7.3	85	Total			

Subcatchment DA-2: Offsite Runoff to Rear of Property



Summary for Subcatchment DA-3: Onsite Runoff

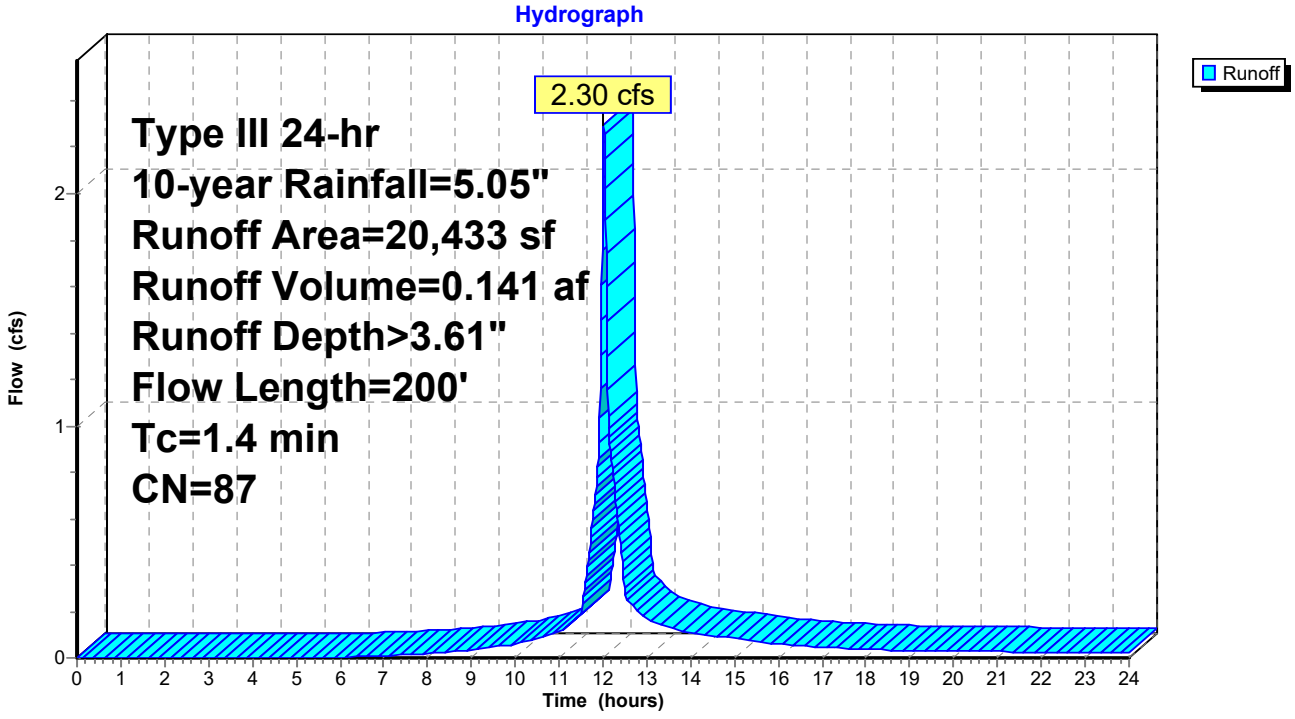
Runoff = 2.30 cfs @ 12.02 hrs, Volume= 0.141 af, Depth> 3.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.05"

Area (sf)	CN	Description
16,576	98	Paved parking, HSG A
3,857	39	>75% Grass cover, Good, HSG A
20,433	87	Weighted Average
3,857		18.88% Pervious Area
16,576		81.12% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.7	50	0.0200	1.24		**Sheet Flow, A-B** Smooth surfaces n= 0.011 P2= 3.44"
0.7	150	0.0300	3.52		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
1.4	200	Total			

Subcatchment DA-3: Onsite Runoff



Summary for Subcatchment DA-4: Onsite Roof Runoff

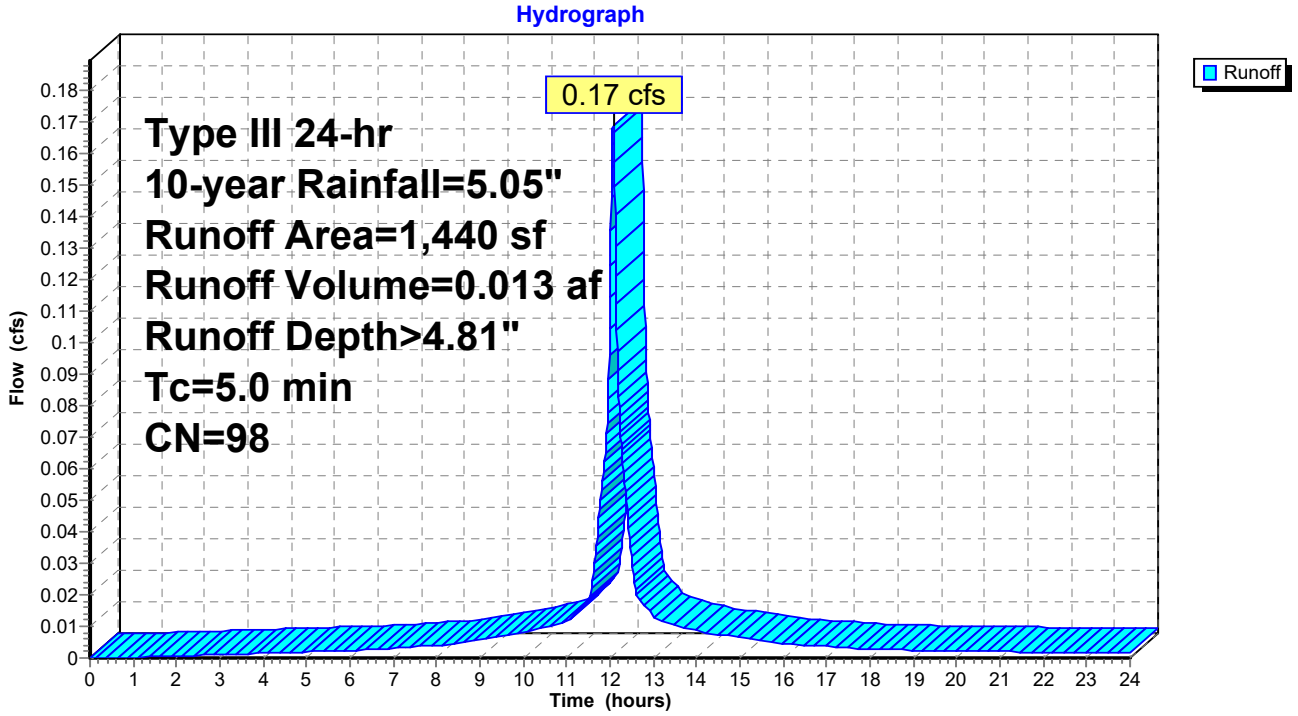
Runoff = 0.17 cfs @ 12.07 hrs, Volume= 0.013 af, Depth> 4.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.05"

Area (sf)	CN	Description
1,440	98	Roofs, HSG A
1,440		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry, Roof Runoff**

Subcatchment DA-4: Onsite Roof Runoff



Summary for Subcatchment DA-5: Onsite Runoff

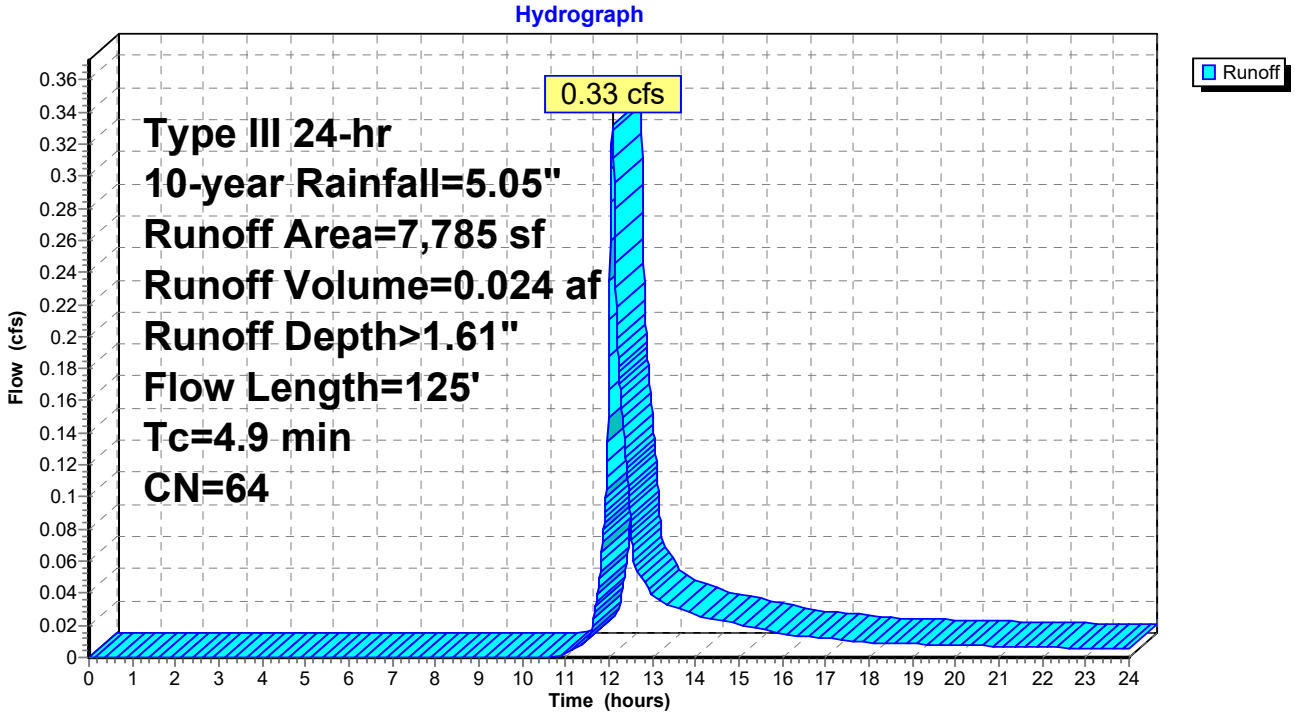
Runoff = 0.33 cfs @ 12.08 hrs, Volume= 0.024 af, Depth> 1.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.05"

Area (sf)	CN	Description
1,440	98	Roofs, HSG A
1,875	98	Paved parking, HSG A
4,470	39	>75% Grass cover, Good, HSG A
7,785	64	Weighted Average
4,470		57.42% Pervious Area
3,315		42.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.5	50	0.0320	0.19		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.4	75	0.0270	3.34		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
4.9	125	Total			

Subcatchment DA-5: Onsite Runoff



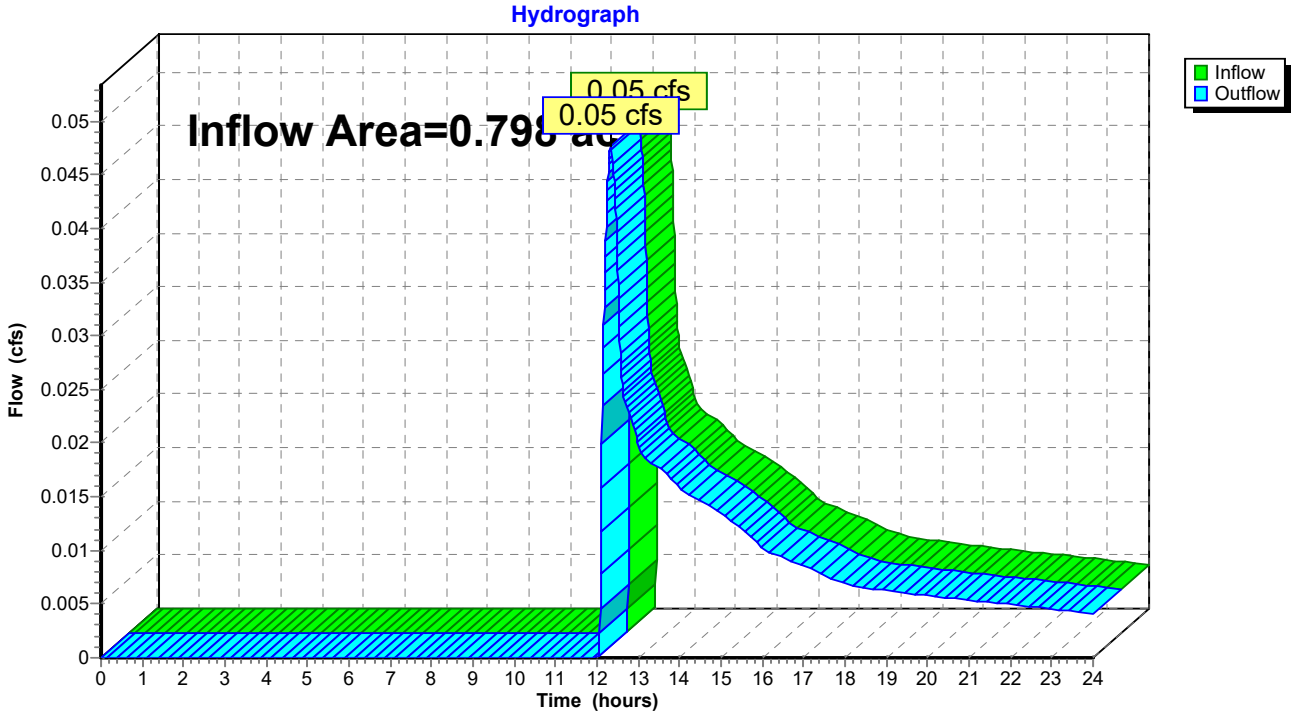
Summary for Reach 1R: Offsite Runoff to Main Street w/ Overflow

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.798 ac, 50.39% Impervious, Inflow Depth > 0.15" for 10-year event
Inflow = 0.05 cfs @ 12.34 hrs, Volume= 0.010 af
Outflow = 0.05 cfs @ 12.34 hrs, Volume= 0.010 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Reach 1R: Offsite Runoff to Main Street w/ Overflow



Summary for Pond 1P: Leaching Chambers

Inflow Area = 0.469 ac, 81.12% Impervious, Inflow Depth > 3.61" for 10-year event
Inflow = 2.30 cfs @ 12.02 hrs, Volume= 0.141 af
Outflow = 0.12 cfs @ 13.84 hrs, Volume= 0.125 af, Atten= 95%, Lag= 109.3 min
Discarded = 0.12 cfs @ 13.84 hrs, Volume= 0.125 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Peak Elev= 20.26' @ 13.84 hrs Surf.Area= 1,449 sf Storage= 2,945 cf

Plug-Flow detention time= 249.6 min calculated for 0.125 af (89% of inflow)
Center-of-Mass det. time= 196.8 min (993.3 - 796.5)

