March 27, 2024

Mr. Kenneth Buckland
Director of Planning and Community Development
Town of Wareham
54 Marion Road
Wareham, MA 02571

Subject: Planning Board Peer Review
Proposed Eversource Training Facility
37 Doty Street
Wareham, Massachusetts 02576
CEC Project 323-322

Dear Mr. Buckland:

On behalf of the applicant for the above referenced property NStar Electric Co. d/b/a Eversource Energy (Eversource), Civil & Environmental Consultants, Inc. ("CEC") has prepared this memorandum in response to comments provided in a memorandum from Allen & Major Associates, Inc. ("A&M"), dated March 14, 2024.

The comments provided are summarized below in italics, followed by CEC's response in bold.

A&M COMMENTS

Note: *Any previously addressed comments shown as "Issue resolved, no further comment" are not shown below. Following comment numbers remain unchanged from the previous comment document taking place on March 14, 2024.*

WAREHAM BY-LAWS AND ZONING BY-LAWS:

2. Zoning By-Law Section 1031 requires "new projects or expansions exceeding 5,000 square feet of non- residential development or more than three multi-family dwelling units, the landscape plan shall be prepared by a registered landscape architect whose seal shall appear on the plan." Landscaping is currently shown on the site plans but has not been prepared by a Landscape Architect. A landscape plan should be provided in accordance with the Zoning By-Law. Please provide a landscape table showing the requirements of the By-Laws and what is being provided associated with the required buffers and parking lots.

   Updated Comment: The applicant has provided an updated landscaping plan stamped by a registered Landscape Architect. The applicant is seeking to remove the existing landscaped islands within the main parking field to "provide safe access to the site during emergency response functions". The lack of landscaped islands is not in compliance with Section 1060 Parking Lots of the Zoning By-laws. The applicant has not requested a

waiver/variance of the Planning Board, so it is unclear how the current application complies with the By-law.

CEC Response: The applicant requests relief from the requirement to provide the internal parking lot landscaped islands to provide safe access to and from the site during emergency response functions. The parking lot will be used by Eversource line trucks and other response vehicles during emergencies and require an open parking lot area to maneuver. Eversource will provide traffic control features and staff during emergencies as shown on the Emergency Response Operations Graphic, Sheet C201. During normal operation as a training facility, access to the parking lot will not be accessible to the public and will be gated.

SITE PLAN AND DRAINAGE CALCULATIONS:

7. The design engineer should review the time of concentration calculation for sheet flow and update the calculations accordingly. The value for the 2-yr event differs from the actual 2-yr rainfall event as noted within the HydroCAD report.

   Updated Comment: The applicant indicates the 2-year calculation has been revised, however, the HydroCAD worksheets continue to reflect a value of 3.20" of rainfall in the two-year event.

   CEC Response: The value for the 2-year event has been revised in the HydroCAD worksheets.

8. According to the project narrative, the design engineer states that the proposed building has an existing roof drain dry well to infiltrate the roof runoff and they are to remain in place, which contradicts the HydroCAD report. The HydroCAD report has the entire roof area directed into the detention basin.

   The design engineer should review the input areas contributing to Watershed A1: Flow to existing Detention Basin and revise the report and calculations accordingly.

   Updated Comment: The applicant indicates the roof area has been removed from the calculations. The watershed plans should be amended to note this to avoid confusion. However, this issue is resolved, no further comment.

   CEC Response: The Pre and Post Watershed Plans have been revised to reflect the roof area being removed from the sub-catchments. A new sub-catchment was added to delineate the roof area.

12. The design engineer is providing a sediment forebay, but no calculations have been provided. The design engineer should provide the required calculations for the sizing of the sediment forebay in accordance with the Stormwater Handbook and applicable details on the plans.

Updated Comment: The sediment forebay calculations have been provided as requested, however no details on the forebay construction are included on the plans.

CEC Response: Details for the sediment forebays are included on the wet basin cross section detail. See Sheet C801.

13. The design engineer is stating 70% TSS removal in the worksheet for an extended dry detention basin. The Massachusetts Stormwater Handbook allows 50% TSS removal when combined with a sediment forebay. The TSS worksheet should be updated accordingly to reflect the reported values in the handbook or provide documentation justifying the higher TSS removal rate.

Updated Comment: The applicant has changed the designation of the existing detention basins to a "wet basin" to obtain the required TSS and phosphorous removal credits, but no details are provided to support the designation change at that the basin qualifies as a wet basin in accordance with the MassDEP handbook including permanent plunge pool depths, runoff volume, etc. This information is required in order to support the change in designation.

CEC Response: Details are provided to support the designation change to a wet basin. Calculations are provided documenting compliance with the permanent pool volume equaling >2 times the water quality volume, minimum pool surface area, and pool drain (See Calculations in Appendix C).

16. Erosion & Sedimentation Control/Demolition Plan

    a. No work is shown on this plan. The plan should be updated to show the proposed erosion control measures being installed to protect the existing drainage system, downgradient properties and the wetland resource areas.

    b. The plan should identify the areas being demolished, cleared, removal of topsoil, etc.

    c. Two existing catch basins, located on the westerly side of the building will be located within the proposed material logistics storage and meter training area. Are these catch basins to remain or to be removed and how will they be protected during construction if to remain?

Updated comment: The applicant has provided an erosion control plan as noted. The plan depicts installation of a stone construction entrance over the existing pavement. Is the intent to remove the existing driveway to install this? The existing pavement should be retained, and the applicant would be responsible for sediment tracking and cleaning of Doty Street as required. The plan notes the abandonment of the three catch basins noted in comment 16c. but do not depict where the stormwater runoff currently collected by these basins is intended to flow. The parking lot is to be milled, regraded, and repaved, but flow along the

westerly side of the building would flow toward the proposed curb line and then need to flow northerly and then further easterly until it is captured.

<u>CEC Response</u>: The rip-rap construction entrance pad has been removed and a note stating applicant & contractor shall be responsible for sediment tracking and cleaning of Doty Street as required has been added to the plan.

The plans have been revised to show the existing drainage connection between the existing catch basin within the pavement area and drain manhole to the south. The CB is to remain as noted. The other 2 CB's are to be removed as noted and a note has been added to Sheet C300 to grade area these areas toward existing parking lot.

17. Site Layout

    a. The site layout plan illustrates several new utility poles with camera and lights to be installed in close proximity to the perimeter. What are the purpose of the lights and what are the hours of operation? A photometric plan should be provided to confirm and verify that light spillover has been minimized.

Updated Comment: A photometric plan has been provided. The plan depicts some light trespass onto Doty Street. The updated information does not provide any additional information on the use or operating hours of these lights.

<u>CEC Response</u>: The lights at the training yards will be on motion sensor with bi-level function and manual override from dusk to dawn. The bi-level will have 50% reduction from factory on the light output. The motion sensor will have five (5) minute timing and return to reduced level after five (5) minutes with no motion.

18. Grading/Drainage Plan

    a. The extended detention basin as designed does not meet the Massachusetts Stormwater Handbook Standards and should be updated to include the following:
        i. Provide calculations demonstrating that the required water quality volume meets the 24-hr time to allow solids to settle.
        ii. A 15-ft wide access road is required around the basin providing access to the forebay and the outlet control structure.
        iii. No emergency spillway has been provided.

    b. Several drainage pipes are shown discharging into the grassed water quality swale and do not have any rip rap dissipation pads. The plans should be updated to provide appropriately sized pads at each outfall based on the maximum anticipated velocity. The design engineer should provide appropriate calculations to support the design.

Updated Comment: The updated report has changed the classification of the existing stormwater basin without support information (see comment above). The access to the basin for routine maintenance is available through a limited area on the north side of the basin. The remainder is hindered by the chain link fencing around the truck training area. If the applicant's intent for maintenance is to remove the chain link fence for access, it should be noted in the Operation and Maintenance plan. Please clarify. The plan has been revised to include rip-rap at the pipe entry points, but no calculations have been provided to support the areas shown.

CEC Response: See response to Comment #13. Drawdown calculations are provided documents compliance with pool drawdown time for basin maintenance. A rip-rap spillway at el. 34.5 has been provided as the emergency spillway HydroCAD model has been updated to reflect this emergency overflow. Calculations for the rip-rap sizing at the pipe entry points are now provided with Appendix C.

19. Detail Sheets

    a. No detail is provided on the extended detention basin. Detail should be provided showing the minimum side slope, finish treatment of side slope and bottom area, access road, etc.

    b. A spillway detail is provided on Sheet C801, but A&M is unable to find where the detail is being used. Please identify the spillway location and provide the elevation of the spillway crest.

Updated Comment: The basin bottom and one linear side is proposed for regrading. Without a detail, there is no way to confirm the finished condition and may lead to constructability issues. By having confirmed construction intent, it removes any variability that may occur during routine inspections during construction.

CEC Response: A basin detail has been added to the Plan set as shown on Sheet C801. The emergency rip-rap spillway detail has been added to the plan sheet.

20. The site currently has mature vegetated areas adjacent to Doty Street and Route 58 and based on the site layout plan a majority of this vegetation is being removed to accommodate the proposed training areas. The applicant is proposing to install new landscape along the frontage as shown on Sheet C700. To the extent the existing vegetation can be maintained, is there an opportunity to review the dimensions of the training areas and determine if they can be lessened to preserve the mature vegetated areas. As previous mentioned above, the limits of clearing should be added to the demolition plan.

Updated Comment: The limits of clearing have not been provided on the revised drawings. The plan also denotes "existing trees to be protected during construction" (Sheet C700) but do not indicate any particular trees or methods of adequate protection. A&M defers to the Planning Board on the adequacy of the existing trees and the amount of tree removal requested.

CEC Response: The limit of clearing has been added to the revised drawings. Where feasible vegetation will be maintained. A tree protection fencing detail has been added for the trees that are proposed to remain. See Sheet C701.

21. The site relies on multiple stormwater management areas to function. The applicant should provide a statement for record that the existing systems have been inspected and maintained in accordance with the Stormwater Handbook before receiving additional flows as a result of the increased impervious. Any structures requiring cleaning, should be cleaned and documented prior to receiving additional stormwater runoff. These will include the catch basins, dry well systems, detention basin, and basin inlets and outlets.

    Updated Comment: A&M has no issue with the intended inspection to be conducted prior to construction. Should the Planning Board act on the application, A&M suggests a condition be added to require this work to be performed and the results provided to the Planning Office.

    CEC Response: The applicant accepts a condition requiring notification of the Planning Office of inspections of the existing stormwater features to remain.

22. The abutter list provided with the application appears incomplete as it only encompasses lot A1. Lot B1 contains portions of the building and stormwater management areas that would require inclusion in the list.

    Updated Comment: The abutter's list should be provided for record. If the abutters for Lot B1 are the same as A1, then no further action may be required. The applicant should provide this information for record, otherwise it presumes the Planning Department/Board would be required to verify the applicant's assertion.

    CEC Response: An updated abutters list is included as part of this submission.

23. Neither the plans or narratives identify the potential uses for the series of concrete pads on the easterly training site. Their use should be identified to the extent it may have impact to the adjacent resource areas or stormwater basins. Potential impact could include use of hazardous material that would drain into the catch basin and flow through the system.

    Updated Comment: The applicant has provided a detail of the concrete pads satisfying the original comment. The plans or narrative do not describe the proposed activities and A&M defers to the Planning Board if additional detail is required to satisfy any concerns.

CEC Response: The training yard functions: The function of the training areas are to simulate the real world electrical service conditions.