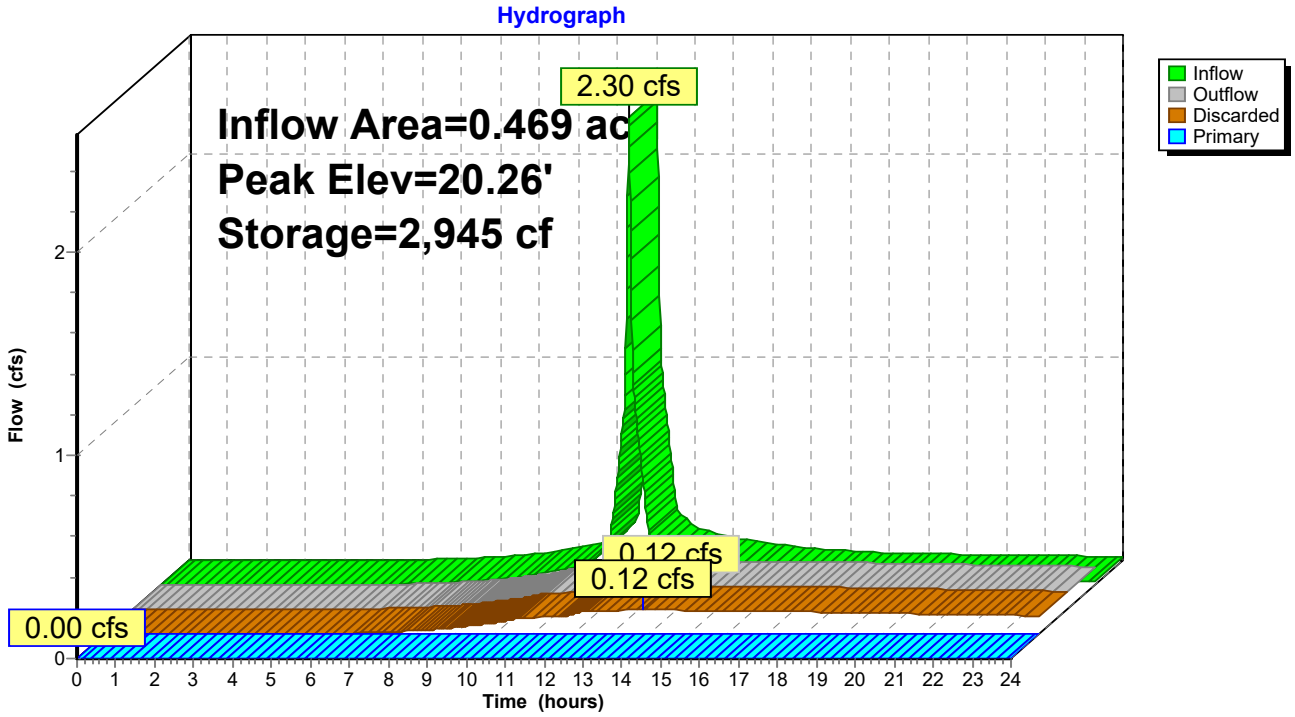
Volume	Invert	Avail.Storage	Storage Description
#1	16.50'	3,052 cf	**23.00'W x 63.00'L x 6.67'H Prismatoid** 9,665 cf Overall - 2,036 cf Embedded = 7,629 cf x 40.0% Voids
#2	16.50'	2,036 cf	**6.00'D x 6.00'H Vertical Cone/Cylinder** x 12 Inside #1
		5,087 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	16.50'	**2.410 in/hr Exfiltration over Wetted area**
#2	Primary	22.51'	**24.0" x 24.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.12 cfs @ 13.84 hrs HW=20.26' (Free Discharge)
↑—**1=Exfiltration** (Exfiltration Controls 0.12 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=16.50' (Free Discharge)
↑—**2=Orifice/Grate** (Controls 0.00 cfs)

Pond 1P: Leaching Chambers



Summary for Pond 2P: Drywell

Inflow Area = 0.033 ac,100.00% Impervious, Inflow Depth > 4.81" for 10-year event
Inflow = 0.17 cfs @ 12.07 hrs, Volume= 0.013 af
Outflow = 0.01 cfs @ 10.23 hrs, Volume= 0.011 af, Atten= 95%, Lag= 0.0 min
Discarded = 0.01 cfs @ 10.23 hrs, Volume= 0.011 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Peak Elev= 26.02' @ 14.18 hrs Surf.Area= 144 sf Storage= 263 cf

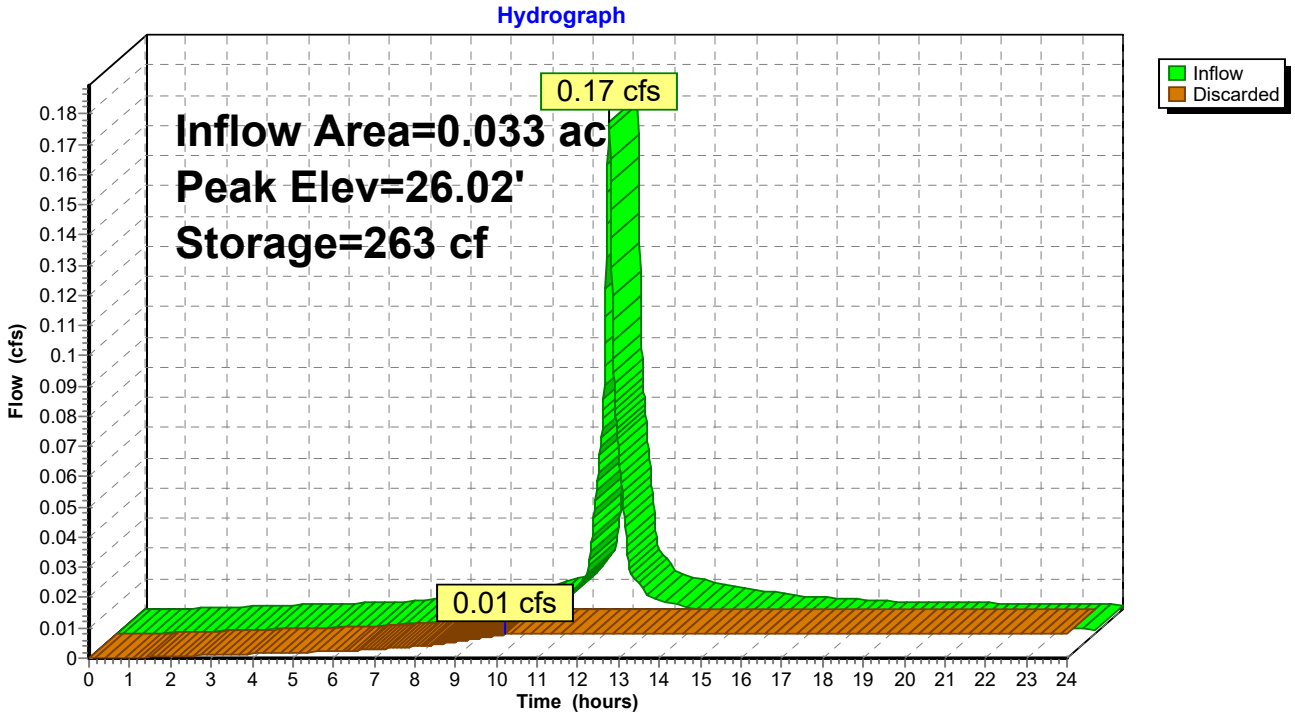
Plug-Flow detention time= 241.4 min calculated for 0.011 af (84% of inflow)
Center-of-Mass det. time= 175.2 min (921.7 - 746.5)

Volume	Invert	Avail.Storage	Storage Description
#1	22.50'	316 cf	**12.00'W x 12.00'L x 6.67'H Prismatoid** 960 cf Overall - 170 cf Embedded = 791 cf x 40.0% Voids
#2	22.50'	170 cf	**6.00'D x 6.00'H Vertical Cone/Cylinder** Inside #1
		486 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	22.50'	**2.410 in/hr Exfiltration over Surface area**

Discarded OutFlow Max=0.01 cfs @ 10.23 hrs HW=22.57' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.01 cfs)

Pond 2P: Drywell



Summary for Pond 3P: Leaching Chambers

Inflow Area = 0.179 ac, 42.58% Impervious, Inflow Depth > 1.61" for 10-year event
Inflow = 0.33 cfs @ 12.08 hrs, Volume= 0.024 af
Outflow = 0.02 cfs @ 11.77 hrs, Volume= 0.024 af, Atten= 93%, Lag= 0.0 min
Discarded = 0.02 cfs @ 11.77 hrs, Volume= 0.024 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Peak Elev= 24.57' @ 14.48 hrs Surf.Area= 420 sf Storage= 453 cf

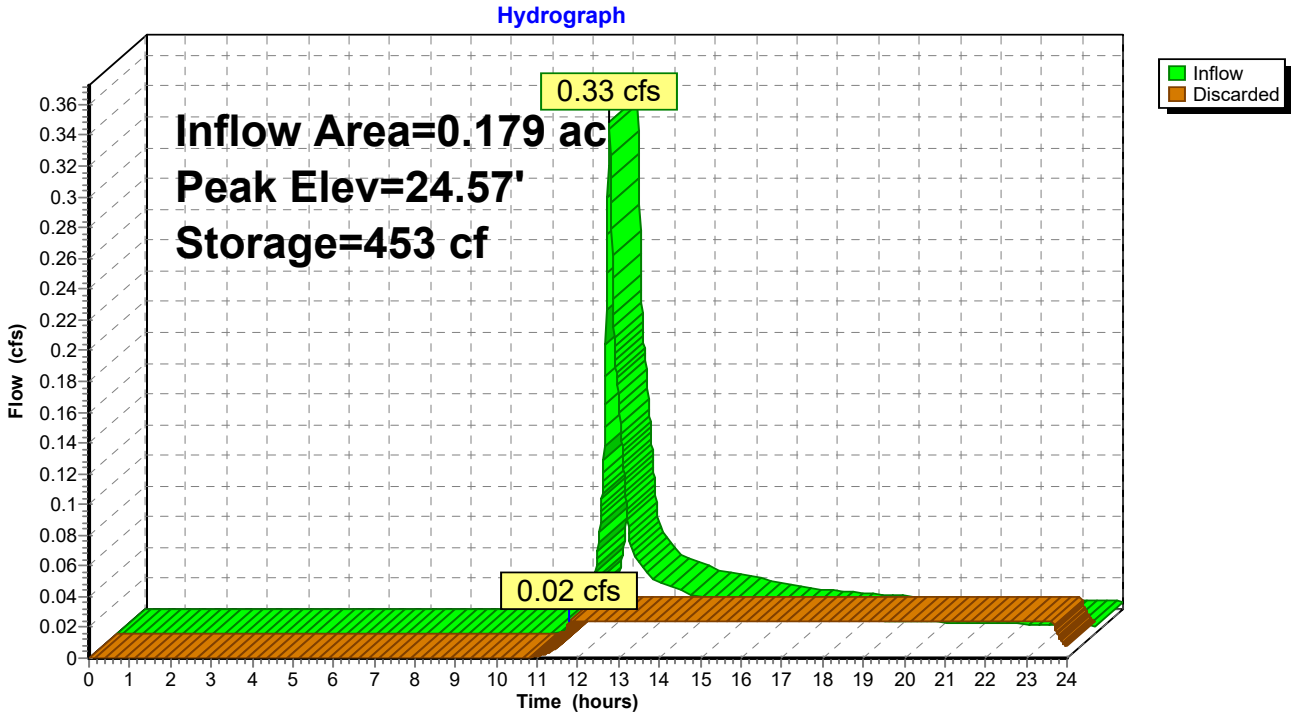
Plug-Flow detention time= 203.0 min calculated for 0.024 af (99% of inflow)
Center-of-Mass det. time= 200.3 min (1,061.5 - 861.2)

Volume	Invert	Avail.Storage	Storage Description
#1	22.50'	917 cf	**14.00'W x 30.00'L x 6.67'H Prismatoid** 2,801 cf Overall - 509 cf Embedded = 2,292 cf x 40.0% Voids
#2	22.50'	509 cf	**6.00'D x 6.00'H Vertical Cone/Cylinder** x 3 Inside #1
		1,426 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	22.50'	**2.410 in/hr Exfiltration over Surface area**

Discarded OutFlow Max=0.02 cfs @ 11.77 hrs HW=22.57' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.02 cfs)

Pond 3P: Leaching Chambers



Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment DA-1: Offsite Runoff to Main Runoff Area=14,327 sf 6.55% Impervious Runoff Depth>0.69"
Flow Length=140' Tc=5.3 min CN=43 Runoff=0.14 cfs 0.019 af

Subcatchment DA-2: Offsite Runoff to Rear Runoff Area=7,644 sf 19.15% Impervious Runoff Depth>1.16"
Flow Length=85' Tc=7.3 min CN=50 Runoff=0.18 cfs 0.017 af

Subcatchment DA-3: Onsite Runoff Runoff Area=20,433 sf 81.12% Impervious Runoff Depth>4.56"
Flow Length=200' Tc=1.4 min CN=87 Runoff=2.88 cfs 0.178 af

Subcatchment DA-4: Onsite Roof Runoff Runoff Area=1,440 sf 100.00% Impervious Runoff Depth>5.81"
Tc=5.0 min CN=98 Runoff=0.20 cfs 0.016 af

Subcatchment DA-5: Onsite Runoff Runoff Area=7,785 sf 42.58% Impervious Runoff Depth>2.30"
Flow Length=125' Tc=4.9 min CN=64 Runoff=0.49 cfs 0.034 af

Reach 1R: Offsite Runoff to Main Street w/ Overflow Inflow=0.14 cfs 0.019 af
Outflow=0.14 cfs 0.019 af

Pond 1P: Leaching Chambers Peak Elev=21.53' Storage=3,942 cf Inflow=2.88 cfs 0.178 af
Discarded=0.13 cfs 0.142 af Primary=0.00 cfs 0.000 af Outflow=0.13 cfs 0.142 af

Pond 2P: Drywell Peak Elev=27.09' Storage=342 cf Inflow=0.20 cfs 0.016 af
Outflow=0.01 cfs 0.012 af

Pond 3P: Leaching Chambers Peak Elev=26.02' Storage=770 cf Inflow=0.49 cfs 0.034 af
Outflow=0.02 cfs 0.025 af

Total Runoff Area = 1.185 ac Runoff Volume = 0.265 af Average Runoff Depth = 2.68"
54.03% Pervious = 0.640 ac 45.97% Impervious = 0.545 ac

Summary for Subcatchment DA-1: Offsite Runoff to Main Street

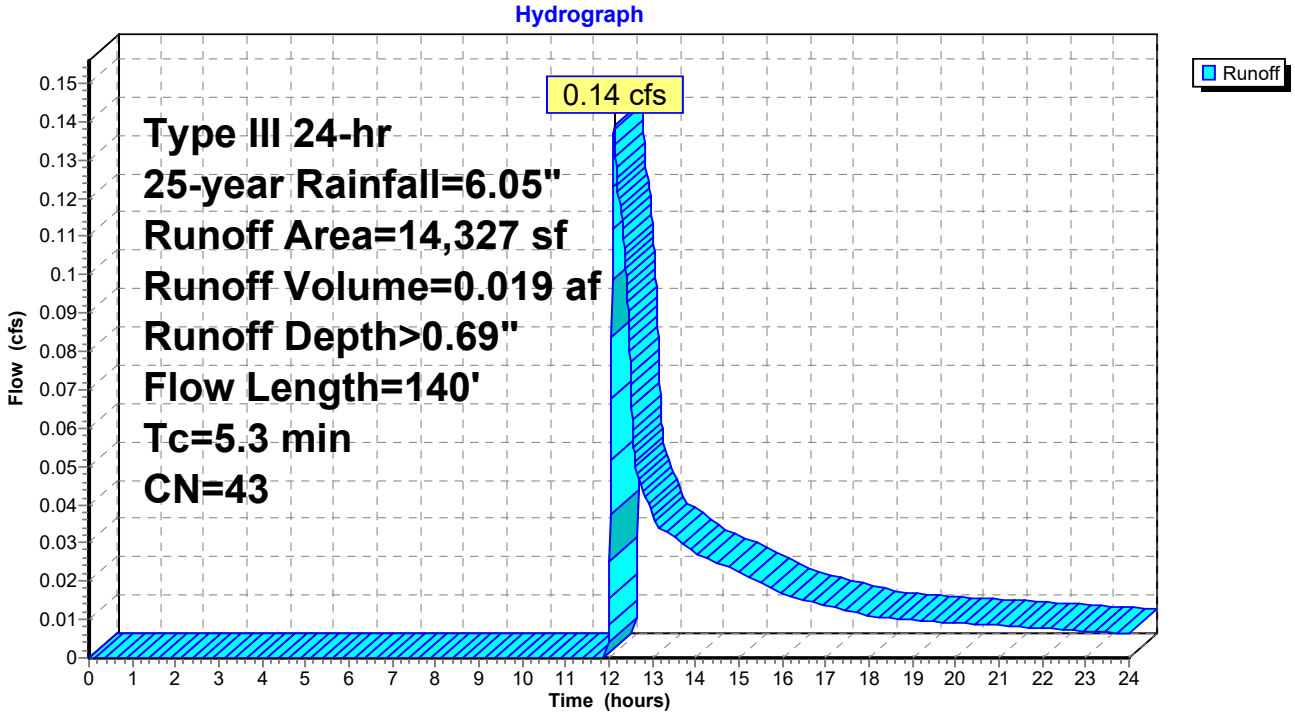
Runoff = 0.14 cfs @ 12.13 hrs, Volume= 0.019 af, Depth> 0.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-year Rainfall=6.05"