- **a. Overhead training area- to the left side of entrance drive.**
  - **i. This area will be for training on electrical poles to simulate the street electrical poles.**
  - **ii. There will also be areas of non-electrified and electrified training.**
  - **iii. This yard will have training for various transformer types that you will find out in the real world.**
  - **iv. The overhead training will also have linemen trucks as part of the training.**
- **b. Underground training area- to the right front of the entrance drive**
  - **i. This area is for training linemen for the underground utility work.**
  - **ii. The area will house simulated underground vault with manhole.**
  - **iii. This training yard will also house various transformer and switch to simulate real work condition.**
- **c. Substation training yard- to the right back side of entrance drive**
  - **i. This training yard will house various substation equipment to simulate real work conditions.**
  - **ii. The pads will house various simulated substation devised and transformers.**
  - **iii. There will also be low rise substation tower approximately 25' high to simulate typical towers use in the real world.**
- **d. Each of the training yard will have pre-engineered sheds to house various training equipment and for storage training devices.**

ADDITIONAL COMMENTS:

25. The drainage report narrative and HydroCAD calculations have been revised but contain discrepancies that need to be reviewed, revised and/or clarified.

    a. Section 2.1 Descriptions of Runoff Controls identifies proprietary separators for the project. The project narrative proposes StormTech units which differ than the ones specified on the plans. The water quality calculations provided do not support Stormtech units. The design engineer should clarify the design intent and revise accordingly.

    CEC Response: The descriptions of the proprietary separators has been revised to "Stormceptor" to match the design calculations.

    b. The design engineer has reclassified the detention basin as a wet basin. The design engineer should provide the appropriate calculation in accordance with the Massachusetts Stormwater Handbook for a wet basin.

    CEC Response: See comment response #13.

c. The proposed project is showing an increase of 2.0 cfs at the design point for the 100-yr storm event. An exceedance during the 100-year storm event is allowable if the applicant is able to confirm that no downstream flooding will occur. The applicant has provided this statement, but no documentation that indicates downstream will not occur.

CEC Response: A narrative and calculation has been added section 3.3 of the Stormwater Report to document compliance that no downstream flooding will occur.

26. The design engineer should review the time of concentration for Subcatchment A1: Flow to resized detention basin on the post drainage calculations and drainage area map. The design engineer states 50-ft of sheet flow on grass, but according to the plan, there is only 25-30 feet of grass before it flows onto the pavement. It also appears that the design engineer is utilizing the same flow path from existing conditions. The parking area is being regraded and the flow path has changed and should follow the new contours.

CEC Response: The time of concentration flow path for Subcatchment A1 has been revised on the pre- and post- watershed maps.

27. The resized detention basin does not meet the required 1-ft of freeboard. The 100-yr elevation in the basin is 34.76 and the HydroCAD stops at elevation 34.80. The plans call for the top of the basin to be at elevation 35. The design engineer should review and revise the basin accordingly.

CEC Response: The design includes an overflow at elevation 35.0 but does not provide 1' of freeboard. As this is a modification of an existing basin, the design strives to minimize expansion of the existing footprint of the pond. In large storm events (greater than or equal to the 100-yr event) the basins emergency spillways are designed so that stormwater will overflow into the existing wetland system on-site and not on adjacent properties.

28. The TSS removal worksheet associated with BMP Treatment Train 2 calls for a Stormceptor 450i and the wet basin with a sediment forebay. A&M is unable to find the location of the Stormceptor 450i on the plans. The design engineer should identify the water quality structure on the plans and provide the appropriate details.

CEC Response: The drawings have been updated to label the existing catchbasin to be removed and to be replaced with a Stormceptor 450i water quality unit.

29. The plans identify four (4) water quality structures as Stormceptor 900. The detail sizing report provided in the report shows the flows to WQU-1 and provides sizing worksheets for a Stormceptor 900 and a Stormceptor 450i. The design engineer should provide sizing worksheets associated with each water quality structure and for the ease of review the units on the plans should match the labeling on the sizing report.

<u>CEC Response</u>: The sizing worksheet is now being provided for all the water quality units.

We hope that you find these responses helpful in your evaluation of the Site Plan Review Application before the Planning Board. Please feel free to contact us with any questions at bpotvin@cecinc.com or via phone at (774) 501-2176.

Sincerely,

CIVIL & ENVIRONMENTAL CONSULTANTS, INC.

Brian E. Potvin, P.E.
Principal

Chris Vandenberghe, P.E.
Project Manager

Enclosures: Site Plans
Stormwater Report

CC: Philip Cordeiro, P.E., Allen & Major Associates, Inc.
Sonia Raposo, Assistant to the Planning Department, Town of Wareham

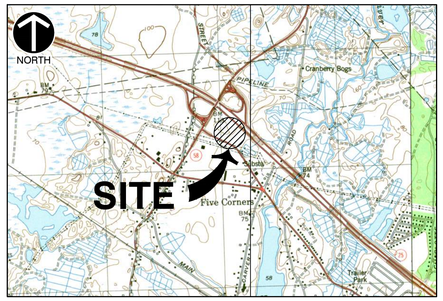
SITE PLANS

EVERSOURCE ENERGY

37 DOTY STREET
WAREHAM, MASSACHUSETTS

PERMITTING PLANS
OCTOBER 2023

REVISED MARCH 27, 2024



VICINITY MAP
SCALE: 1"=2,000'±

DRAWING INDEX				
SHEET NUMBER	DRAWING NUMBER	SHEET TITLE	DATE ISSUED	DATE REVISED
1	C000	COVER SHEET	OCTOBER 11, 2023	MARCH 27, 2024
2	C001	GENERAL NOTES	OCTOBER 11, 2023	MARCH 27, 2024
3	C100	EXISTING CONDITIONS	OCTOBER 11, 2023	MARCH 27, 2024
4	C101	EROSION & SEDIMENTATION CONTROL/DEMOLITION PLAN	OCTOBER 11, 2023	MARCH 27, 2024
5	C200	SITE LAYOUT	OCTOBER 11, 2023	MARCH 27, 2024
6	C201	EMERGENCY RESPONSE OPERATIONS GRAPHIC	OCTOBER 11, 2023	MARCH 27, 2024
7	C300	GRADING/DRAINAGE PLAN	OCTOBER 11, 2023	MARCH 27, 2024
8	C500	UTILITIES PLAN	OCTOBER 11, 2023	MARCH 27, 2024
9	C700	LANDSCAPING PLAN	OCTOBER 11, 2023	MARCH 27, 2024
10	C701	LANDSCAPE DETAILS	OCTOBER 11, 2023	MARCH 27, 2024
11	C800	DETAIL SHEET 1	OCTOBER 11, 2023	MARCH 27, 2024
12	C801	DETAIL SHEET 2	OCTOBER 11, 2023	MARCH 27, 2024
13	C802	DETAIL SHEET 3	OCTOBER 11, 2023	MARCH 27, 2024
14	C803	PHOTOMETRIC PLAN	AUGUST 6, 2023	

SITE MAP
SCALE: 1"=400'±

REFERENCE: ORTHORGRAPHIC AERIAL IMAGERY AND MAPS ARE BASED ON GIS DATA OBTAINED FROM MASSGIS PROVIDED BY THE BUREAU OF GEOGRAPHIC INFORMATION (MASSGIS), COMMONWEALTH OF MASSACHUSETTS, EXECUTIVE OFFICE OF TECHNOLOGY AND SECURITY SERVICES.

OWNER/TEAM INFORMATION

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31 BELLOWS ROAD
RAYNHAM, MA 02767
PH: (774) 501-2176
CONTACT: KARLIS P. SKULTE, P.E.

OWNER/APPLICANT
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247 STATION DRIVE
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573 HOPMEADOW STREET
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SITE DATA

ADDRESS:	37 DOTY STREET WAREHAM, MA 02576
PARCEL I.D.:	103-A1, 103-B1, 103-C1, 103-D1
DEED REFERENCE:	BOOK 56129, PAGE 190
TOTAL AREA:	18.9 ACRES
ZONING DISTRICT:	COMMERCIAL STRIP (CS)

Dig Safe Systems, Inc.
1-888-DIG-SAFE
(1-888-344-7233)

COMMONWEALTH OF MASSACHUSETTS
Brian E. Polvis
CIVIL
NO. 47231
REGISTERED PROFESSIONAL ENGINEER
3/27/24

REVISION RECORD

NO	DATE	DESCRIPTION
1.	03/08/2024	RESPONSE TO PEER REVIEW COMMENTS
2.	03/27/2024	RESPONSE TO SECOND PEER REVIEW COMMENTS

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EVERSOURCE ENERGY
37 DOTY STREET
WAREHAM, MASSACHUSETTS

COVER SHEET

DATE:	OCTOBER 11, 2023
DWG SCALE:	AS SHOWN
PROJECT NO:	323-322
DRAWN BY:	KJ
CHECKED BY:	CJV
APPROVED BY:	BEP

DRAWING NO.: C000
SHEET 1 OF 13

GENERAL NOTES

1. PROPERTY LINE INFORMATION TAKEN FROM PLAN TITLED "APPROVAL NOT REQUIRED PLAN OF LAND ON WAREHAM, MA" PREPARED FOR CLOVER LEAF GROUP NOMINEE TRUST. BY LAMONY R. HEALY LAND SURVEYORS, SCALE: 1"=80' DATED: MARCH 7, 1989 RECORDED IN THE PLYMOUTH COUNTY REGISTRY OF DEEDS AS BOOK 32 PAGE 204.
2. EXISTING SITE TOPOGRAPHIC INFORMATION COMPILED FROM A FIELD SURVEY BY CIVIL AND ENVIRONMENTAL CONSULTANTS IN JULY AND AUGUST 2022. EXISTING CONDITIONS AS DEPICTED ON THESE PLANS ARE GENERAL AND ILLUSTRATIVE IN NATURE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO EXAMINE THE SITE AND BE FAMILIAR WITH EXISTING CONDITIONS PRIOR TO BIDDING ON THIS PROJECT. IF CONDITIONS ENCOUNTERED DURING EXAMINATION ARE SIGNIFICANTLY DIFFERENT FROM THOSE SHOWN, THE CONTRACTOR SHALL NOTIFY THE ENGINEER AND TOWN OF WAREHAM IMMEDIATELY.
3. VERTICAL ELEVATIONS REFER TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).
4. THE SITE IS NOT WITHIN A FLOOD ZONE AS SHOWN ON THE FEDERAL EMERGENCY AGENCY (FEMA) FLOOD INSURANCE RATE MAP (FIRM) FOR THE TOWN OF WAREHAM, MAP# 25023C0467KK, EFFECTIVE JULY 6, 2021.
5. NO TREES SHALL BE REMOVED, NOR VEGETATION DISTURBED BEYOND THE LIMITS OF CONSTRUCTION WITHOUT THE EXPRESS WRITTEN APPROVAL OF THE OWNER'S REPRESENTATIVE.
6. PROTECTION OF EXISTING TREES AND VEGETATION: PROTECT EXISTING TREES AND OTHER VEGETATION INDICATED TO REMAIN IN PLACE AGAINST UNNECESSARY CUTTING, BREAKING OR SKINNING OF ROOTS, SKINNING OR BRUISING OF BARK, SMOTHERING OF TREES BY STOCKPILING CONSTRUCTION MATERIALS OR EXCAVATED MATERIALS WITHIN DRIP LINE, EXCESS FOOT OR VEHICULAR TRAFFIC, OR PARKING OF VEHICLES WITHIN DRIP LINE. PROVIDE TEMPORARY GUARDS TO PROTECT TREES AND VEGETATION TO BE LEFT STANDING.
7. ALL UTILITY DISCONNECTION, REMOVAL, RELOCATION, CUTTING, CAPPING AND/OR ABANDONMENT SHALL BE COORDINATED WITH THE APPROPRIATE UTILITY COMPANY / AGENCY. UTILITY CONTACTS ARE LISTED ON THE COVER SHEET.
8. EROSION & SEDIMENT CONTROL MEASURES SHALL BE IMPLEMENTED IN ACCORDANCE WITH EROSION & SEDIMENT CONTROL PLAN. A STORMWATER POLLUTION PREVENTION CONTROL PLAN SHALL BE PREPARED.
9. THE CONTRACTOR SHALL VERIFY LOCATION AND ELEVATION OF ALL EXISTING UTILITIES (INCLUDING THOSE LABELED PER RECORD DATA) PRIOR TO THE BEGINNING OF CONSTRUCTION OR EARTH MOVING OPERATIONS. INFORM ENGINEER AND TOWN OF WAREHAM OF ANY CONFLICTS DETRIMENTAL TO THE DESIGN INTENT.
10. THE CONTRACTOR SHALL CALL DIGSAFE AT 1-800-322-4844 AT LEAST 72 HOURS, SATURDAYS, SUNDAYS, AND HOLIDAYS EXCLUDED, PRIOR TO EXCAVATING AT ANY LOCATION. A COPY OF THE DIGSAFE PROJECT REFERENCE NUMBER(S) SHALL BE GIVEN TO THE OWNER AND ENGINEER PRIOR TO EXCAVATION.
11. THE CONTRACTOR AND SUBCONTRACTORS SHALL BE RESPONSIBLE FOR COMPLYING WITH APPLICABLE FEDERAL, STATE AND LOCAL REQUIREMENTS, TOGETHER WITH EXERCISING PRECAUTIONS AT ALL TIMES FOR THE PROTECTION OF PERSONS (INCLUDING EMPLOYEES) AND PROPERTY. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND SUBCONTRACTORS TO INITIATE, MAINTAIN AND SUPERVISE ALL SAFETY REQUIREMENTS, PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK.
12. THE CONTRACTOR SHALL INDEMNIFY AND HOLD HARMLESS THE OWNER AND OWNER'S REPRESENTATIVE FOR ANY AND ALL INJURIES AND/OR DAMAGES TO PERSONNEL, EQUIPMENT AND/OR EXISTING FACILITIES OCCURRING IN THE COURSE OF THE DEMOLITION AND CONSTRUCTION DESCRIBED IN THE PLANS AND SPECIFICATIONS.
13. CONTRACTOR SHALL OBTAIN A PERMIT FOR ALL CONSTRUCTION ACTIVITIES IN ACCORDANCE WITH LOCAL, STATE, & FEDERAL REGULATIONS.
14. THE CONTRACTOR SHALL COMPLY WITH ALL LOCAL CODES, OBTAIN ALL APPLICABLE PERMITS, AND PAY ALL REQUIRED FEES PRIOR TO BEGINNING WORK.
15. ANY WORK PERFORMED IN RIGHT OF WAYS SHALL BE IN ACCORDANCE WITH THE APPLICABLE LOCAL OR STATE REQUIREMENTS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN THE NECESSARY PERMITS FOR THE WORK, SCHEDULE NECESSARY INSPECTIONS, AND PROVIDE THE NECESSARY TRAFFIC CONTROL MEASURES AND DEVICES, ETC., FOR WORK PERFORMED IN THE RIGHT OF WAYS.
16. THE CONTRACTOR IS TO PERFORM ALL INSPECTIONS AS REQUIRED BY THE UNITED STATES EPA FOR THE NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM (NPDES) PERMIT AND FURNISH OWNERS REPRESENTATIVE WITH WRITTEN REPORTS.
17. CONTRACTOR SHALL IMPLEMENT ALL SOIL AND EROSION CONTROL PRACTICES IN ACCORDANCE WITH THE EROSION AND SEDIMENT CONTROL PLAN, STORM WATER POLLUTION PREVENTION PLAN AND STATE AND LOCAL REGULATIONS.
18. ALL GROUND SURFACE AREAS THAT HAVE BEEN EXPOSED OR LEFT BARE AS A RESULT OF CONSTRUCTION AND ARE TO FINAL GRADE AND ARE TO REMAIN SO, SHALL BE SEEDED AND MULCHED AS SOON AS PRACTICAL IN ACCORDANCE WITH SPECIFICATIONS. IF NO SPECIFICATIONS ARE SUPPLIED, USE STATE OF MASSACHUSETTS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS.
19. ALL WORK PERFORMED BY THE CONTRACTOR SHALL CONFORM TO THE LATEST REGULATIONS OF THE AMERICANS WITH DISABILITIES ACT.
20. THE CONTRACTOR SHALL REFER TO OTHER PLANS WITHIN THIS CONSTRUCTION SET FOR OTHER PERTINENT INFORMATION. IT IS NOT THE ENGINEER'S INTENT THAT ANY SINGLE PLAN SHEET IN THIS SET OF DOCUMENTS FULLY DEPICTS ALL WORK ASSOCIATED WITH THE PROJECT.
21. BEFORE INSTALLATION OF STORM OR SANITARY SEWER, OR OTHER UTILITY, THE CONTRACTOR SHALL VERIFY ALL CROSSINGS, BY EXCAVATION WHERE NECESSARY, AND INFORM THE OWNER AND THE ENGINEER OF ANY CONFLICTS. THE ENGINEER WILL BE HELD HARMLESS IN THE EVENT HE IS NOT NOTIFIED OF DESIGN CONFLICTS PRIOR TO CONSTRUCTION.
22. ADJUST/RECONSTRUCT ALL EXISTING CASTINGS, CLEANOUTS, ETC. WITHIN PROJECT AREA TO GRADE AS REQUIRED.
23. CONTRACTOR TO REMOVE & REPLACE PAVEMENT AS SPECIFIED.
24. SITE SIGNAGE AND STRIPING SHALL BE IN ACCORDANCE WITH THE MASSACHUSETTS MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES.

DEMOLITION NOTES

1. NO TREES SHALL BE REMOVED, NOR VEGETATION DISTURBED BEYOND THE LIMITS OF CONSTRUCTION WITHOUT THE EXPRESS WRITTEN APPROVAL OF THE OWNER'S REPRESENTATIVE.
2. TREE PROTECTION FENCING SHALL BE IN ACCORDANCE WITH THE DETAILED DRAWINGS. DO NOT OPERATE OR STORE EQUIPMENT, NOR HANDLE OR STORE MATERIALS WITHIN THE DRIP LINES OF THE TREES SHOWN TO REMAIN.
3. PROTECTION OF EXISTING TREES AND VEGETATION: PROTECT EXISTING TREES AND OTHER VEGETATION INDICATED TO REMAIN IN PLACE AGAINST UNNECESSARY CUTTING, BREAKING OR SKINNING OF ROOTS, SKINNING OR BRUISING OF BARK, SMOTHERING OF TREES BY STOCKPILING CONSTRUCTION MATERIALS OR EXCAVATED MATERIALS WITHIN DRIP LINE, EXCESS FOOT OR VEHICULAR TRAFFIC, OR PARKING OF VEHICLES WITHIN DRIP LINE. PROVIDE TEMPORARY GUARDS TO PROTECT TREES AND VEGETATION TO BE LEFT STANDING.
4. ALL DEMOLITION WASTE AND CONSTRUCTION DEBRIS SHALL BECOME THE PROPERTY OF THE CONTRACTOR UNLESS OTHERWISE DESIGNATED AND SHALL BE REMOVED BY THE CONTRACTOR AND DISPOSED OF OFFSITE IN A STATE APPROVED WASTE SITE AND IN ACCORDANCE WITH ALL LOCAL AND STATE CODES AND PERMIT REQUIREMENTS. TAKE CARE TO PROTECT UTILITIES THAT ARE TO REMAIN. REPAIR DAMAGE ACCORDING TO THE APPROPRIATE UTILITY COMPANY STANDARDS AND AT THE CONTRACTOR'S EXPENSE.
5. ALL UTILITY DISCONNECTION, REMOVAL, RELOCATION, CUTTING, CAPPING AND/OR ABANDONMENT SHALL BE COORDINATED WITH THE APPROPRIATE UTILITY COMPANY / AGENCY. UTILITY CONTACTS ARE LISTED ON THE COVER SHEET.
6. THE BURNING OF CLEARED MATERIAL AND DEBRIS SHALL NOT BE ALLOWED UNLESS CONTRACTOR OBTAINS PRIOR WRITTEN AUTHORIZATION FROM THE LOCAL AUTHORITIES.
7. EROSION & SEDIMENT CONTROL MEASURES AROUND AREAS OF DEMOLITION SHALL BE PROPERLY INSTALLED AND FUNCTION PROPERLY PRIOR TO INITIATION OF DEMOLITION ACTIVITIES.
8. IF ASBESTOS OR HAZARDOUS MATERIALS ARE FOUND ON SITE, SUCH MATERIALS SHALL BE REMOVED BY A LICENSED HAZARDOUS MATERIALS CONTRACTOR. CONTRACTOR SHALL NOTIFY OWNER IMMEDIATELY IF HAZARDOUS MATERIALS ARE ENCOUNTERED.
9. CONTRACTOR SHALL ADHERE TO ALL LOCAL, STATE, FEDERAL AND OSHA REGULATIONS DURING ALL DEMOLITION ACTIVITIES.
10. CONTRACTOR SHALL PROTECT ALL CORNER PINS, MONUMENTS, PROPERTY CORNERS AND BENCHMARKS DURING DEMOLITION ACTIVITIES. IF DISTURBED, CONTRACTOR SHALL HAVE DISTURBED ITEMS RESET BY A LICENSED SURVEYOR AT NO ADDITIONAL COST TO THE OWNER.
11. CONTRACTOR SHALL PROTECT ALL EXISTING UTILITIES, STRUCTURES, AND FEATURES TO REMAIN. ANY ITEMS TO REMAIN THAT HAVE BEEN DISTURBED OR DAMAGED AS A RESULT OF CONSTRUCTION SHALL BE REPAIRED OR REPLACED BY THE CONTRACTOR AT CONTRACTOR'S EXPENSE.
12. CONTRACTOR SHALL PROVIDE AND MAINTAIN TRAFFIC CONTROL MEASURES IN ACCORDANCE WITH STATE DEPARTMENT OF TRANSPORTATION REGULATIONS AND AS REQUIRED BY LOCAL AGENCIES WHEN WORKING IN AND/OR ALONG STREETS, ROADS, HIGHWAYS, ETC.. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN APPROVAL AND COORDINATE WITH LOCAL AND/OR STATE AGENCIES REGARDING THE NEED, EXTENT AND LIMITATIONS ASSOCIATED WITH INSTALLING AND MAINTAINING TRAFFIC CONTROL MEASURES.
13. PROVIDE NEAT, STRAIGHT, FULL DEPTH, SAW CUTS OF EXISTING PAVEMENT WHERE INDICATED ALONG LIMITS OF PAVEMENT DEMOLITION.
14. ALL UTILITY AND STRUCTURE REMOVAL, RELOCATION, CUTTING, CAPPING AND/OR ABANDONMENT SHALL BE COORDINATED AND PROPERLY DOCUMENTED BY A CERTIFIED PROFESSIONAL, WHEN APPLICABLE, WITH THE APPROPRIATE UTILITY COMPANY, MUNICIPALITY AND/OR AGENCY. DEMOLITION OF REGULATED ITEMS MAY INCLUDE, BUT ARE NOT LIMITED TO; WELLS, ASBESTOS, UNDER GROUND STORAGE TANKS, SEPTIC TANKS AND ELECTRIC TRANSFORMERS. DEMOLITION CONTRACTOR SHALL REFER TO ANY ENVIRONMENTAL STUDIES FOR DEMOLITION RECOMMENDATIONS AND GUIDANCE. AVAILABLE ENVIRONMENTAL STUDIES MAY INCLUDE, BUT ARE NOT LIMITED TO PHASE I ESA, PHASE II, WETLAND AND STREAM DELINEATION AND ASBESTOS SURVEY. ALL APPLICABLE ENVIRONMENTAL STUDIES SHALL BE MADE AVAILABLE UPON REQUEST.
15. ALL PAVEMENT, BASE COURSES, SIDEWALKS, CURBS, BUILDINGS, FOUNDATIONS, ETC., WITHIN THE AREA TO BE DEMOLISHED SHALL BE REMOVED TO FULL DEPTH. EXISTING BASE COURSE MATERIALS MAY BE WORKED INTO THE NEW PAVEMENT OR BUILDING SUBGRADE IF THE GRADATION, CONSISTENCY, COMPACTION, SUBGRADE CONDITION, ETC., ARE IN ACCORDANCE WITH THE SPECIFICATIONS AND RECOMMENDATIONS OF THE GEOTECHNICAL INVESTIGATION REPORT. BASE COURSE MATERIALS SHALL NOT BE WORKED INTO THE SUBGRADE AREAS TO RECEIVE LANDSCAPING.
16. THE CONTRACTOR SHALL USE SUITABLE METHODS TO CONTROL DUST AND DIRT CAUSED BY THE DEMOLITION ACTIVITIES.