Area (sf)	CN	Description
939	98	Paved parking, HSG A
13,388	39	>75% Grass cover, Good, HSG A
14,327	43	Weighted Average
13,388		93.45% Pervious Area
939		6.55% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.6	50	0.0300	0.18		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.7	90	0.0900	2.10		**Shallow Concentrated Flow, B-C** Short Grass Pasture Kv= 7.0 fps
5.3	140	Total			

Subcatchment DA-1: Offsite Runoff to Main Street



Summary for Subcatchment DA-2: Offsite Runoff to Rear of Property

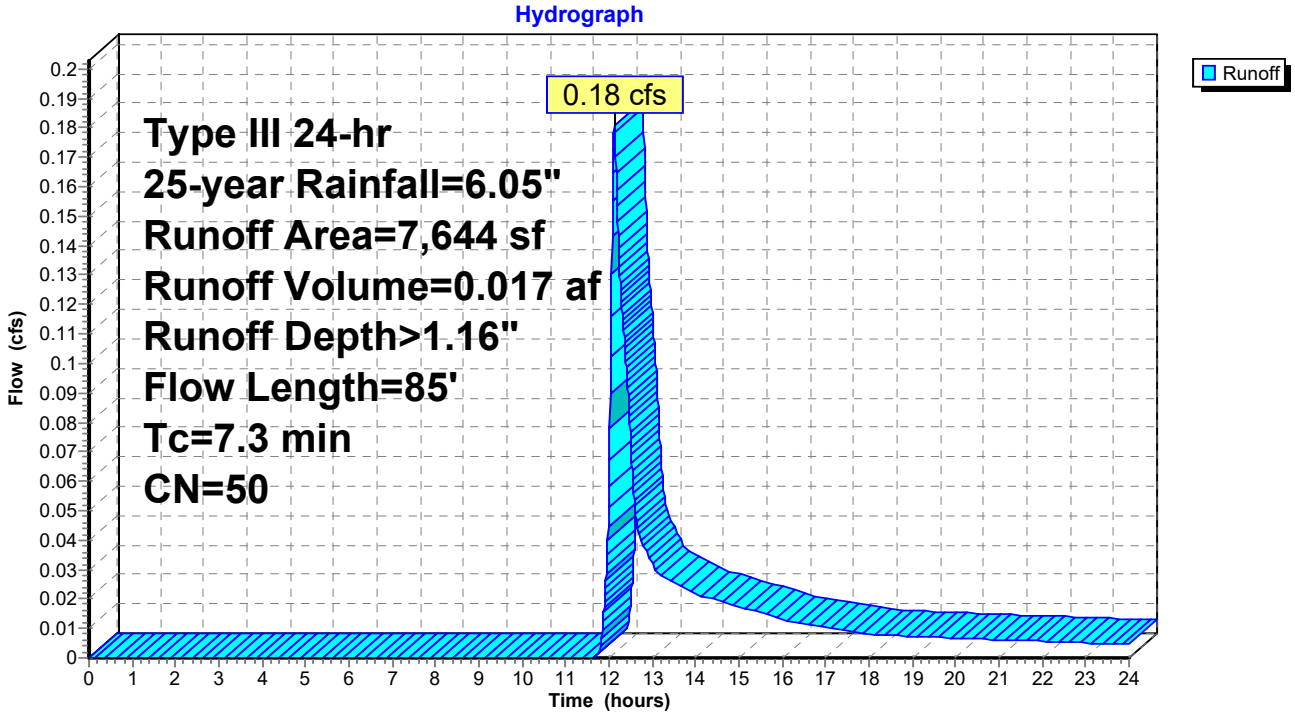
Runoff = 0.18 cfs @ 12.13 hrs, Volume= 0.017 af, Depth> 1.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-year Rainfall=6.05"

Area (sf)	CN	Description
6,180	39	>75% Grass cover, Good, HSG A
1,464	98	Paved parking, HSG A
7,644	50	Weighted Average
6,180		80.85% Pervious Area
1,464		19.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.7	50	0.0120	0.13		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.6	35	0.0170	0.91		**Shallow Concentrated Flow, B-C** Short Grass Pasture Kv= 7.0 fps
7.3	85	Total			

Subcatchment DA-2: Offsite Runoff to Rear of Property



Summary for Subcatchment DA-3: Onsite Runoff

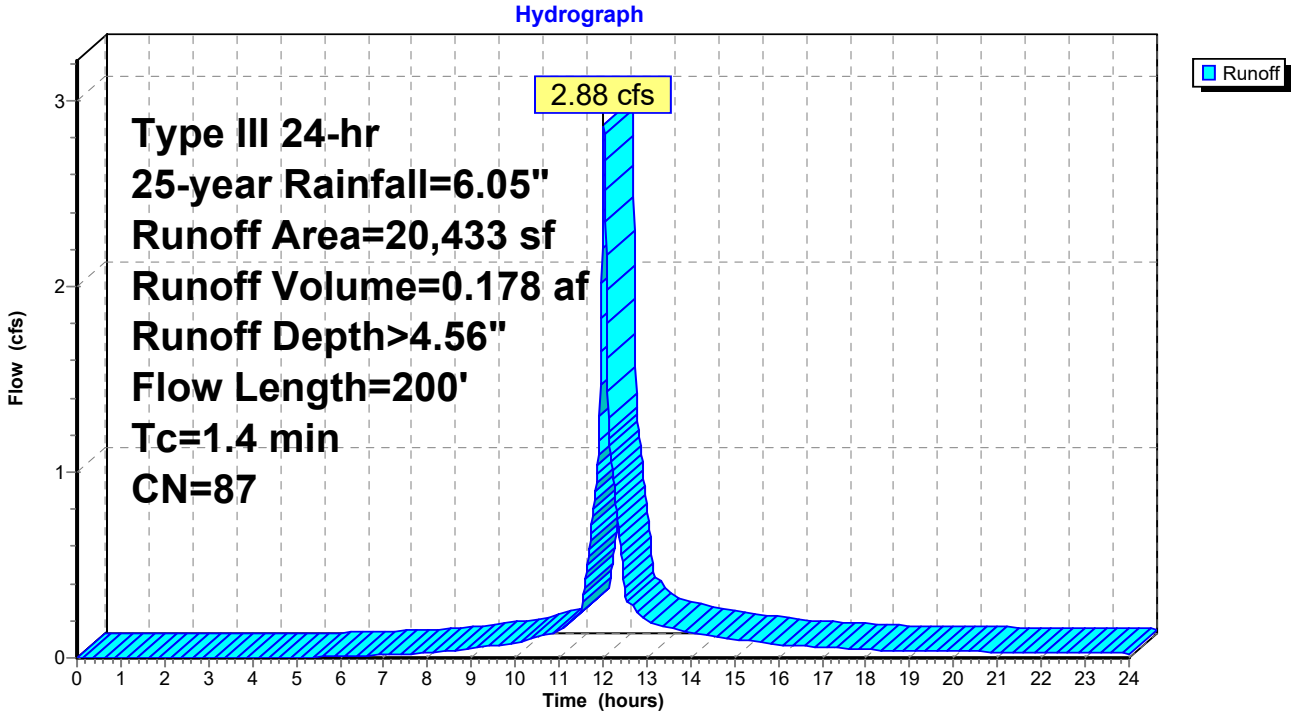
Runoff = 2.88 cfs @ 12.02 hrs, Volume= 0.178 af, Depth> 4.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-year Rainfall=6.05"

Area (sf)	CN	Description
16,576	98	Paved parking, HSG A
3,857	39	>75% Grass cover, Good, HSG A
20,433	87	Weighted Average
3,857		18.88% Pervious Area
16,576		81.12% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.7	50	0.0200	1.24		**Sheet Flow, A-B** Smooth surfaces n= 0.011 P2= 3.44"
0.7	150	0.0300	3.52		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
1.4	200	Total			

Subcatchment DA-3: Onsite Runoff



Summary for Subcatchment DA-4: Onsite Roof Runoff

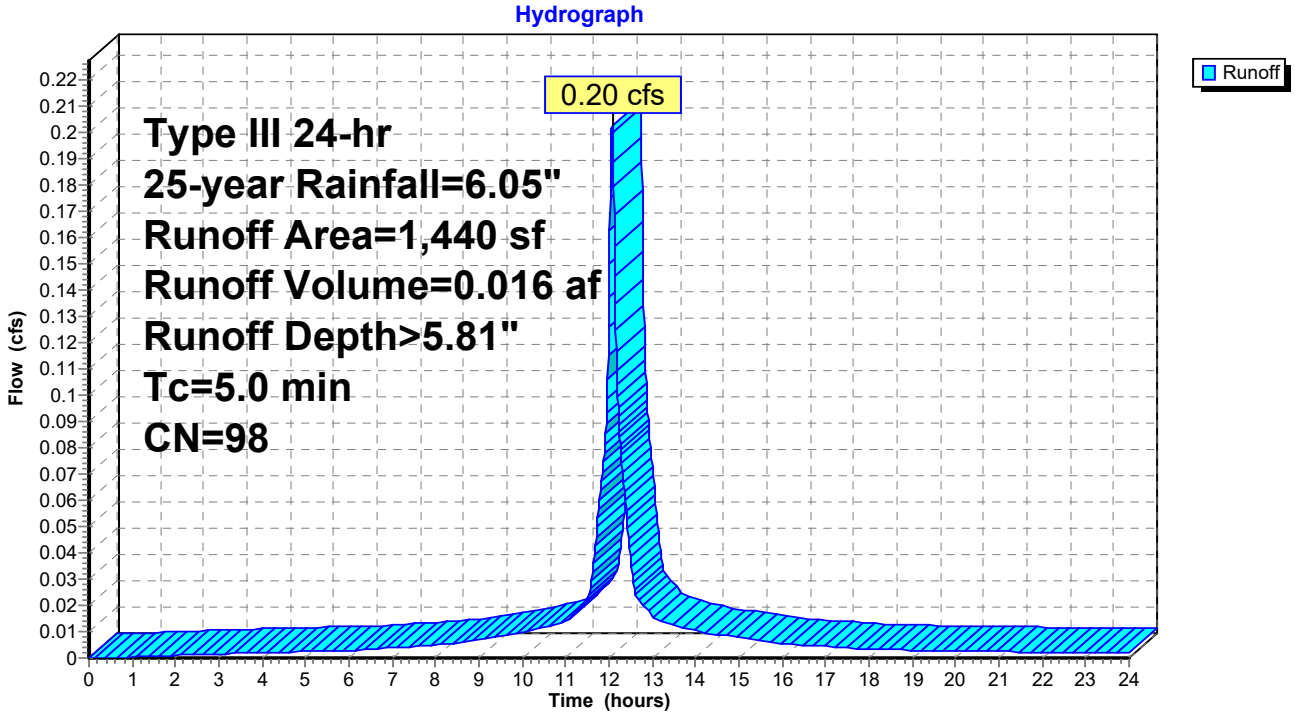
Runoff = 0.20 cfs @ 12.07 hrs, Volume= 0.016 af, Depth> 5.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-year Rainfall=6.05"

Area (sf)	CN	Description
1,440	98	Roofs, HSG A
1,440		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry, Roof Runoff**

Subcatchment DA-4: Onsite Roof Runoff



Summary for Subcatchment DA-5: Onsite Runoff

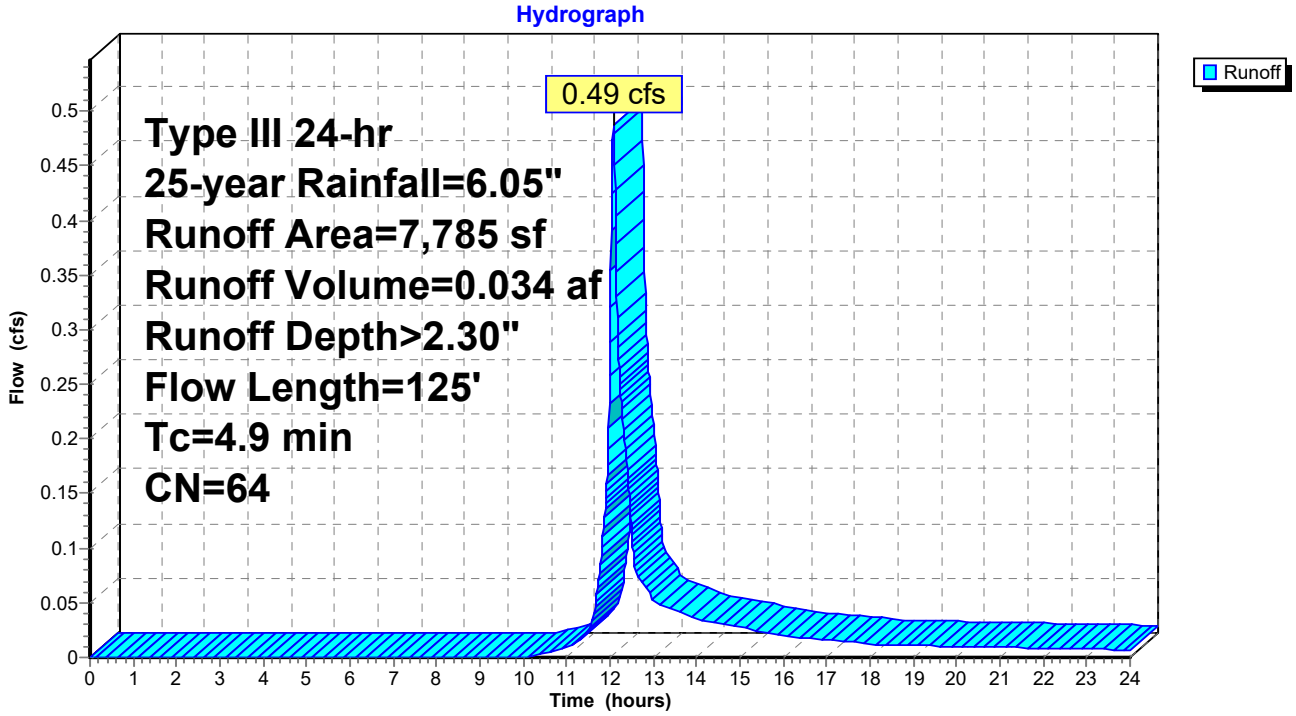
Runoff = 0.49 cfs @ 12.08 hrs, Volume= 0.034 af, Depth> 2.30"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-year Rainfall=6.05"