LAYOUT NOTES

1. THE CONTRACTOR SHALL CHECK EXISTING GRADES, DIMENSIONS, AND INVERTS IN THE FIELD AND REPORT ANY DISCREPANCIES TO THE OWNER'S REPRESENTATIVE PRIOR TO BEGINNING WORK.
2. THE CONTRACTOR SHALL VERIFY THE EXACT LOCATION OF ALL EXISTING UTILITIES, INCLUDING IRRIGATION LINES, AND SHALL TAKE CARE TO PROTECT UTILITIES THAT ARE TO REMAIN. THE CONTRACTOR SHALL RELOCATE EXISTING UTILITIES AS INDICATED OR AS NECESSARY FOR CONSTRUCTION.
3. THE CONTRACTOR SHALL PROVIDE A SMOOTH TRANSITION BETWEEN EXISTING PAVEMENT AND NEW PAVEMENT. FIELD ADJUSTMENT OF FINAL GRADES MAY BE NECESSARY. THE CONTRACTOR SHALL INSTALL ALL UTILITIES, INCLUDING IRRIGATION SLEEVING, PRIOR TO THE INSTALLATION OF PAVED SURFACES.
4. THE CONTRACTOR SHALL PROTECT ALL TREES TO REMAIN.
5. ALL DAMAGE TO EXISTING PAVEMENT TO REMAIN WHICH RESULTS FROM THE CONTRACTOR'S OPERATIONS SHALL BE REPLACED WITH EQUIVALENT MATERIALS AT THE CONTRACTOR'S EXPENSE.
6. SITE DIMENSIONS SHOWN ARE TO THE FACE OF CURB OR EDGE OF PAVEMENT UNLESS OTHERWISE NOTED.
7. COORDINATES ARE FOR BUILDING COLUMNS, EXTERIOR BUILDING WALLS, CENTER OF DRIVEWAYS, CENTER OF SANITARY SEWER MANHOLES, AND CENTER OF STRUCTURES PLACED SIX INCHES INSIDE FACE OF CURB FOR DRAIN INLETS, UNLESS OTHERWISE NOTED.
8. CONTRACTOR SHALL MAINTAIN ONE SET OF AS-BUILT / RECORD DRAWINGS ON-SITE DURING CONSTRUCTION FOR DISTRIBUTION TO THE OWNER AND/OR OWNER'S REPRESENTATIVE UPON COMPLETION.
9. REFER TO THE ARCHITECTURAL, PLUMBING & ELECTRICAL DRAWINGS FOR EXACT DIMENSIONS AND LOCATIONS OF UTILITY SERVICE ENTRY LOCATIONS AND PRECISE BUILDING DIMENSIONS.
10. THIS SITE LAYOUT IS SPECIFIC TO THE APPROVALS NECESSARY FOR THE CONSTRUCTION IN ACCORDANCE WITH THE TOWN OF WAREHAM. NO CHANGES TO THE SITE LAYOUT ARE ALLOWED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER. CHANGES MADE TO THE SITE LAYOUT WITHOUT APPROVAL IS SOLELY THE RESPONSIBILITY OF THE CONTRACTOR. CHANGES INCLUDE BUT ARE NOT LIMITED TO, INCREASED IMPERVIOUS PAVEMENT, ADDITION / DELETION OF PARKING SPACES, MOVEMENT OF CURB LINES, CHANGES TO DRAINAGE STRUCTURES AND PATTERNS, CHANGES TO LANDSCAPING, ETC.

GRADING NOTES

1. ALL PROPOSED GRADES SHOWN ARE FINAL GRADES, TOP OF GROUND LEVEL, TOP OF PAVEMENT, OR GRATE ELEVATION AT THE DRAWDOWN POINT UNLESS NOTED OTHERWISE.
2. ALL ELEVATIONS SHOWN ARE FINISHED GRADE ELEVATIONS.
3. CONTRACTOR SHALL STRICTLY ADHERE TO THE EROSION & SEDIMENT CONTROL PLAN PREPARED FOR THIS PROJECT.
4. EARTHWORK SHALL INCLUDE CLEARING AND GRUBBING, STRIPPING AND STOCKPILING TOPSOIL, MASS GRADING, EXCAVATION, FILLING, UNDER CUT AND REPLACEMENT, IF REQUIRED, AND COMPACTION.
5. CONTRACTOR TO REFILL UNDERCUT AREAS WITH SUITABLE MATERIAL AND COMPACT AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER.
6. CONTRACTOR TO PLACE TOPSOIL OVER THE SUBGRADE OF UNPAVED, DISTURBED AREAS TO A DEPTH INDICATED ON THE LANDSCAPE PLANS (4" MINIMUM).
7. PAVEMENT SLOPES ACROSS ACCESSIBLE PARKING STALLS AND ADJOINING ACCESS AISLES SHALL BE MAXIMUM 2% AND SHALL CONFORM TO THE LATEST REGULATIONS OF THE AMERICANS WITH DISABILITIES ACT.
8. ALL SLOPES IN NON-PAVED AREAS SHALL BE 3:1 (HORIZONTAL:VERTICAL) MAXIMUM UNLESS NOTED OTHERWISE.
9. ALL AREAS NOT PAVED SHALL BE STABILIZED IN ACCORDANCE WITH THE EROSION & SEDIMENT CONTROL PLAN, UNLESS NOTED OTHERWISE.
10. COMPACTED FILLS ARE TO BE MADE TO A MINIMUM OF THREE FEET ABOVE THE CROWN OF ANY PROPOSED SEWER PRIOR TO CUTTING OF TRENCHES FOR PLACEMENT OF SAID SEWERS. ALL FILLS SHALL BE CONTROLLED, COMPACTED, AND INSPECTED BY AN APPROVED TESTING LABORATORY OR AN INSPECTOR FROM THE APPROPRIATE GOVERNMENTAL AGENCY.
11. ALL EXCESS SOIL MATERIALS SHALL BECOME THE PROPERTY OF THE CONTRACTOR UNLESS NOTED OTHERWISE. EXCESS SOIL MATERIALS SHALL BE REMOVED BY THE CONTRACTOR AND DISPOSED OF OFFSITE AT NO ADDITIONAL COST TO THE OWNER IN ACCORDANCE WITH ALL LOCAL AND STATE CODES AND PERMIT REQUIREMENTS.
12. THE CONTRACTOR IS RESPONSIBLE FOR BALANCING THE SITE EARTHWORK BY IMPORTING OR EXPORTING AS NECESSARY TO ACHIEVE DESIGN GRADES AND SPECIFICATIONS.

STORM DRAINAGE NOTES

1. DISTANCES SHOWN ON PIPING ARE HORIZONTAL DISTANCES FROM CENTER OF STRUCTURE TO CENTER OF STRUCTURE, UNLESS NOTED OTHERWISE.
2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL COSTS ASSOCIATED WITH THE INSTALLATION, INSPECTION, TESTING AND FINAL ACCEPTANCE OF ALL NEW STORMWATER MANAGEMENT FACILITIES. CONTRACTOR SHALL COORDINATE WITH ALL APPLICABLE REGULATING AGENCIES CONCERNING INSTALLATION, INSPECTION AND APPROVAL OF THE STORM DRAINAGE SYSTEM CONSTRUCTION.
3. ALL STORMWATER MANAGEMENT FACILITIES, INCLUDING COLLECTION AND CONVEYANCE STRUCTURES, SHALL BE INSTALLED IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND STATE CODES AND REGULATIONS.
4. ALL PROPOSED STORM SEWERS, SURFACE OR OTHER DRAINAGE FACILITIES WITHIN THE PROPERTY ARE TO BE PRIVATE AND MAINTAINED BY THE OWNER.
5. THE CONTRACTOR IS TO CONSTRUCT CURBS, CATCH BASINS, DOWNSPOUTS, PIPING AND CONNECTION ETC. AS REQUIRED TO CONVEY THE ROOF AND PAVED SURFACE DRAINAGE TO THE INFILTRATION CHAMBERS.
6. ALL CATCH BASINS AND MANHOLES WITH A DEPTH GREATER THAN 4' SHALL BE PROVIDED WITH STEPS. STEPS SHALL MEET THE REQUIREMENTS OF MASSACHUSETTS DEPARTMENT OF TRANSPORTATION SPECIFICATIONS.
7. STORM SEWER PIPE LABELED "ST" SHALL BE ONE OF THE FOLLOWING: PVC SDR-35, OR HIGH DENSITY POLYETHYLENE. STORM SEWER PIPE LABELED "RCP" SHALL BE REINFORCED CONCRETE PIPE. ALL STORM SEWER PIPE IS TO BE INSTALLED PER MASSDOT SPECIFICATIONS.
8. STORM SEWER IS TO BE BEDDED WITH CLEAN GRANULAR MATERIAL-AGGREGATES NOT TO BE LARGER THAN 3/4" AND NOT SMALLER THAN NO. 8 SIEVE, AND SHALL BE FREE OF SILT AND FINES. BEDDING TO EXTEND MINIMUM OF 6" BELOW & 12" ABOVE THE PIPE AND AS SHOWN ON THE DETAILS.

UTILITY NOTES

1. THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES AND, WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANY AT LEAST 72 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATION OF UTILITIES. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THE PLANS IN A MANNER WHICH WILL NOT NEGATIVELY AFFECT ANY EXISTING USERS OF THESE UTILITIES.
2. THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING UTILITY (WATER, SEWER, GAS, ELECTRIC, TELEPHONE AND CABLE) LOCATIONS, INVERTS AND CONDITIONS PRIOR TO CONSTRUCTION. ANY CONDITIONS FOUND TO DIFFER FROM THOSE SHOWN ON THE DRAWINGS AND REQUIRING MODIFICATIONS TO THE SITE DESIGN SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE CONSTRUCTION. DIFFERING UTILITY CONDITIONS THAT ARE ENCOUNTERED BY THE CONTRACTOR, THAT REQUIRE MODIFICATION OF SITE DESIGN AND THAT ARE NOT BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO CORRECT AT HIS SOLE COST.
3. ALL ELECTRIC LINES AND ASSOCIATED INFRASTRUCTURE SHOULD BE INSTALLED PER EVERSOURCE REQUIREMENTS.
4. ALL GAS LINES AND ASSOCIATED INFRASTRUCTURE SHOULD BE INSTALLED PER APPLICABLE NATURAL GAS COMPANY REQUIREMENTS.
5. ALL TELEPHONE LINES AND ASSOCIATED INFRASTRUCTURE SHOULD BE INSTALLED PER APPLICABLE UTILITY COMPANY REQUIREMENTS.
6. ALL UNDERGROUND UTILITIES MUST BE CLEARLY & PERMANENTLY MARKED WITH UNDERGROUND MARKING TAPE AND AS REQUIRED BY THE APPROPRIATE UTILITY COMPANY.
7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING UTILITY COMPANIES 48 HOURS PRIOR TO BEGINNING EXCAVATION.
8. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL PAVEMENT REPAIRS REQUIRED AS A RESULT OF ANY UTILITY WORK.
9. LOCATIONS AND ELEVATIONS OF UTILITY LINES ENTERING BUILDINGS SHOULD BE COORDINATED WITH THE ARCHITECTURAL & MEP DRAWINGS PRIOR TO CONSTRUCTION.
10. ALL MANHOLE COVERS, GRATES, RIMS, AND UTILITY STRUCTURES TO REMAIN SHALL BE ADJUSTED TO PROPOSED GRADE.
11. CONTRACTOR TO PROVIDE ALL FITTINGS AND BENDS NECESSARY TO ACCOMPLISH WORK.
12. A MINIMUM EIGHTEEN (18) INCHES VERTICAL CLEARANCE BETWEEN WATER, GAS, ELECTRICAL, AND TELEPHONE LINES AND STORM PIPING SHALL BE PROVIDED.
13. UTILITY CONNECTION DESIGN AS REFLECTED ON THE PLAN MAY CHANGE SUBJECT TO UTILITY PROVIDER AND GOVERNING AUTHORITY STAFF REVIEW.
14. THE CONTRACTOR SHALL ARRANGE FOR AND COORDINATE WITH THE RESPECTIVE UTILITY PROVIDERS FOR SERVICE INSTALLATIONS AND CONNECTIONS. THE CONTRACTOR SHALL COORDINATE WORK TO BE PERFORMED BY THE VARIOUS UTILITY PROVIDERS AND SHALL PAY ALL FEES FOR CONNECTIONS, DISCONNECTIONS RELOCATIONS, INSPECTIONS, AND DEMOLITION UNLESS OTHERWISE STATED IN THE PROJECT SPECIFICATIONS MANUAL AND/OR GENERAL CONDITIONS OF THE CONTRACT.
15. ALL EXISTING PAVEMENT WHERE UTILITY PIPING IS TO BE INSTALLED SHALL BE SAW CUT, AFTER UTILITY INSTALLATION IS COMPLETED, THE CONTRACTOR SHALL INSTALL PERMANENT PAVEMENT REPAIR AS DETAILED ON THE DRAWINGS OR AS REQUIRED BY THE OWNER HAVING JURISDICTION. IN THE EVENT THAT PAVEMENT REPAIR CAN NOT BE PROVIDED DUE TO WEATHER CONDITIONS, PROVIDE TEMPORARY PAVEMENT REPAIR UNTIL PERMANENT REPAIR CAN BE PROVIDED.
16. RELOCATION OF UTILITY PROVIDER FACILITIES SUCH AS POLES, SHALL BE DONE IN ACCORDANCE WITH THE REQUIREMENTS OF THE UTILITY PROVIDER.
17. BUILDING UTILITY PENETRATIONS AND LOCATIONS ARE SHOWN FOR THE CONTRACTOR'S INFORMATION AND SHALL BE VERIFIED WITH THE BUILDING MEP DRAWINGS AND WITH THE OWNER'S CONSTRUCTION MANAGER.
18. ALL UTILITY CONSTRUCTION IS SUBJECT TO INSPECTION FOR APPROVAL PRIOR TO BACKFILLING, IN ACCORDANCE WITH THE APPROPRIATE UTILITY PROVIDER REQUIREMENTS.
19. SITE CONTRACTOR SHALL COORDINATE INSTALLATION OF CONDUIT AND CABLES FOR SITE LIGHTING WITH THE ARCHITECT.
20. THE CONTRACTOR SHALL ARRANGE AND COORDINATE WITH UTILITY PROVIDERS FOR WORK TO BE PERFORMED BY UTILITY PROVIDERS. THE CONTRACTOR SHALL PAY ALL UTILITY FEES UNLESS OTHERWISE STATED IN THE PROJECT SPECIFICATION MANUAL AND GENERAL CONDITIONS, AND REPAIR PAVEMENTS AS NECESSARY.
21. ALTERNATIVE METHODS AND PRODUCTS OTHER THAN THOSE SPECIFIED MAY BE USED IF REVIEWED AND APPROVED BY THE OWNER, ENGINEER, AND APPROPRIATE REGULATORY AGENCIES PRIOR TO INSTALLATION.
22. THE CONTRACTOR SHALL MAINTAIN ALL FLOWS AND UTILITY CONNECTIONS TO EXISTING BUILDINGS WITHOUT INTERRUPTION UNLESS/UNTIL AUTHORIZED TO DISCONNECT BY THE OWNERS, THE CIVIL ENGINEER, UTILITY PROVIDERS AND GOVERNING AUTHORITIES.
23. CONTRACTOR SHALL REFER TO ARCHITECTURAL PLANS AND SPECIFICATIONS FOR ACTUAL LOCATIONS OF ALL UTILITY ENTRANCES. CONTRACTOR SHALL COORDINATE INSTALLATION OF UTILITIES IN SUCH A MANNER AS TO AVOID CONFLICTS AND TO ENSURE PROPER DEPTHS ARE ACHIEVED AS WELL AS COORDINATING WITH THE REGULATORY AGENCY AS TO LOCATION OF AND SCHEDULING OF CONNECTIONS TO THEIR FACILITIES.
24. ALL REQUIRED UTILITIES SERVING THE BUILDINGS SHALL BE COORDINATED AND CONSTRUCTED TO WITHIN FIVE FEET OF BUILDING UTILITY ENTRANCE LOCATION AT THE INVERTS NOTED. ALL REQUIRED CONNECTION FEES SHALL BE PAID BY THE BUILDING CONTRACTOR. ANY NECESSARY EXTENSIONS, RELOCATIONS, OR CORRECTIONS WITHIN FIVE FEET OF THE BUILDING NECESSARY TO COMPLETE CONNECTION OF UTILITIES TO THE BUILDINGS SHALL BE MADE BY THE BUILDING CONTRACTOR.
25. THE CONTRACTOR MUST NOTIFY THE TOWN ENGINEERING/DEPARTMENT OF PUBLIC WORKS A MINIMUM OF 72 HOURS PRIOR TO CONSTRUCTION. ALL CONNECTIONS TO TOWN FACILITIES SHALL BE COORDINATED WITH AND WITNESSED BY TOWN DPW PERSONNEL.
26. ALL ON-SITE UTILITIES SHALL BE UNDERGROUND, WHERE APPLICABLE.
27. CONTRACTOR TO COORDINATE GAS MAIN, ELECTRIC, AND TELEPHONE INSTALLATION WITH APPROPRIATE UTILITY COMPANIES.
28. CONTRACTOR SHALL MAINTAIN A MINIMUM OF 30 INCHES OF COVER FOR ALL UNDERGROUND ELECTRIC, TELEPHONE AND GAS UTILITIES OR AS REQUIRED BY THE UTILITY COMPANY, WHICHEVER IS MORE RESTRICTIVE.
29. ALL DETAILS OF ELECTRIC, GAS, & TELEPHONE UTILITY SERVICE SHALL BE APPROVED BY THE APPLICABLE UTILITY COMPANY AND INSTALLED TO THEIR REQUIREMENTS AS WELL AS THOSE OF THE DIRECTOR OF PUBLIC WORKS.
30. SEE PLAN FOR EXTERIOR CAMERA LOCATION. ALL CAMERA SHALL BE ON POLE.
31. PROVIDE TWO 2" CONDUITS PER EACH CAMERA LOCATION TO OUTBUILDING FOR NETWORK CONNECTION AND POWER.

TABLE OF ABBREVIATIONS

General		Utilities	
ACR	ACCESSIBLE CURB RAMP	ABAN	ABANDON
ADA	AMERICANS WITH DISABILITIES ACT	ADJ	ADJUST
APPROX	APPROXIMATE	CATV	CABLE TV
ARCH	ARCHITECTURAL	CIP	CAST IRON PIPE
BC	BOTTOM OF CURB	CMP	CORRUGATED METAL PIPE
BCB	BITUMINOUS CONCRETE BERM	CO	CLEANOUT
BCC	BITUMINOUS CONCRETE CURB	COND	CONDUIT
BIT	BITUMINOUS	CS	CURB STOP AND BOX
BLDG	BUILDING	DIA	DIAMETER
BLSF	BORDERING LAND SUBJECT TO FLOODING	DCB	DOUBLE CATCH BASIN
BOT	BOTTOM	DET	DETENTION
BS	BOTTOM OF SLOPE	DIP	DUCTILE IRON PIPE
BW	BOTTOM OF WALL	DMH	DRAIN MANHOLE
BWLL	BROKEN WHITE LANE LINE	DW	DOMESTIC WATER
CCB	CAPE COD BERM	EMH	ELECTRIC MANHOLE
CLF	CHAIN LINK FENCE	FA	FIRE ALARM
CONC	CONCRETE	FES	FLARED END SECTION
DPW	DEPARTMENT OF PUBLIC WORKS	FP	FIRE PROTECTION
DYCL	DOUBLE YELLOW CENTER LINE	FM	FORCE MAIN
ECC	EXTRUDED CONCRETE CURB	FO	FIBER OPTICS
ELEV	ELEVATION	F&C	FRAME AND COVER
EOP	EDGE OF PAVEMENT	F&G	FRAME AND GRATE
EX	EXISTING	GG	GAS GATE
EXIST	EXISTING	GI	GUTTER INLET
FDN	FOUNDATION	GM	GAS METER
FFE	FIRST FLOOR ELEVATION	GT	GREASE TRAP
GRAN	GRANITE	HDPE	HIGH DENSITY POLYETHYLENE PIPE
GTD	GRADE TO DRAIN	HH	HAND HOLE
HP	HIGH POINT	HW	HEADWALL
LA	LANDSCAPE AREA	HYD	HYDRANT
LOD	LIMIT OF DISTURBANCE	INF	INFILTRATION
LOW	LIMIT OF WORK	INV	INVERT ELEVATION
LP	LOW POINT	I=	INVERT ELEVATION
MAX	MAXIMUM	MES	METAL END SECTION
MCC	MONOLITHIC CONCRETE CURB	MW	MONITORING WELL
ME	MATCH EXISTING	OHW	OVERHEAD WIRE
MIN	MINIMUM	PB	PULL BOX
NDZ	NO DISTURB ZONE	PIV	POST INDICATOR VALVE
NIC	NOT IN CONTRACT	PVC	POLYVINYLCHLORIDE PIPE
NTS	NOT TO SCALE	RCP	REINFORCED CONCRETE PIPE
PCC	PRECAST CONCRETE CURB	RD	ROOF DRAIN
PL	PROPERTY LINE	R=	RIM ELEVATION
PROP	PROPOSED	SAS	SOIL ABSORPTION SYSTEM
R	RADIUS	SCB	SINGLE CATCH BASIN
RA	RIVERFRONT AREA	SLP	SITE LIGHT POLE
REM	REMOVE	SMH	SEWER MANHOLE
RET	RETAIN	SYS	SYSTEM
ROW	RIGHT-OF-WAY	TMH	TELEPHONE MANHOLE
R&D	REMOVE AND DISPOSE	TSV	TAPPING SLEEVE, VALVE, AND BOX
R&R	REMOVE AND RESET	UD	UNDERDRAIN
R&S	REMOVE AND SALVAGE	UG	UNDERGROUND
SGE	SLOPED GRANITE EDGING	UP	UTILITY POLE
SWEL	SOLID WHITE EDGE LINE	WM	WATER METER
SWLL	SOLID WHITE LANE LINE	WQI	WATER QUALITY INLET
TC	TOP OF CURB	WQU	WATER QUALITY UNIT
TR	TRASH BAY	WV	WATER VALVE AND BOX
TS	TOP OF SLOPE		
TW	TOP OF WALL		
TYP	TYPICAL		
VGC	VERTICAL GRANITE CURB		

COMMONWEALTH OF MASSACHUSETTS
Brian E. Potvin
CIVIL
NO. 47231
REGISTERED PROFESSIONAL ENGINEER
3/27/24

REVISION RECORD

NO	DATE	DESCRIPTION
1	03/08/2024	Response to peer review comments
2	03/27/2024	RESPONSE TO SECOND PEER REVIEW COMMENTS

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EVERSOURCE ENERGY
37 DOTY STREET
WAREHAM, MASSACHUSETTS

GENERAL NOTES

DATE:	OCTOBER 11, 2023	DRAWN BY:	KJ
DWG SCALE:	NOT TO SCALE	CHECKED BY:	CJV
PROJECT NO:			323-322
APPROVED BY:			BEP

DRAWING NO.: C001

NORTH

NOTES

1. PROPERTY LINE INFORMATION TAKEN FROM PLAN TITLED "APPROVAL NOT REQUIRED PLAN OF LAND ON WAREHAM, MA" PREPARED FOR CLOVER LEAF GROUP NOMINEE TRUST. BY LAMONT R. HEALY LAND SURVEYORS, SCALE: 1"=80' DATED: MARCH 7, 1989 RECORDED IN THE PLYMOUTH COUNTY REGISTRY OF DEEDS AS BOOK 32 PAGE 204 AND AN ACCEPTANCE BY TOWN OF WAREHAM AT THE APRIL 22, 2002 ANNUAL TOWN MEETING UNDER ARTICLE 41-J TO ALTER OR RELOCATE DOTY STREET.
2. EXISTING SITE TOPOGRAPHIC INFORMATION COMPILED FROM A FIELD SURVEY BY CIVIL AND ENVIRONMENTAL CONSULTANTS IN JULY AND AUGUST 2022. EXISTING CONDITIONS AS DEPICTED ON THESE PLANS ARE GENERAL AND ILLUSTRATIVE IN NATURE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO EXAMINE THE SITE AND BE FAMILIAR WITH EXISTING CONDITIONS PRIOR TO BIDDING ON THIS PROJECT. IF CONDITIONS ENCOUNTERED DURING EXAMINATION ARE SIGNIFICANTLY DIFFERENT FROM THOSE SHOWN, THE CONTRACTOR SHALL NOTIFY THE ENGINEER AND TOWN OF WAREHAM IMMEDIATELY.
3. VERTICAL ELEVATIONS REFER TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).
4. THE SITE IS NOT WITHIN A FLOOD ZONE AS SHOWN ON THE FEDERAL EMERGENCY AGENCY (FEMA) FLOOD INSURANCE RATE MAP (FIRM) FOR THE TOWN OF WAREHAM, MAP# 25023C0467K, EFFECTIVE JULY 6, 2021.