Area (sf)	CN	Description
1,440	98	Roofs, HSG A
1,875	98	Paved parking, HSG A
4,470	39	>75% Grass cover, Good, HSG A
7,785	64	Weighted Average
4,470		57.42% Pervious Area
3,315		42.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.5	50	0.0320	0.19		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.4	75	0.0270	3.34		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
4.9	125	Total			

Subcatchment DA-5: Onsite Runoff



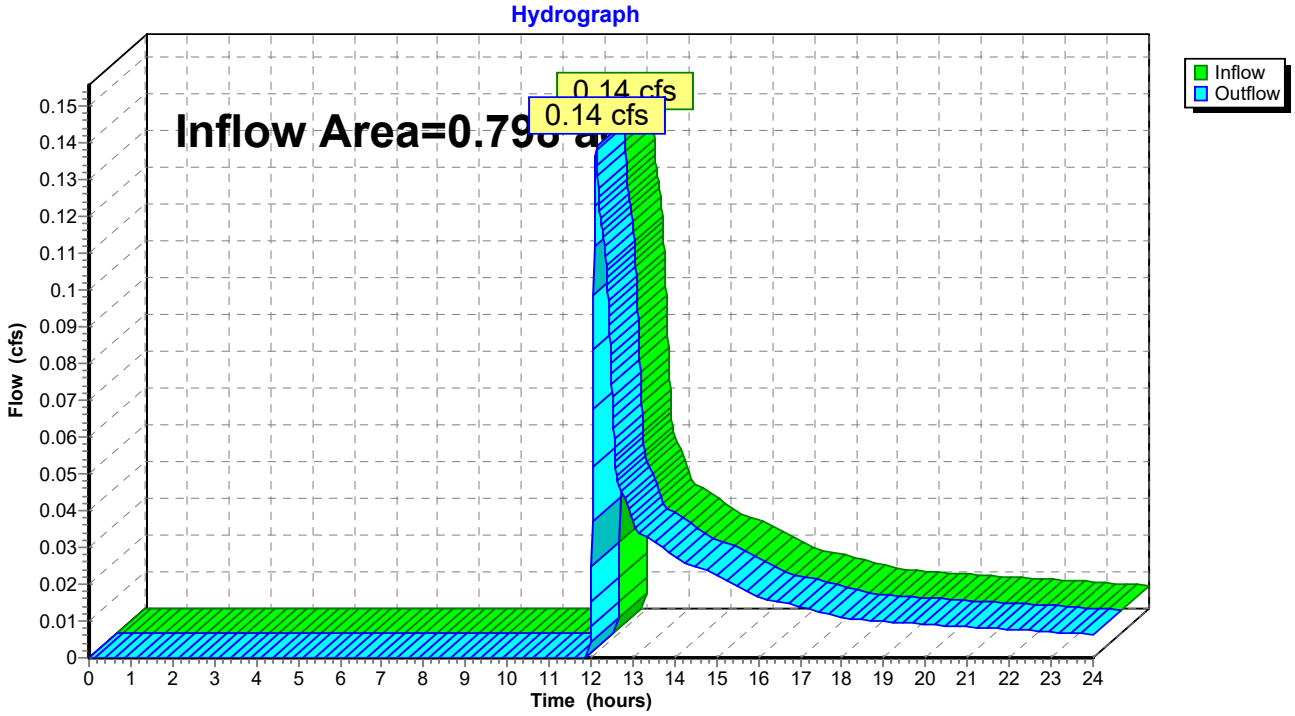
Summary for Reach 1R: Offsite Runoff to Main Street w/ Overflow

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.798 ac, 50.39% Impervious, Inflow Depth > 0.29" for 25-year event
Inflow = 0.14 cfs @ 12.13 hrs, Volume= 0.019 af
Outflow = 0.14 cfs @ 12.13 hrs, Volume= 0.019 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Reach 1R: Offsite Runoff to Main Street w/ Overflow



Summary for Pond 1P: Leaching Chambers

Inflow Area = 0.469 ac, 81.12% Impervious, Inflow Depth > 4.56" for 25-year event
Inflow = 2.88 cfs @ 12.02 hrs, Volume= 0.178 af
Outflow = 0.13 cfs @ 14.11 hrs, Volume= 0.142 af, Atten= 96%, Lag= 125.3 min
Discarded = 0.13 cfs @ 14.11 hrs, Volume= 0.142 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Peak Elev= 21.53' @ 14.11 hrs Surf.Area= 1,449 sf Storage= 3,942 cf

Plug-Flow detention time= 271.3 min calculated for 0.142 af (79% of inflow)
Center-of-Mass det. time= 194.9 min (984.9 - 790.0)

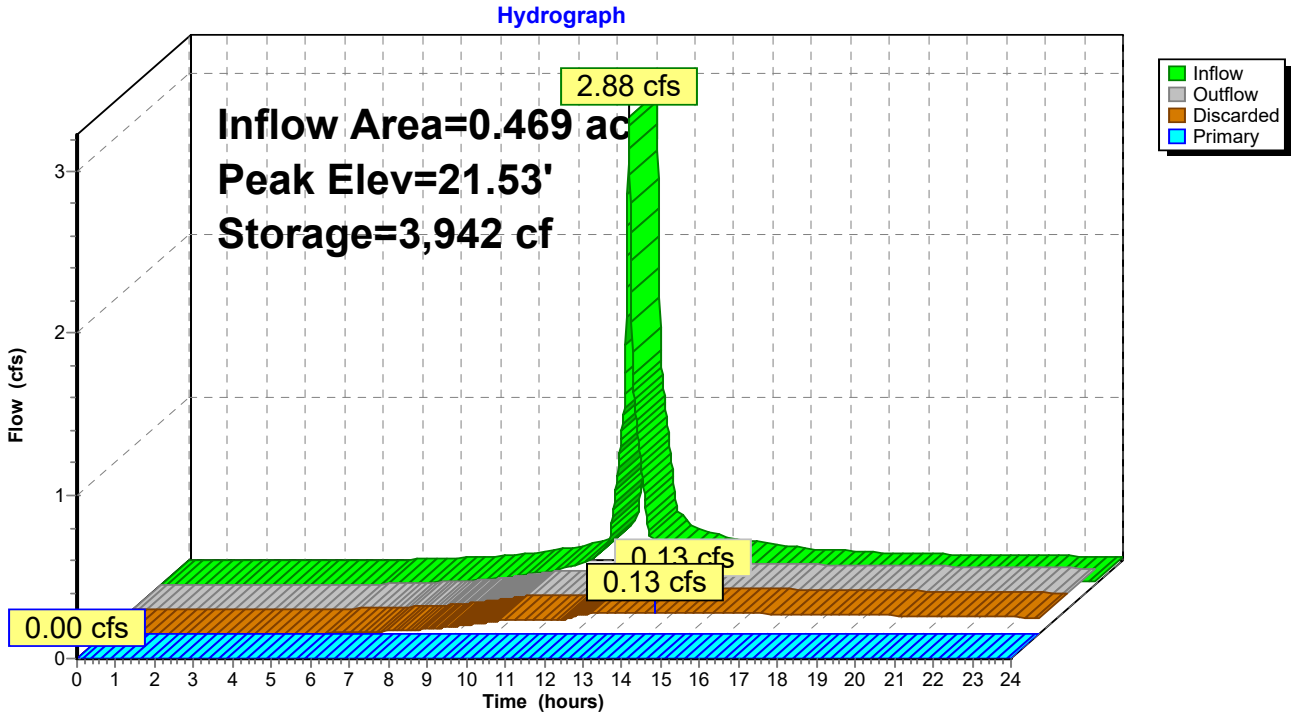
Volume	Invert	Avail.Storage	Storage Description
#1	16.50'	3,052 cf	**23.00'W x 63.00'L x 6.67'H Prismatoid** 9,665 cf Overall - 2,036 cf Embedded = 7,629 cf x 40.0% Voids
#2	16.50'	2,036 cf	**6.00'D x 6.00'H Vertical Cone/Cylinder** x 12 Inside #1
		5,087 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	16.50'	**2.410 in/hr Exfiltration over Wetted area**
#2	Primary	22.51'	**24.0" x 24.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.13 cfs @ 14.11 hrs HW=21.53' (Free Discharge)
↑—**1=Exfiltration** (Exfiltration Controls 0.13 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=16.50' (Free Discharge)
↑—**2=Orifice/Grate** (Controls 0.00 cfs)

Pond 1P: Leaching Chambers



Summary for Pond 2P: Drywell

Inflow Area = 0.033 ac,100.00% Impervious, Inflow Depth > 5.81" for 25-year event
Inflow = 0.20 cfs @ 12.07 hrs, Volume= 0.016 af
Outflow = 0.01 cfs @ 9.58 hrs, Volume= 0.012 af, Atten= 96%, Lag= 0.0 min
Discarded = 0.01 cfs @ 9.58 hrs, Volume= 0.012 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Peak Elev= 27.09' @ 14.87 hrs Surf.Area= 144 sf Storage= 342 cf

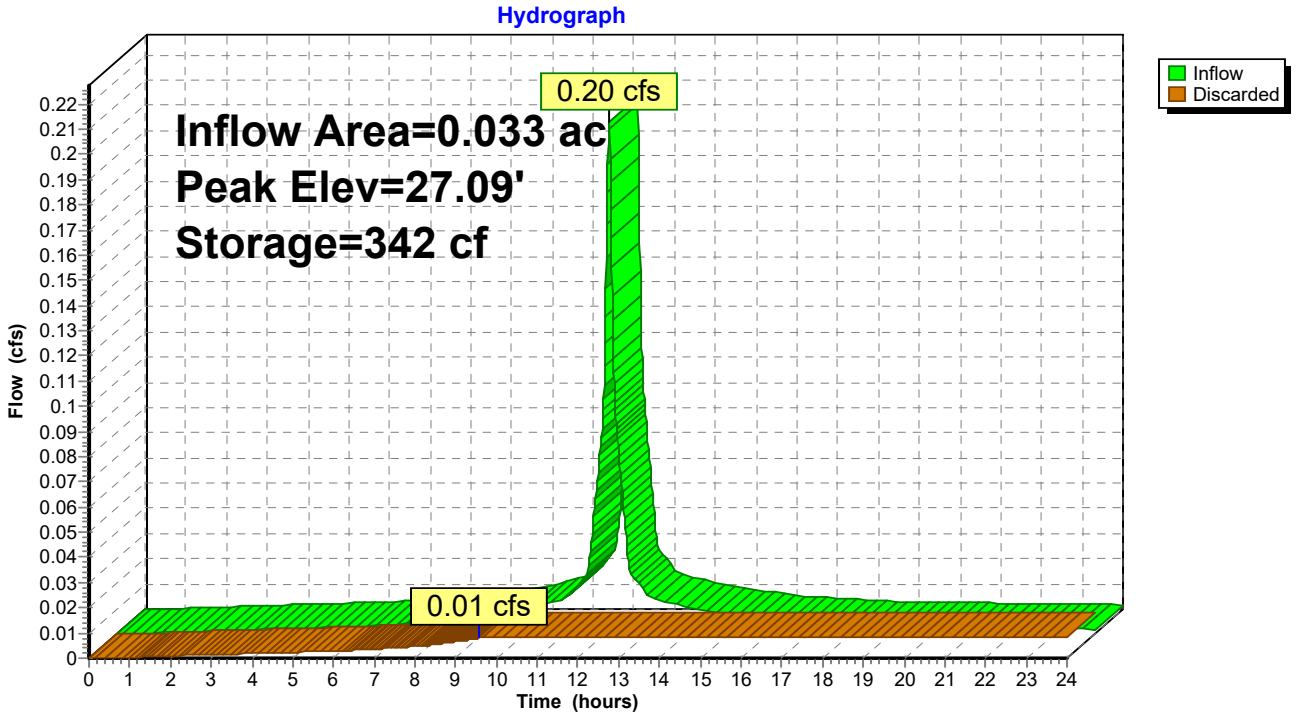
Plug-Flow detention time= 248.3 min calculated for 0.012 af (73% of inflow)
Center-of-Mass det. time= 157.6 min (901.3 - 743.6)

Volume	Invert	Avail.Storage	Storage Description
#1	22.50'	316 cf	**12.00'W x 12.00'L x 6.67'H Prismatoid** 960 cf Overall - 170 cf Embedded = 791 cf x 40.0% Voids
#2	22.50'	170 cf	**6.00'D x 6.00'H Vertical Cone/Cylinder** Inside #1
		486 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	22.50'	**2.410 in/hr Exfiltration over Surface area**

Discarded OutFlow Max=0.01 cfs @ 9.58 hrs HW=22.57' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.01 cfs)

Pond 2P: Drywell



Summary for Pond 3P: Leaching Chambers

Inflow Area = 0.179 ac, 42.58% Impervious, Inflow Depth > 2.30" for 25-year event
Inflow = 0.49 cfs @ 12.08 hrs, Volume= 0.034 af
Outflow = 0.02 cfs @ 11.63 hrs, Volume= 0.025 af, Atten= 95%, Lag= 0.0 min
Discarded = 0.02 cfs @ 11.63 hrs, Volume= 0.025 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Peak Elev= 26.02' @ 15.54 hrs Surf.Area= 420 sf Storage= 770 cf

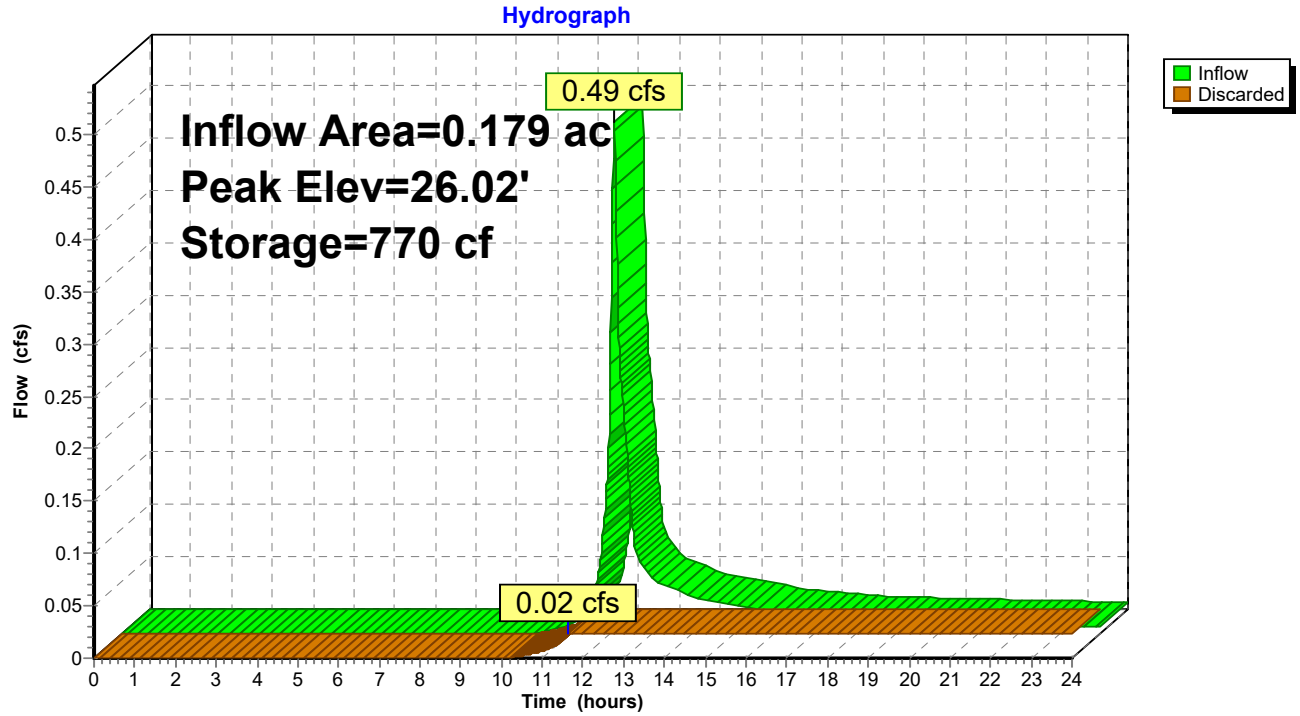
Plug-Flow detention time= 300.7 min calculated for 0.025 af (73% of inflow)
Center-of-Mass det. time= 204.3 min (1,054.7 - 850.5)