LEGEND

- EXISTING SUBJECT PROPERTY LINE
- EXISTING INDEX (MAJOR) CONTOUR
- EXISTING INTERMEDIATE (MINOR) CONTOUR
- EXISTING WETLAND LINE
- 100-FT WETLAND BUFFER ZONE LINE
- 50-FT WETLAND BUFFER ZONE LINE
- EXISTING CHAIN LINK FENCE LINE
- EXISTING CURB
- EXISTING EDGE OF PAVEMENT
- EXISTING ASPHALT PAVEMENT
- EXISTING STRUCTURE
- EXISTING STORM PIPE
- EXISTING WATER LINE
- EXISTING SANITARY SEWER LINE
- EXISTING GAS LINE
- EXISTING OVERHEAD WIRE
- EXISTING WATER VALVE
- EXISTING FIRE HYDRANT
- EXISTING SEWER MANHOLE
- EXISTING DRAIN MANHOLE
- EXISTING WATER SHUT OFF/WATER GATE
- EXISTING GAS SHUT OFF/GAS GATE
- EXISTING CATCH BASIN
- EXISTING UTILITY POLE
- EXISTING SIGN
- EXISTING SPIGOT
- EXISTING GAS METER
- EXISTING AIR CONDITIONING UNIT
- EXISTING DECIDUOUS TREE
- EXISTING CONIFEROUS TREE
- EXISTING ELECTRIC METER

POINT OF BEGINNING
BLUE STAR MEMORIAL HIGHWAY
INTERSTATE-495
CRUSHED STONE
L=694.76
R=5850.01
Δ=006°48'16"
20'X20' DUMPSTER PAD
DMH RIM=37.50
DMH RIM=36.90
DMH RIM=37.02
DMH RIM=37.46
DMH RIM=37.10
CB F&G=37.39 INV=
CONCRETE SIDEWALK (TYP)
EXISTING BUILDING 26,560± SF
ELECTRICAL OUTLET
ELECTRICAL BOX
EMH
TMH
ELECTRIC METER
EMH RIM=37.74
GAS METER
CB F&G=35.39
3" DOMESTIC WATER
6" FIRE PROTECTION WATER
METAL COVER
CB F&G=37.36
CB F&G=37.50
8" CLDI
SMH RIM=37.56
SMH RIM=37.22
SMH RIM=37.14
SEPTIC TANK
SMH RIM=37.44
SMH RIM=37.38
ELECTRICAL BOX
STOP LINE
CB F&G=35.51
WOOD GUARDRAIL
CB F&G=34.30
CB F&G=33.68
12" HDPE INV=30.36
ST
DETENTION POND
12" HDPE INV=30.75
12" HDPE INV=30.85
18" HDPE INV=30.86
12" HDPE INV=30.69
CB F&G=33.92
EXISTING SEDIMENTATION FOREBAY
EXISTING RIP-RAP SPILLWAY
SEPTIC SYSTEM LEACH FIELD
10" CLEANOUT (TYP)
SMH RIM=37.72
ACCESS PANEL RIM=37.66
DMH RIM=36.92
CB F&G=37.90
DMH RIM=37.82
CB F&G=37.78
EMH
TMH RIM=36.80
CB F&G=36.36
CB F&G=34.72
8" CLASS 52 D.I. WATER MAIN
6" SERVICE
6" WATER
100BZ
50BZ
BUFFER ZONE
50' NO ACTIVITY ZONE
COMMONWEALTH ELECTRIC EASEMENT (APPROXIMATE)
MH F&G=34.22
24" HDPE INV=28.51
12" RCP
CB F&G=30.19
CB F&G=30.25
SMH RIM=30.36
PYLON SIGN
PP#883/4
PP#883/3
EX. TREELINE
S46°38'36"E 141.98
CB/FND
Δ=007°39'41"
L=104.30
R=780.00
S67°23'46"E 209.57
(1964 TOWN LAYOUT)
(LO # 5427)
DOTY STREET
Δ=005°28'30"
L=200.67
R=2100.00
S61°55'16"E 272.86
Δ=02°37'32"
L=134.
R=327.8
CB F&G=42.89 INV=
8" CLASS 52 D.I. WATER MAIN
MAP 103 LOT 1015 B
N/F
WALDON ROBY
ROUTE 58
642.91
S30°15'58"W
ACCESS OF PERFORATED SCH 40 PVC
SEPTIC SYSTEM LEACH FIELD
SEPTIC SYSTEM LEACH FIELD
ELECTRICAL BOX
SEPTIC SYSTEM LEACH FIELD

SCALE IN FEET
0 40 80

COMMONWEALTH OF MASSACHUSETTS
THOMAS W. HARDMAN No. 33597 REGISTERED PROFESSIONAL ENGINEER

REVISION RECORD

NO	DATE	DESCRIPTION
1.	3/8/2024	RESPONSE TO PEER REVIEW COMMENTS
2.	05/27/2024	RESPONSE TO SECOND PEER REVIEW COMMENTS

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31 Bellows Road · Raynham, MA 02767
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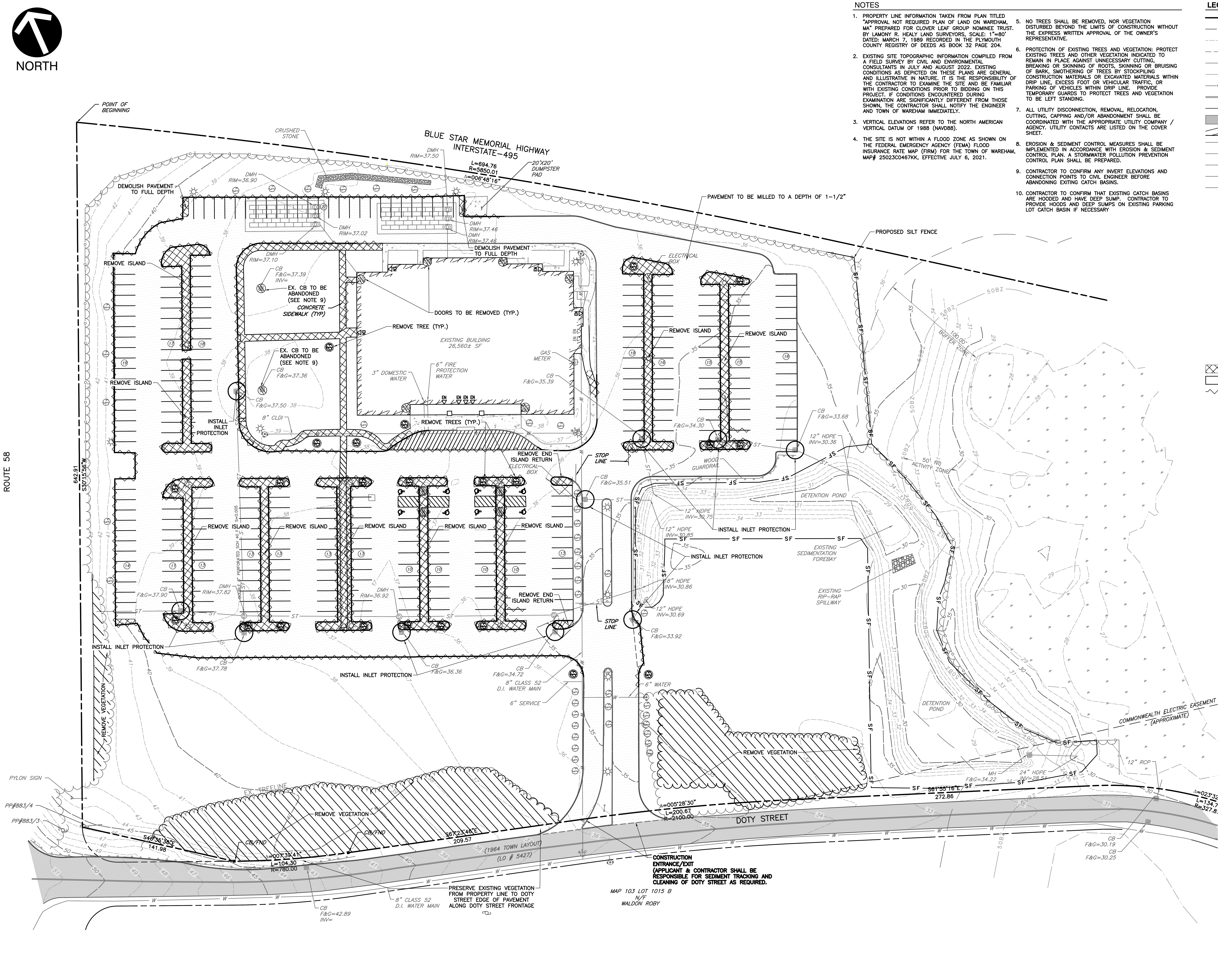
EVERSOURCE ENERGY
37 DOTY STREET
WAREHAM, MASSACHUSETTS

EXISTING CONDITIONS

DATE:	OCTOBER 11, 2023
DWG SCALE:	1" = 40'
PROJECT NO:	323-322
DRAWN BY:	KJ
CHECKED BY:	CJV
APPROVED BY:	BEP

DRAWING NO.: C100
SHEET 3 OF 13

P:\320-001\323-322\-0400\dwg\C101\323322-C101.dwg[C100] LS(3/27/2024 - cvandenberghe) - LP: 3/27/2024 2:38 PM