Volume	Invert	Avail.Storage	Storage Description
#1	22.50'	917 cf	**14.00'W x 30.00'L x 6.67'H Prismatoid** 2,801 cf Overall - 509 cf Embedded = 2,292 cf x 40.0% Voids
#2	22.50'	509 cf	**6.00'D x 6.00'H Vertical Cone/Cylinder** x 3 Inside #1
		1,426 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	22.50'	**2.410 in/hr Exfiltration over Surface area**

Discarded OutFlow Max=0.02 cfs @ 11.63 hrs HW=22.57' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.02 cfs)

Pond 3P: Leaching Chambers



Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment DA-1: Offsite Runoff to Main Runoff Area=14,327 sf 6.55% Impervious Runoff Depth>1.34"
Flow Length=140' Tc=5.3 min CN=43 Runoff=0.40 cfs 0.037 af

Subcatchment DA-2: Offsite Runoff to Rear Runoff Area=7,644 sf 19.15% Impervious Runoff Depth>2.00"
Flow Length=85' Tc=7.3 min CN=50 Runoff=0.35 cfs 0.029 af

Subcatchment DA-3: Onsite Runoff Runoff Area=20,433 sf 81.12% Impervious Runoff Depth>6.05"
Flow Length=200' Tc=1.4 min CN=87 Runoff=3.76 cfs 0.236 af

Subcatchment DA-4: Onsite Roof Runoff Runoff Area=1,440 sf 100.00% Impervious Runoff Depth>7.35"
Tc=5.0 min CN=98 Runoff=0.26 cfs 0.020 af

Subcatchment DA-5: Onsite Runoff Runoff Area=7,785 sf 42.58% Impervious Runoff Depth>3.45"
Flow Length=125' Tc=4.9 min CN=64 Runoff=0.75 cfs 0.051 af

Reach 1R: Offsite Runoff to Main Street w/ Overflow Inflow=0.80 cfs 0.058 af
Outflow=0.80 cfs 0.058 af

Pond 1P: Leaching Chambers Peak Elev=22.59' Storage=4,751 cf Inflow=3.76 cfs 0.236 af
Discarded=0.14 cfs 0.159 af Primary=0.61 cfs 0.021 af Outflow=0.75 cfs 0.180 af

Pond 2P: Drywell Peak Elev=28.99' Storage=476 cf Inflow=0.26 cfs 0.020 af
Outflow=0.01 cfs 0.012 af

Pond 3P: Leaching Chambers Peak Elev=28.70' Storage=1,347 cf Inflow=0.75 cfs 0.051 af
Outflow=0.02 cfs 0.026 af

Total Runoff Area = 1.185 ac Runoff Volume = 0.374 af Average Runoff Depth = 3.79"
54.03% Pervious = 0.640 ac 45.97% Impervious = 0.545 ac

Summary for Subcatchment DA-1: Offsite Runoff to Main Street

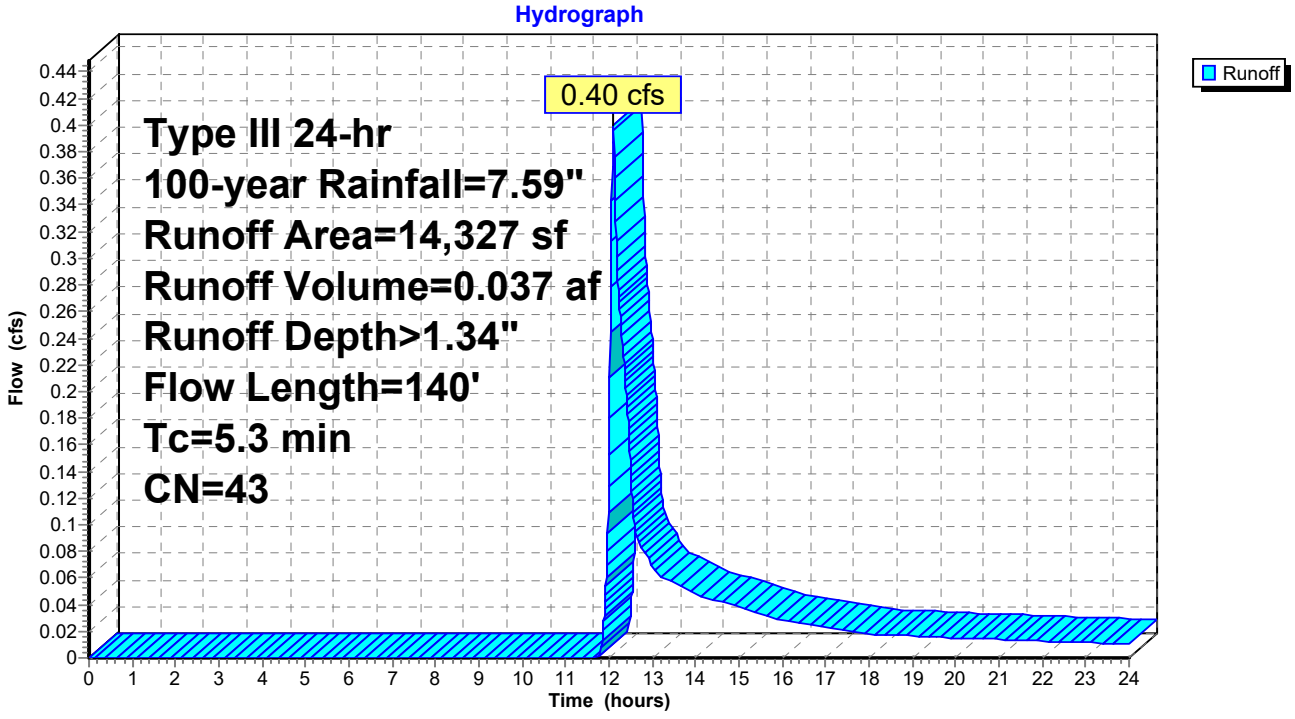
Runoff = 0.40 cfs @ 12.10 hrs, Volume= 0.037 af, Depth> 1.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.59"

Area (sf)	CN	Description
939	98	Paved parking, HSG A
13,388	39	>75% Grass cover, Good, HSG A
14,327	43	Weighted Average
13,388		93.45% Pervious Area
939		6.55% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.6	50	0.0300	0.18		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.7	90	0.0900	2.10		**Shallow Concentrated Flow, B-C** Short Grass Pasture Kv= 7.0 fps
5.3	140	Total			

Subcatchment DA-1: Offsite Runoff to Main Street



Summary for Subcatchment DA-2: Offsite Runoff to Rear of Property

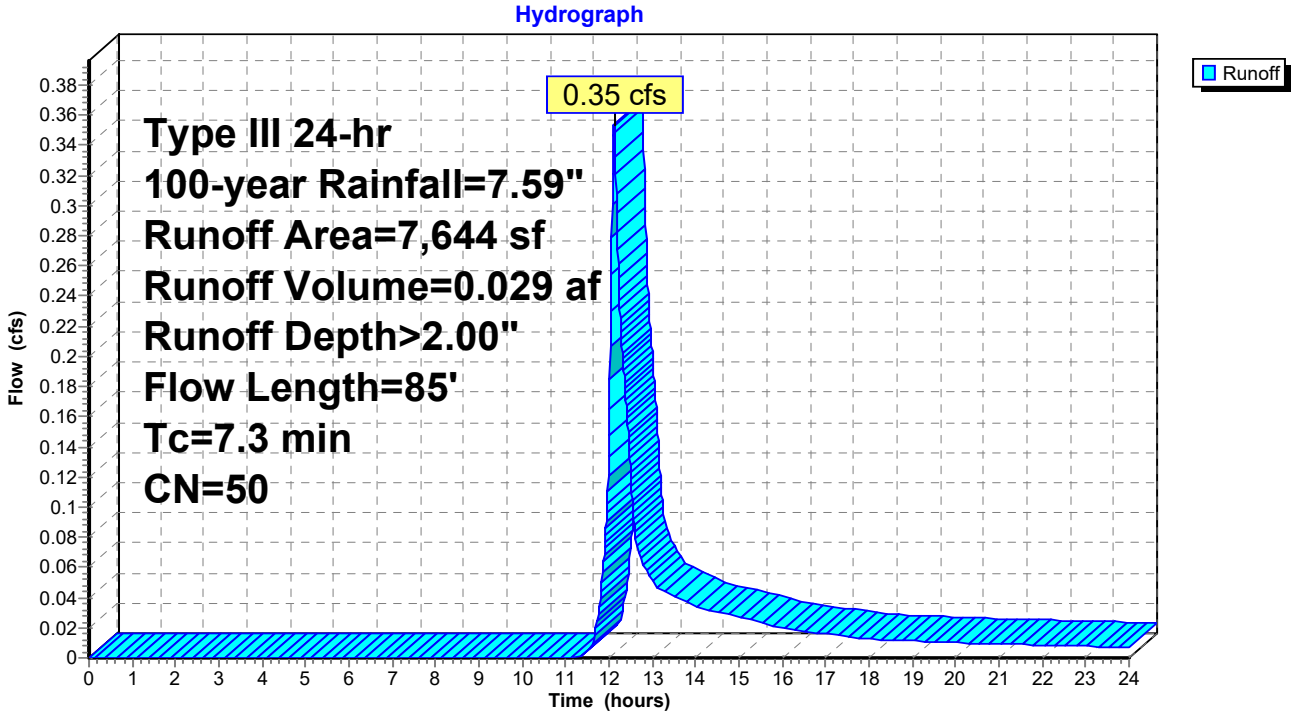
Runoff = 0.35 cfs @ 12.12 hrs, Volume= 0.029 af, Depth> 2.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.59"

Area (sf)	CN	Description
6,180	39	>75% Grass cover, Good, HSG A
1,464	98	Paved parking, HSG A
7,644	50	Weighted Average
6,180		80.85% Pervious Area
1,464		19.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.7	50	0.0120	0.13		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.6	35	0.0170	0.91		**Shallow Concentrated Flow, B-C** Short Grass Pasture Kv= 7.0 fps
7.3	85	Total			

Subcatchment DA-2: Offsite Runoff to Rear of Property



Summary for Subcatchment DA-3: Onsite Runoff

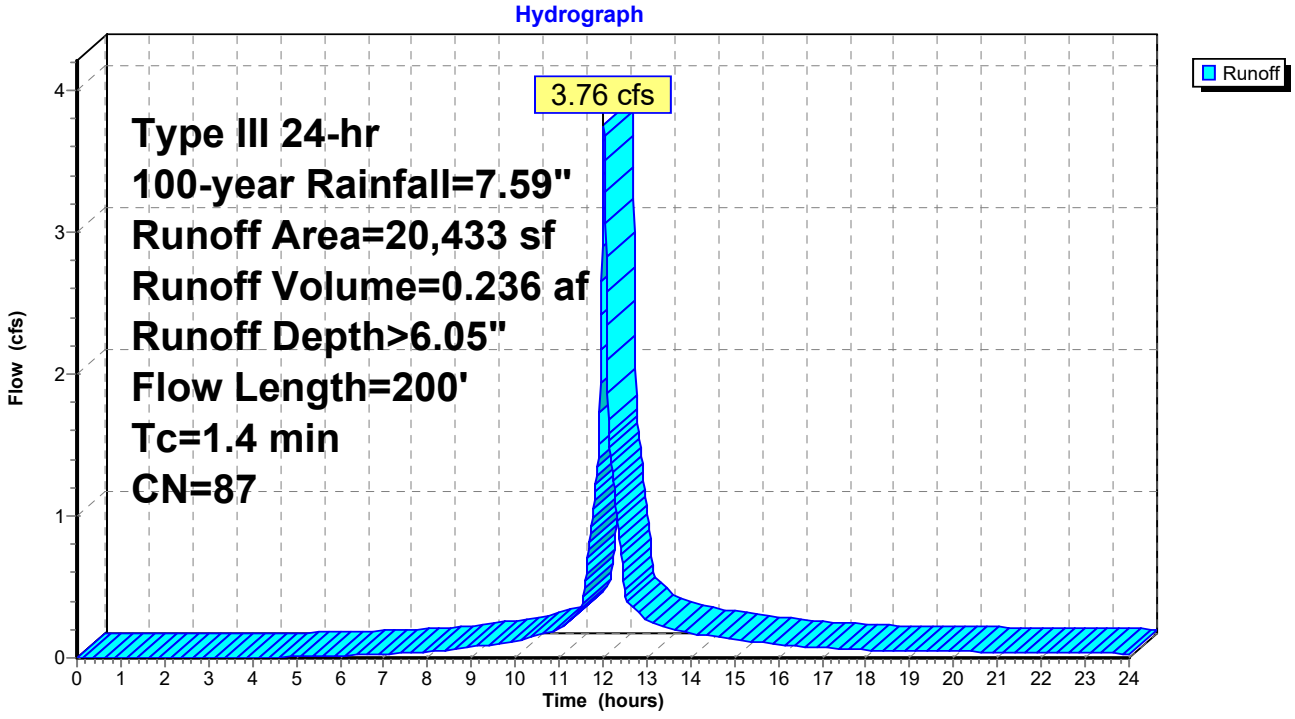
Runoff = 3.76 cfs @ 12.02 hrs, Volume= 0.236 af, Depth> 6.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.59"

Area (sf)	CN	Description
16,576	98	Paved parking, HSG A
3,857	39	>75% Grass cover, Good, HSG A
20,433	87	Weighted Average
3,857		18.88% Pervious Area
16,576		81.12% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.7	50	0.0200	1.24		**Sheet Flow, A-B** Smooth surfaces n= 0.011 P2= 3.44"
0.7	150	0.0300	3.52		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
1.4	200	Total			

Subcatchment DA-3: Onsite Runoff



Summary for Subcatchment DA-4: Onsite Roof Runoff

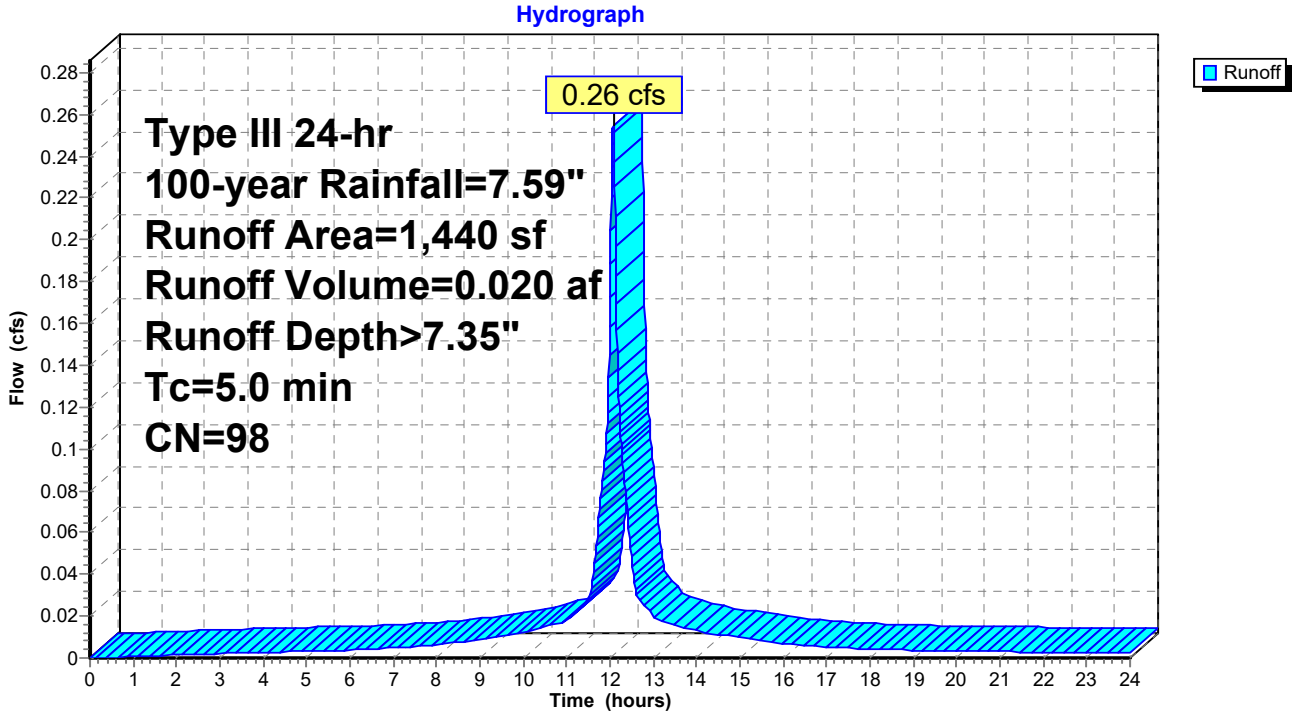
Runoff = 0.26 cfs @ 12.07 hrs, Volume= 0.020 af, Depth> 7.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.59"

Area (sf)	CN	Description
1,440	98	Roofs, HSG A
1,440		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry, Roof Runoff**

Subcatchment DA-4: Onsite Roof Runoff



Summary for Subcatchment DA-5: Onsite Runoff

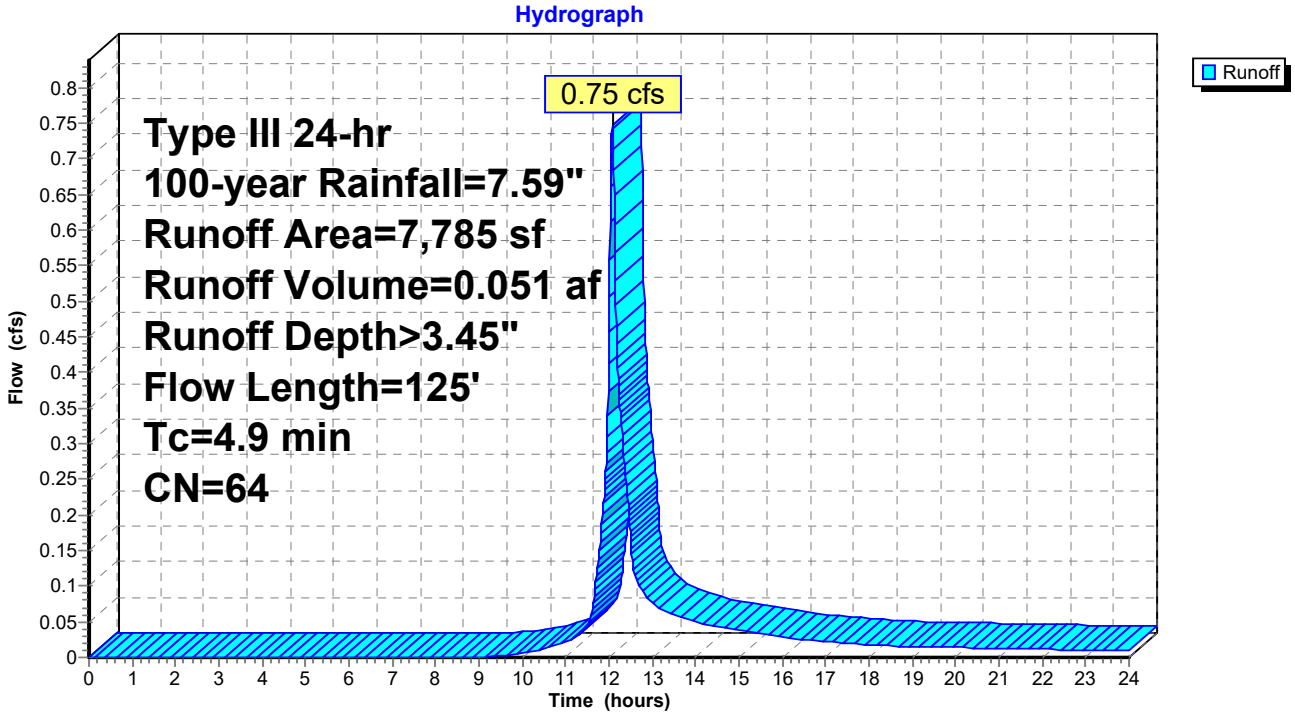
Runoff = 0.75 cfs @ 12.08 hrs, Volume= 0.051 af, Depth> 3.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.59"

Area (sf)	CN	Description
1,440	98	Roofs, HSG A
1,875	98	Paved parking, HSG A
4,470	39	>75% Grass cover, Good, HSG A
7,785	64	Weighted Average
4,470		57.42% Pervious Area
3,315		42.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.5	50	0.0320	0.19		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.44"
0.4	75	0.0270	3.34		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
4.9	125	Total			

Subcatchment DA-5: Onsite Runoff



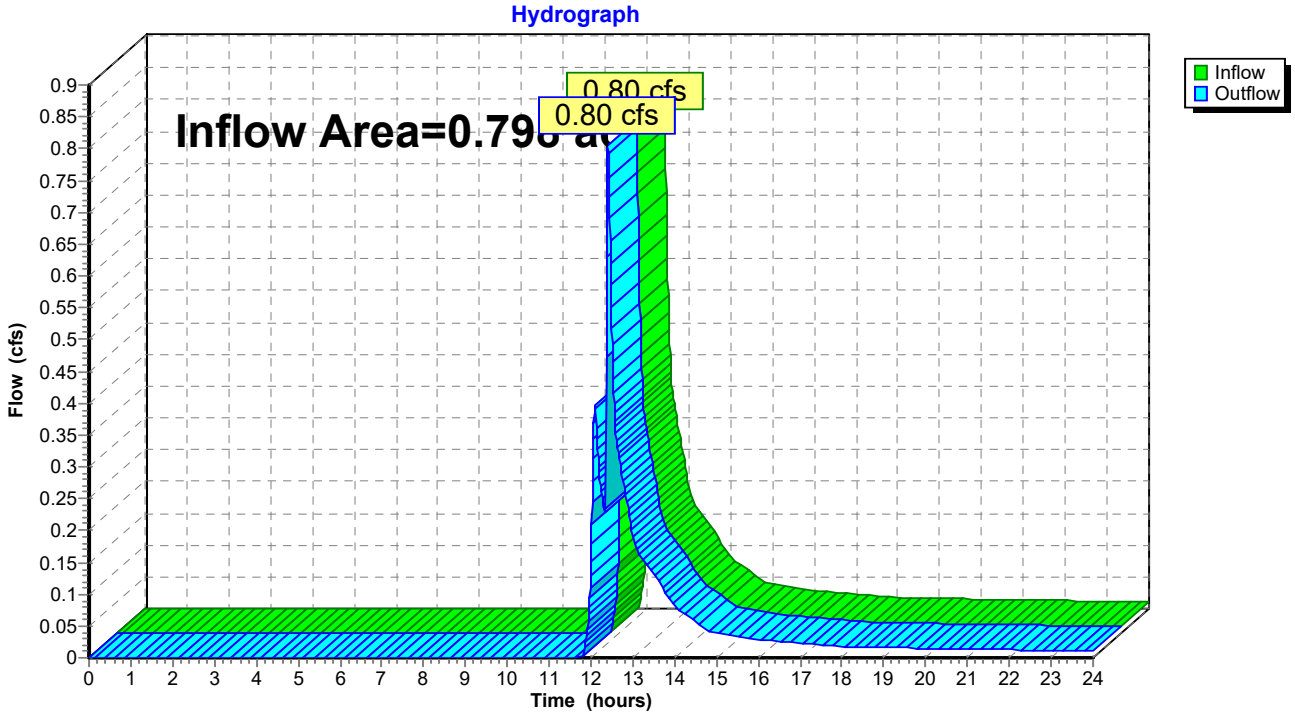
Summary for Reach 1R: Offsite Runoff to Main Street w/ Overflow

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.798 ac, 50.39% Impervious, Inflow Depth > 0.87" for 100-year event
Inflow = 0.80 cfs @ 12.41 hrs, Volume= 0.058 af
Outflow = 0.80 cfs @ 12.41 hrs, Volume= 0.058 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Reach 1R: Offsite Runoff to Main Street w/ Overflow



Summary for Pond 1P: Leaching Chambers

Inflow Area = 0.469 ac, 81.12% Impervious, Inflow Depth > 6.05" for 100-year event
Inflow = 3.76 cfs @ 12.02 hrs, Volume= 0.236 af
Outflow = 0.75 cfs @ 12.41 hrs, Volume= 0.180 af, Atten= 80%, Lag= 23.2 min
Discarded = 0.14 cfs @ 12.41 hrs, Volume= 0.159 af
Primary = 0.61 cfs @ 12.41 hrs, Volume= 0.021 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Peak Elev= 22.59' @ 12.41 hrs Surf.Area= 1,449 sf Storage= 4,751 cf

Plug-Flow detention time= 247.8 min calculated for 0.180 af (76% of inflow)
Center-of-Mass det. time= 165.5 min (947.8 - 782.3)

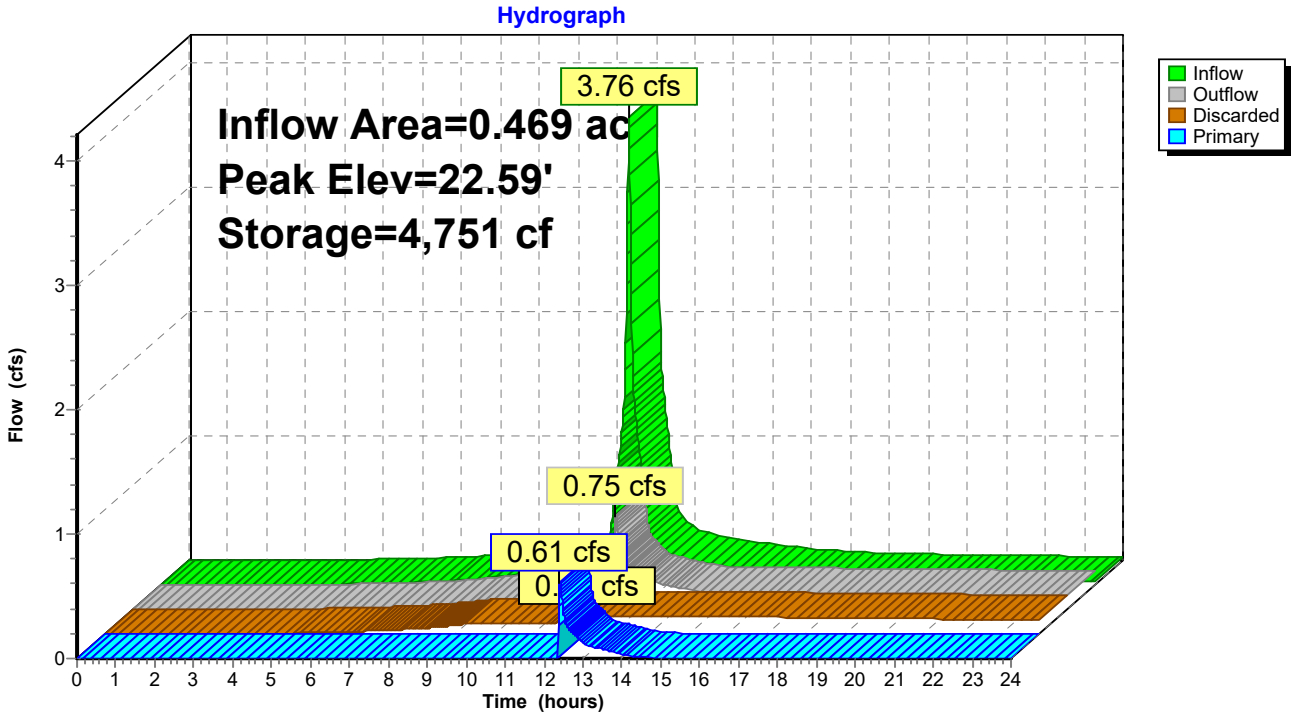
Volume	Invert	Avail.Storage	Storage Description
#1	16.50'	3,052 cf	**23.00'W x 63.00'L x 6.67'H Prismatoid** 9,665 cf Overall - 2,036 cf Embedded = 7,629 cf x 40.0% Voids
#2	16.50'	2,036 cf	**6.00'D x 6.00'H Vertical Cone/Cylinder** x 12 Inside #1
		5,087 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	16.50'	**2.410 in/hr Exfiltration over Wetted area**
#2	Primary	22.51'	**24.0" x 24.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.14 cfs @ 12.41 hrs HW=22.59' (Free Discharge)
↑**1=Exfiltration** (Exfiltration Controls 0.14 cfs)

Primary OutFlow Max=0.58 cfs @ 12.41 hrs HW=22.59' (Free Discharge)
↑**2=Orifice/Grate** (Weir Controls 0.58 cfs @ 0.92 fps)

Pond 1P: Leaching Chambers



Summary for Pond 2P: Drywell

Inflow Area = 0.033 ac,100.00% Impervious, Inflow Depth > 7.35" for 100-year event
Inflow = 0.26 cfs @ 12.07 hrs, Volume= 0.020 af
Outflow = 0.01 cfs @ 8.84 hrs, Volume= 0.012 af, Atten= 97%, Lag= 0.0 min
Discarded = 0.01 cfs @ 8.84 hrs, Volume= 0.012 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Peak Elev= 28.99' @ 15.57 hrs Surf.Area= 144 sf Storage= 476 cf