NOTES

1. PROPERTY LINE INFORMATION TAKEN FROM PLAN TITLED "APPROVAL NOT REQUIRED PLAN OF LAND ON WAREHAM, MA" PREPARED FOR CLOVER LEAF GROUP NOMINEE TRUST. BY LAMONY R. HEALY LAND SURVEYORS, SCALE: 1"=80' DATED: MARCH 7, 1989 RECORDED IN THE PLYMOUTH COUNTY REGISTRY OF DEEDS AS BOOK 32 PAGE 204.
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3. VERTICAL ELEVATIONS REFER TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).
4. THE SITE IS NOT WITHIN A FLOOD ZONE AS SHOWN ON THE FEDERAL EMERGENCY AGENCY (FEMA) FLOOD INSURANCE RATE MAP (FIRM) FOR THE TOWN OF WAREHAM, MAP# 25023C0467KK, EFFECTIVE JULY 6, 2021.
5. NO TREES SHALL BE REMOVED, NOR VEGETATION DISTURBED BEYOND THE LIMITS OF CONSTRUCTION WITHOUT THE EXPRESS WRITTEN APPROVAL OF THE OWNER'S REPRESENTATIVE.
6. PROTECTION OF EXISTING TREES AND VEGETATION: PROTECT EXISTING TREES AND OTHER VEGETATION INDICATED TO REMAIN IN PLACE AGAINST UNNECESSARY CUTTING, BREAKING OR SKINNING OF ROOTS, SKINNING OR BRUISING OF BARK, SMOTHERING OF TREES BY STOCKPILING CONSTRUCTION MATERIALS OR EXCAVATED MATERIALS WITHIN DRIP LINE, EXCESS FOOT OR VEHICULAR TRAFFIC, OR PARKING OF VEHICLES WITHIN DRIP LINE. PROVIDE TEMPORARY GUARDS TO PROTECT TREES AND VEGETATION TO BE LEFT STANDING.
7. ALL UTILITY DISCONNECTION, REMOVAL, RELOCATION, CUTTING, CAPPING AND/OR ABANDONMENT SHALL BE COORDINATED WITH THE APPROPRIATE UTILITY COMPANY / AGENCY. UTILITY CONTACTS ARE LISTED ON THE COVER SHEET.
8. EROSION & SEDIMENT CONTROL MEASURES SHALL BE IMPLEMENTED IN ACCORDANCE WITH EROSION & SEDIMENT CONTROL PLAN. A STORMWATER POLLUTION PREVENTION CONTROL PLAN SHALL BE PREPARED.
9. CONTRACTOR TO CONFIRM ANY INVERT ELEVATIONS AND CONNECTION POINTS TO CIVIL ENGINEER BEFORE ABANDONING EXITING CATCH BASINS.
10. CONTRACTOR TO CONFIRM THAT EXISTING CATCH BASINS ARE HOODED AND HAVE DEEP SUMP. CONTRACTOR TO PROVIDE HOODS AND DEEP SUMPS ON EXISTING PARKING LOT CATCH BASIN IF NECESSARY

LEGEND

EXISTING SUBJECT PROPERTY LINE
EXISTING INDEX (MAJOR) CONTOUR
EXISTING INTERMEDIATE (MINOR) CONTOUR
EXISTING WETLAND LINE
100-FT WETLAND BUFFER ZONE LINE
50-FT WETLAND BUFFER ZONE LINE
EXISTING CHAIN LINK FENCE LINE
EXISTING CURB
EXISTING EDGE OF PAVEMENT
EXISTING ASPHALT PAVEMENT
EXISTING STRUCTURE
EXISTING STORM PIPE
EXISTING WATER LINE
EXISTING SANITARY SEWER LINE
EXISTING GAS LINE
EXISTING OVERHEAD WIRE
EXISTING WATER VALVE
EXISTING FIRE HYDRANT
EXISTING SEWER MANHOLE
EXISTING DRAIN MANHOLE
EXISTING WATER SHUT OFF/WATER GATE
EXISTING GAS SHUT OFF/GAS GATE
EXISTING CATCH BASIN
EXISTING UTILITY POLE
EXISTING SIGN
EXISTING SPIGOT
EXISTING GAS METER
EXISTING AIR CONDITIONING UNIT
EXISTING DECIDUOUS TREE
EXISTING CONIFEROUS TREE
EXISTING ELECTRIC METER
PAVEMENT TO BE REMOVED FULL DEPTH
PAVEMENT TO BE MILLED
LINEAR OBJECT TO BE REMOVED
OBJECT TO BE REMOVED

REVISION RECORD

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EVERSOURCE ENERGY
37 DOTY STREET
WAREHAM, MASSACHUSETTS

EROSION & SEDIMENTATION CONTROL/DEMOLITION PLAN

DATE:	OCTOBER 11, 2023	DRAWN BY:	KJ
DWG SCALE:	1" = 40'	CHECKED BY:	CJV
PROJECT NO:	323-322		
APPROVED BY:	DRAFT		

SCALE IN FEET
0 40 80

COMMONWEALTH OF MASSACHUSETTS
Brian E. Potvin
CIVIL
NO. 47231
REGISTERED PROFESSIONAL ENGINEER
3/27/24

DRAWING NO.: C101
SHEET 4 OF 13

NORTH

PARKING SUMMARY		
	REQUIRED	PROVIDED
USE: OFFICES (BUSINESS, PROFESSIONAL AND ADMINISTRATIVE	1 SPACE PER 250 SF GROSS FLOOR AREA	226
GROSS FLOOR AREA: 39,592 SF		ADA SPACES: 8
TOTAL SPACES:	159	234

ZONING TABLE		
PRINCIPAL BUILDING		
	REQUIRED	PROVIDED
MINIMUM LOT AREA (SQUARE FEET)	30,000	823,984
MINIMUM FRONTAGE (FEET)	150	1,442.57
MINIMUM FRONT SETBACK (FEET)	20	367.27 (EXISTING)
MINIMUM SIDE/REAR SETBACK (FEET)	10	253.73/71.40 (EXISTING)
MAXIMUM HEIGHT (FEET)	40	N/A (EXISTING)
MAXIMUM BUILDING COVERAGE (%)	40	3.22 (EXISTING)
MAXIMUM IMPERVIOUS SURFACE (%)	65	38±
DISTANCE OF ANY STRUCTURE FROM A RESIDENTIAL DISTRICT (FEET)	40	71±

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8. EROSION & SEDIMENT CONTROL MEASURES SHALL BE IMPLEMENTED IN ACCORDANCE WITH EROSION & SEDIMENT CONTROL PLAN. A STORMWATER POLLUTION PREVENTION CONTROL PLAN SHALL BE PREPARED.
9. SEE PLAN FOR EXTERIOR CAMERA LOCATION. ALL CAMERA SHALL BE ON POLE.
10. PROVIDE 7'-0" HIGH CHAIN LINK FENCING AT THE PERIMETER OF EACH TRAINING YARD.
11. PROVIDE AUTO OPENING GATE SYSTEM AT EACH TRAINING YARD WITH CARD READER ACCESS.
12. PROVIDE AUTO OPENING GATE SYSTEM WITH CARD READER ACCESS AT THE ENTRANCE TO THE SITE. PROVIDE FOR EXIT AND ENTRANCE.
13. PROVIDE 7'-0" HIGH CHAIN LINK FENCING AT PERIMETER OF MATERIAL STORAGE AREAS.
14. RE-PAVED PARKING LOT SHALL BE RE-STRIPED AS SHOWN.
15. ALL PARKING SPACES SHALL BE 9.5'W X 19'L.

LEGEND

- EXISTING SUBJECT PROPERTY LINE
- 100BZ — 100-FT WETLAND BUFFER ZONE LINE
- 50BZ — 50-FT WETLAND BUFFER ZONE LINE
- EXISTING CHAIN LINK FENCE LINE
- EXISTING ROADWAY CENTERLINE
- EXISTING CURB
- EXISTING EDGE OF PAVEMENT
- EXISTING ASPHALT PAVEMENT
- EXISTING UTILITY POLE
- EXISTING SIGN
- EXISTING DECIDUOUS TREE
- PROPOSED CURB & GUTTER
- PROPOSED EDGE OF PAVED DRIVE
- PROPOSED EDGE OF UNPAVED DRIVE
- PROPOSED GRAVEL DRIVE
- PROPOSED CONCRETE
- PROPOSED STRIPING
- PROPOSED PAVEMENT SHOULDER
- PROPOSED SIDEWALK
- PROPOSED EASEMENT
- PROPOSED MUNICIPAL BOUNDARY
- PROPOSED PARCEL LINE
- PROPOSED SUBJECT PROPERTY BOUNDARY
- PROPOSED RIGHT-OF-WAY
- PROPOSED BUILDING
- PROPOSED BUILDING OVERHANG
- PROPOSED FENCE
- PROPOSED GUARDRAIL
- PROPOSED TREELINE
- PROPOSED SIGN
- CTR PROPOSED ENTRANCE
- OHD PROPOSED OVERHEAD DOOR

SITE DATA

ADDRESS:	37 DOTY STREET WAREHAM, MA 02576
PARCEL I.D.:	103-A1, 103-B1, 103-C1, 103-D1
DEED REFERENCE:	BOOK 56129, PAGE 190
TOTAL AREA:	18.9 ACRES
ZONING DISTRICT:	COMMERCIAL STRIP (CS)

POINT OF BEGINNING
BLUE STAR MEMORIAL HIGHWAY INTERSTATE-495
L=694.76
R=5850.01
Δ=006°48'16"
CRUSHED STONE
PAD MOUNT TRANSFORMER
POLE LINE AREA WITH 6' HIGH DISPLAY POLES
PCC
FENCE E – SLIDING GATE SYSTEM WITH CARD ACCESS
MATCH EXISTING CURB
BUILDING GENERATOR
MATERIAL LOGISTICS STORAGE
10' WIDE AREA FOR OFF LOADING
8'X10' SCISSOR LIFT
EXISTING BUILDING TO REMAIN AND TO BE RENOVATED ONE STORY 26,382 SF MEZZANINE 13, 210 SF TOTAL 39,592 SF
FENCE D – SLIDING GATE SYSTEM WITH CARD ACCESS
EV TRAINING AREA
3 POLE STRUCTURE (SEE DETAIL)
METER TRAINING AREA
PROP. LED LIGHT WALL PACKS (SEE DETAIL,TYP.)
EXISTING CHAIN LINK FENCE TO REMAIN
TRUCK PARKING AREA (TYP.)
253.73'
MILLED PAVEMENT TO BE PAVED WITH 1-1/2" BITUMINOUS TOP COURSE (TYP.)
ADD 4' LONG VGC SECTION
367.27'
7' HIGH CHAIN LINK PERIMETER FENCING WITH BARBED WIRE CONNECT TO EXISTING FENCE
ROUTE 58
S32°15'58"W 642.91
ISLISHED ISLANDS TO BE PLACED WITH STANDARD ITY FLEXIBLE PAVEMENT (SEE DETAIL)
PTZ CAMERA ON UTILITY POLE WITH LIGHT (TYP.)
4'X5' CONCRETE PAD (TYP. OF 10)
PYLON SIGN
GATE ACCESS
7' HIGH CHAIN LINK FENCE
GATE C – SLIDING GATE WITH CARD ACCESS
CONCRETE PAD FOR 20'X40' SHED
NON-ELECTRIFIED POLE TRAINING AREA
CONCRETE PAD FOR 12'X16' SHED
URD TRAINING
ELECTRIFIED POLE TRAINING AREA
POLE STORAGE
S46°35'36"E 141.98
CB/END
Δ=007°39'41" L=104.30 R=780.00
S67°23'46"E 209.57
7' HIGH CHAIN LINK PERIMETER FENCING
MAP 103 LOT 1015 B N/F WALDON ROBY
PEDESTRIAN GATE ACCESS
GATE A – SLIDING GATE WITH CARD ACCESS
GATE B – SLIDING GATE WITH CARD ACCESS
GATE G – SLIDING GATE WITH CARD ACCESS
PEDESTRIAN GATE
TRUCK TRAINING AREA
VAULT TRAINING AREA
GATE H – SLIDING GATE WITH CARD ACCESS
CONCRETE PADS FOR TRANSFORMERS
(3) FIBERGLASS PADS
UNDERGROUND MANHOLE AREA
CONCRETE PAD FOR 40'X20' SHED
SUBSTATION TRAINING AREA
CONCRETE PAD FOR 15'X20' SHED
CONCRETE PAD FOR 12'X16' SHED
15'X15' CONCRETE PAD
15'X40' CONCRETE PAD
12'X12' CONCRETE PAD
24'X24' CONCRETE PAD
GATE G (2) 6' WIDE SWING GATES (FOR BASIN MAINTENANCE ACCESS)
7' HIGH CHAIN LINK FENCE W/ BARBED WIRE
DETENTION POND
RESIZED DETENTION BASIN
EXISTING CHAIN LINK FENCE TO REMAIN
COMMONWEALTH ELECTRIC EASEMENT (APPROXIMATE)
S61°55'16"E 272.86
Δ=005°28'30" L=200.67 R=2100.00
Δ=023°32' L=137. R=307.9
DOTY STREET
GATE H (2) 6' WIDE SWING GATES (FOR BASIN MAINTENANCE ACCESS)
7' HIGH CHAIN LINK PERIMETER FENCING W/ BARBED WIRE CONNECT TO EXISTING FENCE
7' HIGH CHAIN LINK FENCE W/ BARBED WIRE
FIELD TRANSMISSION MATERIAL STORAGE
GATE F – SLIDING GATE WITH CARD ACCESS
FIELD TRANSMISSION PARKING AREA
100' BUFFER ZONE
50' NO ACTIVITY ZONE
1201.97'
±24.50'
±26.50'
19.00'
9.50'
14.00'
18.00'
71.40'
253.73'