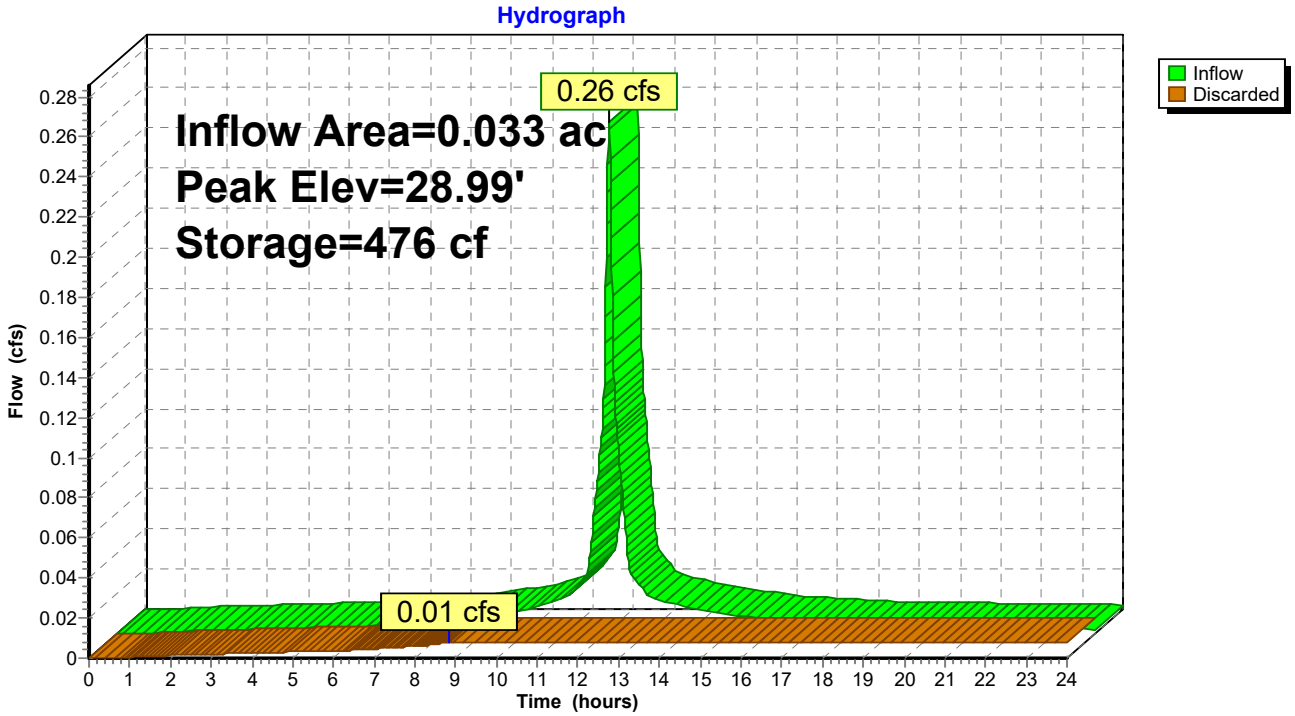
Plug-Flow detention time= 246.6 min calculated for 0.012 af (60% of inflow)
Center-of-Mass det. time= 134.9 min (875.4 - 740.5)

Volume	Invert	Avail.Storage	Storage Description
#1	22.50'	316 cf	**12.00'W x 12.00'L x 6.67'H Prismatoid** 960 cf Overall - 170 cf Embedded = 791 cf x 40.0% Voids
#2	22.50'	170 cf	**6.00'D x 6.00'H Vertical Cone/Cylinder** Inside #1
		486 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	22.50'	**2.410 in/hr Exfiltration over Surface area**

Discarded OutFlow Max=0.01 cfs @ 8.84 hrs HW=22.57' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.01 cfs)

Pond 2P: Drywell



Summary for Pond 3P: Leaching Chambers

Inflow Area = 0.179 ac, 42.58% Impervious, Inflow Depth > 3.45" for 100-year event
Inflow = 0.75 cfs @ 12.08 hrs, Volume= 0.051 af
Outflow = 0.02 cfs @ 11.23 hrs, Volume= 0.026 af, Atten= 97%, Lag= 0.0 min
Discarded = 0.02 cfs @ 11.23 hrs, Volume= 0.026 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Peak Elev= 28.70' @ 16.73 hrs Surf.Area= 420 sf Storage= 1,347 cf

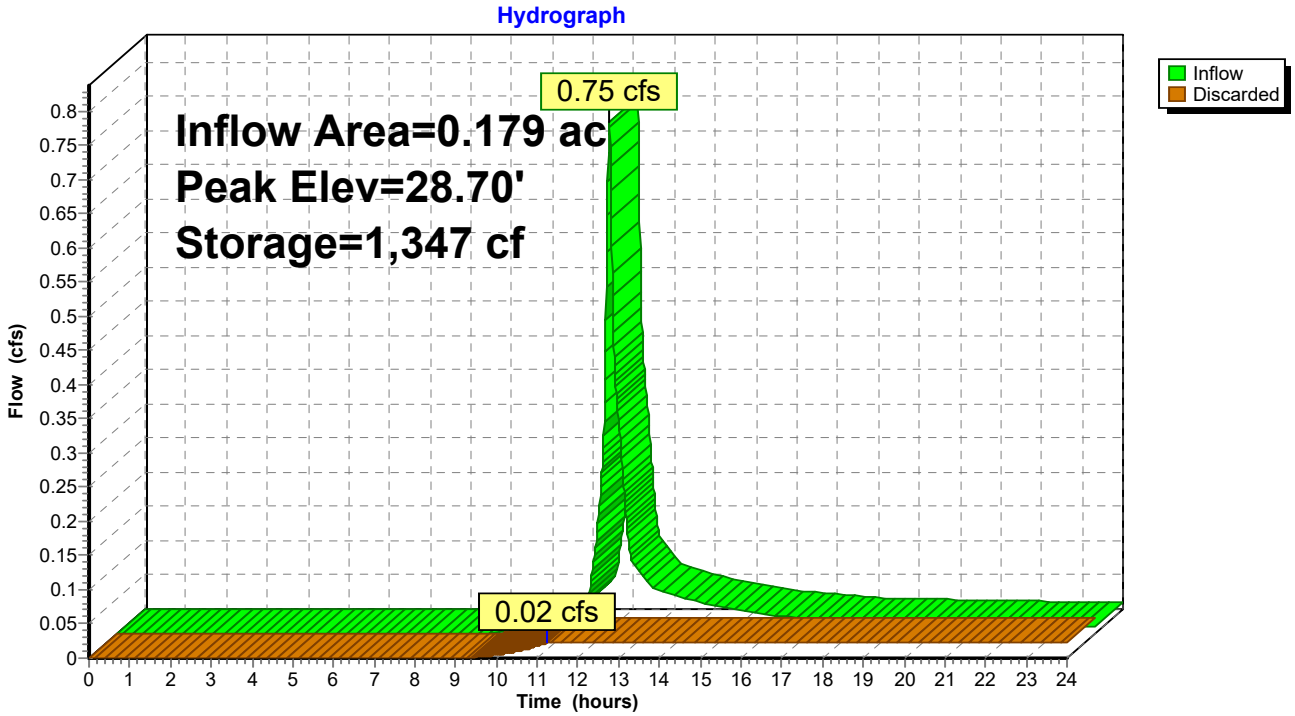
Plug-Flow detention time= 316.0 min calculated for 0.026 af (51% of inflow)
Center-of-Mass det. time= 196.5 min (1,034.9 - 838.4)

Volume	Invert	Avail.Storage	Storage Description
#1	22.50'	917 cf	**14.00'W x 30.00'L x 6.67'H Prismatoid** 2,801 cf Overall - 509 cf Embedded = 2,292 cf x 40.0% Voids
#2	22.50'	509 cf	**6.00'D x 6.00'H Vertical Cone/Cylinder** x 3 Inside #1
		1,426 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	22.50'	**2.410 in/hr Exfiltration over Surface area**

Discarded OutFlow Max=0.02 cfs @ 11.23 hrs HW=22.57' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.02 cfs)

Pond 3P: Leaching Chambers



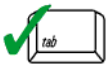
DEP STORMWATER MANAGEMENT FORMS

Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.[1] This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8[2]
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

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[1] The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

[2] For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.

Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

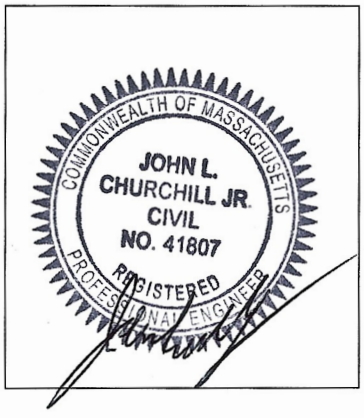
Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



9/13/23

Signature and Date

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

☐ New development

☐ Redevelopment

☒ Mix of New Development and Redevelopment

Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- ☒ No disturbance to any Wetland Resource Areas
- ☐ Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- ☐ Reduced Impervious Area (Redevelopment Only)
- ☒ Minimizing disturbance to existing trees and shrubs
- ☐ LID Site Design Credit Requested:
  - ☐ Credit 1
  - ☐ Credit 2
  - ☐ Credit 3
- ☐ Use of “country drainage” versus curb and gutter conveyance and pipe
- ☐ Bioretention Cells (includes Rain Gardens)
- ☐ Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- ☐ Treebox Filter
- ☐ Water Quality Swale
- ☐ Grass Channel
- ☐ Green Roof
- ☐ Other (describe): _____

Standard 1: No New Untreated Discharges

- ☒ No new untreated discharges
- ☒ Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- ☒ Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.

Checklist for Stormwater Report

Checklist (continued)

Standard 2: Peak Rate Attenuation

☐ Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.

☒ Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.

☒ Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

☒ Soil Analysis provided.

☒ Required Recharge Volume calculation provided.

☐ Required Recharge volume reduced through use of the LID site Design Credits.

☒ Sizing the infiltration, BMPs is based on the following method: Check the method used.

☒ Static ☐ Simple Dynamic ☐ Dynamic Field[1]

☐ Runoff from all impervious areas at the site discharging to the infiltration BMP.

☒ Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.

☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume.

☒ Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:

- ☐ Site is comprised solely of C and D soils and/or bedrock at the land surface
- ☐ M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
- ☐ Solid Waste Landfill pursuant to 310 CMR 19.000
- ☒ Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.

☒ Calculations showing that the infiltration BMPs will drain in 72 hours are provided.

☐ Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

[1] 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.

Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

☐ The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.

☐ Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.

☒ A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.

☒ Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:

- ☐ is within the Zone II or Interim Wellhead Protection Area
- ☐ is near or to other critical areas
- ☒ is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
- ☐ involves runoff from land uses with higher potential pollutant loads.

☐ The Required Water Quality Volume is reduced through use of the LID site Design Credits.

☒ Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.

Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

☒ The BMP is sized (and calculations provided) based on:

- ☒ The ½" or 1" Water Quality Volume or
- ☐ The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.

☐ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.

☐ A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

☐ The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.

☐ The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted ***prior to*** the discharge of stormwater to the post-construction stormwater BMPs.

☐ The NPDES Multi-Sector General Permit does ***not*** cover the land use.

☐ LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.

☐ All exposure has been eliminated.

☐ All exposure has ***not*** been eliminated and all BMPs selected are on MassDEP LUHPPL list.

☐ The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

☐ The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.

☐ Critical areas and BMPs are identified in the Stormwater Report.

Checklist for Stormwater Report

Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

☒ The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:

- ☐ Limited Project
- ☐ Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
- ☐ Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
- ☐ Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
- ☐ Bike Path and/or Foot Path
- ☐ Redevelopment Project
- ☒ Redevelopment portion of mix of new and redevelopment.

☐ Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.

☒ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.

☒ A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.

Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has ***not*** been included in the Stormwater Report but will be submitted ***before*** land disturbance begins.

☐ The project is ***not*** covered by a NPDES Construction General Permit.

☐ The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.

☐ The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

☒ The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:

- ☒ Name of the stormwater management system owners;
- ☒ Party responsible for operation and maintenance;
- ☒ Schedule for implementation of routine and non-routine maintenance tasks;
- ☒ Plan showing the location of all stormwater BMPs maintenance access areas;
- ☐ Description and delineation of public safety features;
- ☐ Estimated operation and maintenance budget; and
- ☐ Operation and Maintenance Log Form.

☐ The responsible party is ***not*** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:

- ☐ A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
- ☐ A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

☒ The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;

☒ An Illicit Discharge Compliance Statement is attached;

☐ NO Illicit Discharge Compliance Statement is attached but will be submitted ***prior to*** the discharge of any stormwater to post-construction BMPs.

STORMWATER OPERATIONS AND MAINTENANCE PLAN

Stormwater Operations and Maintenance Plan

DATE: September 13, 2023

Responsible Party:
Angela Mckeown
484 Liberty Street
Rockland, MA 02370

Project Address:
386 Main Street
Wareham, MA 02571

Engineering By:
JC Engineering, Inc.
2854 Cranberry Highway
East Wareham, MA 02538

The project site will be owned and maintained by the owner of the property. The owner will be responsible for the required inspections and maintenance of the drainage system.

Illicit Discharges
All Illicit discharges to the stormwater management system are prohibited.

Responsible Party Date

Deep Sump Catch Basin
The Deep sump catch basins shall be inspected by the owner/operator on a quarterly basis or after a major storm event. Catch basins sumps will be cleaned annually during the early spring or when the sediment rises to within half the available sump height of the catch basin, whichever comes first.

Infiltration Structures
Once the system is operational, inspections of the leaching structures should occur after every major storm event for the first few months. After the system is in operation, inspections should be every six months. Special attention should be directed towards the depth of sediment in the leaching structures. There should be no accumulation of sediment within the leaching structures. Silt and debris are to be removed using vacuum pumping techniques as required.

Pre-Construction Requirements

Prior to the start of any construction on the site the following procedures are to be implemented.

- Erosion control line to be installed down gradient of all earthwork proposed in that particular phase of work. Erosion control line is to be installed at the limit of work as shown on the plans.

- All major trees designated to be saved are to be flagged in the field and fenced off as necessary to prevent damage during construction.

- A temporary settling pool is to be constructed on the up gradient side of erosion control barrier at the limit of work such that stormwater runoff is channeled to the temporary settling pool and filtered through the hay bales prior to leaving the site.

- Safety barriers, warnings and fences to be installed along Main Street as necessary to protect the general public prior to the start of the work adjacent to the roadway.

- A temporary construction entrance is to be constructed at the access point to the site. The entrance is to be stabilized in such a manner as to prevent the tracking of soil onto the public way.

- A dust monitoring plan will be established prior to the start of construction.

- Weekly training sessions will be conducted for all site contractors at the job.

- A person responsible for daily inspection of all erosion control methodologies and action plan for corrections/repairs when needed will be established.

Construction Period Pollution Prevention

- The contractor must install erosion control measures as shown on the plans and details prior to starting any other work on the site construction. Erosion control must be installed at every inlet structure and inlet swale and maintained for the duration of the project. Erosion control as shown on plans shall be inspected, repaired and/or maintained by the contractor weekly and within 12 hours of each storm event.

- Water and/or covers to minimize dust and erosion from newly graded areas and stock piles of earth will be implemented during construction as needed or when conditions are anticipated to be greater than 20 m.p.h. Application rate of water shall be sufficient to moisten soil so as to not create runoff and/or ponding. No surfactants shall be used.

- A regular street sweeping schedule of hard surfaces will be established prior to construction and will be continued until the completion of the full site development.

- A person will be assigned to monitor the perimeter erosion control methodologies on a daily basis.

- Owner or its representative shall perform weekly review/training sessions.

- Construction of a temporary settling area is to be utilized as a method of controlling concentrated flows from areas that are under construction.

- Temporary settling areas are to be constructed on an as needed basis and located throughout the construction phase as required by earthwork activities.

- At the beginning of earthwork operations on the site a mechanical on-site sweeper is to be maintained such that the public way can be kept clean during the construction phase.

- As elements of the drainage system are installed, silt fences and “silt sacs” are to be installed around all catch basins and under grates until the tributary area to that basin is completely stabilized.

- As general earthwork is completed the exterior perimeters of the areas that have been completed are to be stabilized using erosion control grass.

- Stabilize slopes steeper than 3:1 (horizontal to vertical) with seed, secured geotextile fabric, or rock rip-rap as required to prevent erosion during construction.

- Sediment shall be contained within the construction site and shall be removed when they reach a depth of 6 inches.

- Clean out catch basins, drain manholes and storm drain pipes after completion of construction.

- No stormwater shall be allowed to enter the structures until all catchbasins, drain manholes and stormdrain pipes have been cleaned, the binder course is installed and all disturbed areas are stabilized.

- If the binder course is in place for more than 3 months without a wearing course, the contractor shall set the rim elevation of the drainage structures level with the binder course. The rim elevations shall be reset just prior to placing the wearing course.

- The contractor is responsible for all stormwater best management practices being in place to contain stormwater in the event that drainage structures are not at pavement grade during a storm event, and all cleanup in the event that such measures fail during said storm event.

- Temporary surfaces should be stabilized with as soon as active grading is suspended. Temporary measures include seeding with grass, jute netting, or straw mulch. Permanent stabilization should be established early in the fall to allow good cover before cold weather comes.

- A construction entrance in accordance with construction details shall be installed at the site entrance to prevent sediments from being tracked offsite.

- It is the responsibility of the contractor to maintain and supplement the specified sedimentation controls as necessary to prevent sedimentation of off-site areas and/or any regulated resource areas. Failure by the contractor to control erosion, pollution and/or siltation shall be cause for the owner to employ outside assistance or to use his own forces to provide the necessary corrective measures, the cost of such assistance plus project engineering costs will be the contractor's responsibility. If the owner shall fail their responsibility of this Plan, the Town has the right to enter upon property after 15 days notice to take corrective actions and bill the Owner for their Services.

- Erosion control line shall be installed at the following locations: Toe of slope of embankment construction, Toe of temporary earthwork stockpiles. All locations as indicated on the Plans.

- A log of regular inspections and maintenance is to be maintained by the construction superintendent.

- When all areas tributary to any catch basin on the site are stabilized with permanent plantings and paving, that catch basin is to be cleaned of all

sediment and debris that has accumulated during construction and the "silt sacs" removed.

- During construction of the project, the Owner and/or its representative, is to be the responsible party for enforcing the installation and maintenance of all erosion control devices. A permanent file is to be established for recording daily inspections, problems and maintenance of the erosion control devices. A 24 hour emergency hotline is to be established with the number posted on a sign at the construction entrance to the project and on the construction trailer indicating who can be contacted in case of an emergency on the site.

Long-Term Operation and Maintenance Program

- At the end of construction on the project, Owner shall be provided with a certified as built plan of all utilities constructed on the site.

- All Catch basins shall be inspected by the owner/operator on a quarterly basis or after a major storm event. Catch basins sumps will be cleaned annually during the early spring or when the sediment rises to within half the available sump height of the catch basin, whichever comes first.

- Once the system is operational, inspections of the Infiltration Structures should occur after every major storm event for the first few months. After the system is in operation, inspections should be every six months. Special attention should be directed towards the depth of sediment in the Leaching Pits. Sediment removal from the Leaching Pits accomplished as needed by means of a labor crew. Sediment shall be removed off-site and disposed of in a legal manner. Inspections should also include checking for potential problems that include, but are not limited to, any forms of erosion, tree growth in the leaching area, and sediment accumulation, etc. Trash and debris accumulated within any portion of the Infiltration Structures should be removed at this time. Silt and debris is to be removed using vacuum pumping techniques as required.

- The Owner, is to be responsible for the maintenance of the project after construction has been completed. The owner is to provide the Planning Department and Building Department with a contact name and telephone number for purposes of communication between the owner and the Town Boards and Commissions. At each time that the contact person changes, the above Boards and Commission are to be notified of the new contact information.

- The Owner shall hire a Stormwater Professional to inspect the system quarterly as required.

- This Operations and Maintenance plan is to be incorporated into all necessary documents with the stormwater operations and maintenance plan to ensure that a long-term maintenance program is adhered to by the developer and all future property owners.

- Waste shall be properly stored in sealed containers if stored outside. The preferred method is to store waste either indoors or in a structure with a locking cover to prevent entrance from animals. The containers shall be covered to prevent rainfall from leaching through the household waste.

- Vehicle washing shall be performed with non-detergent cleaners. The preferred method is to clean a vehicle is at a vehicle washing facility.

- Yard maintenance equipment, including lawn mowers and chainsaws shall be stored in a covered area. Periodic maintenance shall be performed on all equipment to ensure that no gas or oil leak into the ground.

- Yard waste shall be disposed in an approved off-site disposal facility or stored on-site in a composting pile.

- If applicable, septic systems shall be properly maintained and inspected in accordance with the State Environmental Code, Title 5. A failing septic systems shall be repaired immediately to prevent effluent from discharging into the storm drains. Never discharge gasoline, oils or chemicals into septic systems.

- Gasoline and oils shall be stored in sealed containers and in a covered, secure, and level area to prevent accidental spills. All gasoline, oil, and chemical spills shall be reported to the Wareham Fire Department and Regional DEP office.

- Lawn fertilizers and pesticides shall be in sealed containers within a covered area and remain dry. Slow release lawn fertilizers shall be used to limit the amount of fertilizer entering the groundwater. Limit the application of fertilizers to lawn area only. Sweep up any spills on impervious material to prevent runoff into the storm drains.

- Pet waste shall be properly disposed of to prevent bacteria from washing into storm drains. Small amounts of waste can be buried or sealed in a plastic bag and thrown into the trash. The preferred method is to flush the waste down the toilet.

- Snow de-icing chemicals shall be stored in a sealed container and a covered area.

- Snow shall be removed from all parking surfaces and fire truck clearance areas to provide adequate access for all safety vehicles. Snow shall be removed from all catch basin grates to avoid flooding during snow melt.

- All sand and loam piles stored on-site shall be properly stabilized or covered to prevent sediment from entering the storm drains. All piles shall be contained in a level, upland area and surrounded by a silt fence and/or haybales.

- All structural and non-structural stormwater management facilities shall be maintained to ensure proper working condition during construction and shall be fully maintained in accordance with this plan. The owner shall be responsible for maintaining the site's storm water management system in compliance with Federal, state, and local requirements and in accordance with best management practices. In the event that the Town determines that the owner has materially failed in its obligation to maintain the drainage system in accordance with best management practices and the Stormwater Operation and Maintenance Plan, the Town shall have the right, upon written notice to the Owner, and Owner's failure to remedy the maintenance issue within fifteen (15) days' notice thereof, to enter upon the site to perform the required maintenance. All costs incurred by the Town in connection with its performance of such required maintenance on the site shall be reimbursed by the Owner to the Town within thirty (30) days of the Owner's receipt of the Town's invoice for such costs.

REFERENCES

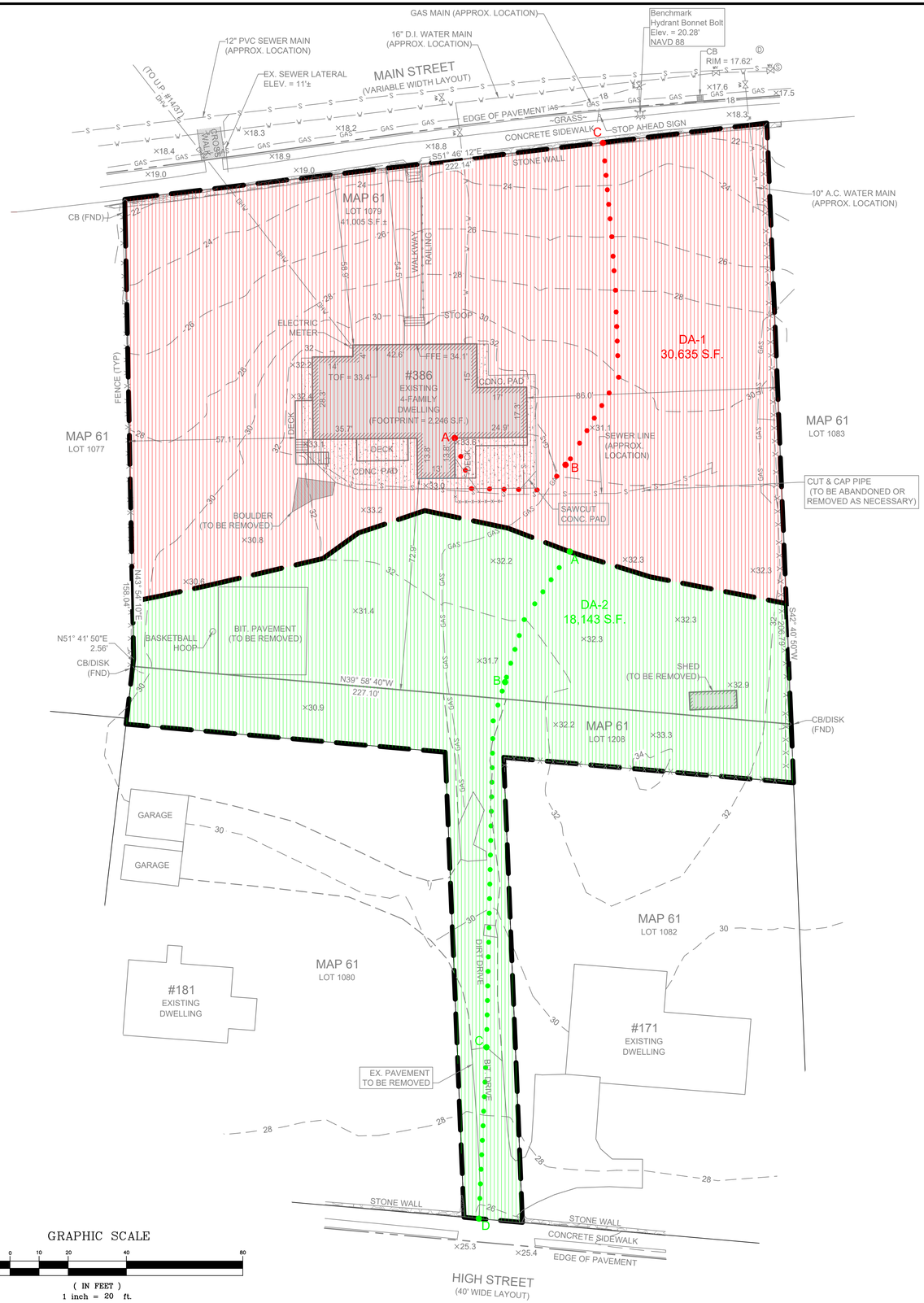
HydroCAD. Stormwater Analysis Software, Heastead Methods, Inc. 1998.

Massachusetts Department of Environmental Protection & Massachusetts Office of Coastal Zone Management. March 1997. *Stormwater Management Handboo*k. Volume 1 & 2.

U.S. Soil Conservation Service 1969. *Soil Survey of Plymouth County, Massachusetts.*

U.S. Soil Conservation Service. June 1986. *Urban Hydrology for Small Watersheds (Technical Release 55)*

DRAINAGE AREA PLANS



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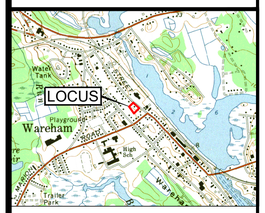
(IN FEET)

1 inch = 20 ft.



LOCUS MAP

SCALE 1" = 2000'

PROPOSED SITE PLAN

AT

386 MAIN STREET

IN

WAREHAM

MASSACHUSETTS

(PLYMOUTH COUNTY)

EXISTING
DRAINAGE AREAS

REVISIONS:

No.	DATE	DESC.
1	6-28-23	MEETING COMMENTS
2	7-25-23	MEETING COMMENTS
3	8-7-23	SITE PLAN REVIEW
4	9-13-23	PEER REVIEW COMMENTS

PREPARED FOR:

ANGELA MCKEOWN
484 LIBERTY STREET
ROCKLAND, MA 02370

PREPARED BY:

JC ENGINEERING, INC
2854 CRANBERRY HIGHWAY
EAST WAREHAM, MA 02538
508-273-0377

DATE: MARCH 21, 2023

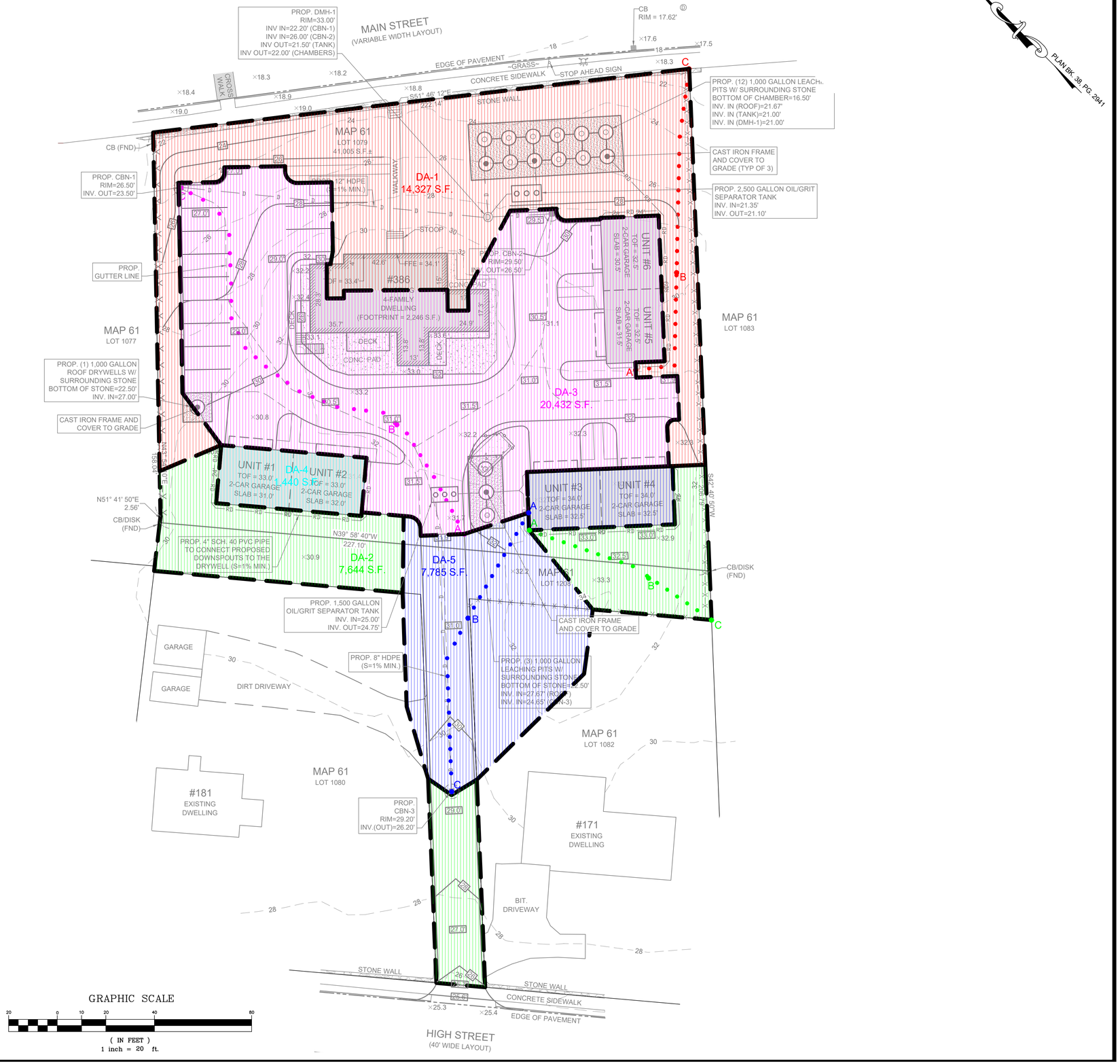
FIELD: BM/JF

CALC./DESIGN: SJI

DRAWN: SJI

CHECK: JLC

JOB NO: 6028



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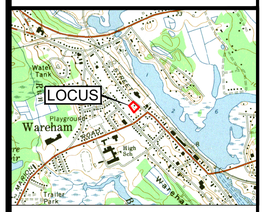

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SCALE 1" = 2000'

PROPOSED SITE PLAN

AT
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SHEET 2 OF 2