SCALE IN FEET
0

Brian E. Polvin CIVIL NO. 47251 REGISTERED PROFESSIONAL ENGINEER COMMONWEALTH OF MASSACHUSETTS
3/27/24

REVISION RECORD		
NO	DATE	DESCRIPTION
1.	3/8/2024	RESPONSE TO PEER REVIEW COMMENTS
2.	03/27/2024	RESPONSE TO SECOND PEER REVIEW COMMENTS

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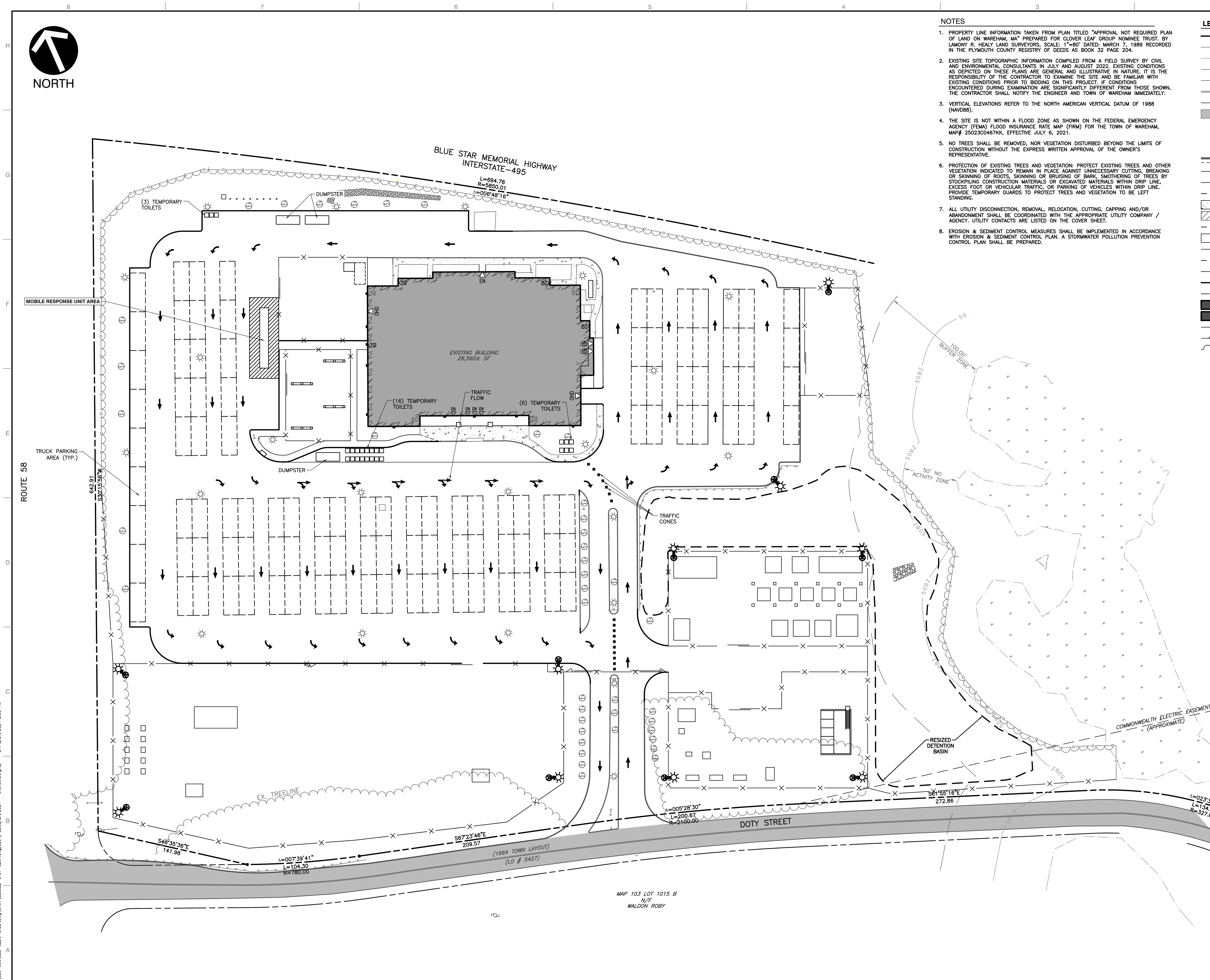
EVERSOURCE ENERGY
37 DOTY STREET
WAREHAM, MASSACHUSETTS

SITE LAYOUT

DATE:	OCTOBER 11, 2023
DWG SCALE:	1" = 40'
PROJECT NO:	323-322
APPROVED BY:	BEP
DRAWN BY:	KJ
CHECKED BY:	CJV

DRAWING NO.: C200
SHEET 5 OF 13

P:\320-000\323-322\-CAD\Dwg\CIVIL\323322-CIV01-C200.dwg[C200] LS:(3/27/2024 - cvandenbergh) - LP: 3/27/2024 2:39 PM



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LEGEND

- EXISTING SUBJECT PROPERTY LINE
- 100-FT WETLAND BUFFER ZONE LINE
- 50-FT WETLAND BUFFER ZONE LINE
- EXISTING CHAIN LINK FENCE LINE
- EXISTING ROADWAY CENTERLINE
- EXISTING CURB
- EXISTING EDGE OF PAVEMENT
- EXISTING ASPHALT PAVEMENT
- EXISTING UTILITY POLE
- EXISTING SIGN
- EXISTING DECIDUOUS TREE
- PROPOSED CURB & GUTTER
- PROPOSED EDGE OF PAVED DRIVE
- PROPOSED EDGE OF UNPAVED DRIVE
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REVISION RECORD

NO	DATE	DESCRIPTION
1	3/8/2024	RESPONSE TO PEER REVIEW COMMENTS
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EVERSOURCE ENERGY
37 DOTY STREET
WAREHAM, MASSACHUSETTS

EMERGENCY RESPONSE OPERATIONS GRAPHIC

DATE:	OCTOBER 11, 2023	DRAWN BY:	KJ
DWG SCALE:	1" = 40'	CHECKED BY:	CJV
PROJECT NO:	323-322	APPROVED BY:	BEP

COMMONWEALTH OF MASSACHUSETTS
Brian E. Potvin
CIVIL
NO. 47231
REGISTERED PROFESSIONAL ENGINEER
3/27/24

DRAWING NO.:
C201
SHEET 6 OF 13

NORTH

NOTES

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6. PROTECTION OF EXISTING TREES AND VEGETATION: PROTECT EXISTING TREES AND OTHER VEGETATION INDICATED TO REMAIN IN PLACE AGAINST UNNECESSARY CUTTING, BREAKING OR SKINNING OF ROOTS, SKINNING OR BRUISING OF BARK, SMOTHERING OF TREES BY STOCKPILING CONSTRUCTION MATERIALS OR EXCAVATED MATERIALS WITHIN DRIP LINE, EXCESS FOOT OR VEHICULAR TRAFFIC, OR PARKING OF VEHICLES WITHIN DRIP LINE. PROVIDE TEMPORARY GUARDS TO PROTECT TREES AND VEGETATION TO BE LEFT STANDING.
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8. EROSION & SEDIMENT CONTROL MEASURES SHALL BE IMPLEMENTED IN ACCORDANCE WITH EROSION & SEDIMENT CONTROL PLAN. A STORMWATER POLLUTION PREVENTION CONTROL PLAN SHALL BE PREPARED.

LEGEND

- EXISTING SUBJECT PROPERTY LINE
- 100-FT WETLAND BUFFER ZONE LINE
- 50BZ — 50-FT WETLAND BUFFER ZONE LINE
- EXISTING CHAIN LINK FENCE LINE
- EXISTING ROADWAY CENTERLINE
- EXISTING CURB
- EXISTING EDGE OF PAVEMENT
- EXISTING ASPHALT PAVEMENT
- EXISTING SIGN
- EXISTING DECIDUOUS TREE
- PROPOSED CURB & GUTTER
- PROPOSED EDGE OF PAVED DRIVE
- PROPOSED EDGE OF UNPAVED DRIVE
- PROPOSED GRAVEL DRIVE
- PROPOSED CONCRETE
- PROPOSED STRIPING
- PROPOSED PAVEMENT SHOULDER
- PROPOSED SIDEWALK
- PROPOSED EASEMENT
- PROPOSED MUNICIPAL BOUNDARY
- PROPOSED PARCEL LINE
- PROPOSED SUBJECT PROPERTY BOUNDARY
- PROPOSED RIGHT-OF-WAY
- PROPOSED BUILDING
- PROPOSED BUILDING OVERHANG
- PROPOSED FENCE
- PROPOSED GUARDRAIL
- PROPOSED TREELINE
- PROPOSED SIGN
- CB — PROPOSED ENTRANCE
- OHD — PROPOSED OVERHEAD DOOR
- 380 — PROPOSED INDEX (MAJOR) CONTOUR
- 381 — PROPOSED INTERMEDIATE (MINOR) CONTOUR
- 595.68 — PROPOSED SPOT ELEVATION
- PROPOSED STORM PIPE
- PROPOSED CATCH BASIN

POINT OF BEGINNING

BLUE STAR MEMORIAL HIGHWAY
INTERSTATE-495
L=694.76
R=5850.01
Δ=006°48'16"

EXISTING BUILDING
26,560± SF

SLOPE GRAVEL AREA TO PARKING LOT

SLOPE GRAVEL AREA TO PARKING LOT

TRUCK PARKING AREA (TYP.)

ROUTE 58

642.91
S32°15'56"W

WATER QUALITY UNIT (STORMCEPTOR STC 900) (WQU-1)

WATER QUALITY UNIT (STORMCEPTOR STC 900) (WQU-2)

WATER QUALITY UNIT (STORMCEPTOR STC 900) (WQU-3)

WATER QUALITY UNIT (STORMCEPTOR STC 900) (WQU-4)

WATER QUALITY UNIT (STORMCEPTOR STC 900) (WQU-5)

RIP-RAP #1

RIP-RAP #2

SEDIMENT FOREBAY BOT.=31.0 TOP=32.0

SEDIMENT FOREBAY BOT. EL=30.0 TOP EL.=31.0

PROP. RIP-RAP STONE FOR INLET (RIP-RAP#3)

12" HDPE INV. IN W/ RIP-RAP=31.3

12" HDPE INV. IN W/ RIP-RAP=30.6

12" HDPE INV=30.36

12" HDPE INV=30.86

REMOVE EX. RIP-RAP SPILLWAY

REPLACE EX. CB W/WATER QUALITY UNIT (STORMCEPTOR STC 450)

CB-1
RIM=35.6
INV OUT (12")=31.85

CB-2
RIM=34.0
INV OUT (12")=31.0

PROPOSED 2'WX1'H EARTHEN BERM EL.=37.0

BOT. OF VAULT AREA EL=25.0±

RECONFIGURED DETENTION POND TOP BERM=35.0

OUTLET CONTROL STRUCTURE
RIM=34.5 (EMERGENCY OVERFLOW)
4' WIDE WIER EL.=33.83
24" HDPE OUTLET EL.=29.18

24" HDPE INV. WITH RIP-RAP =29.0

12" RCP

BROAD-CRESTED WEIR RIP-RAP SPILLWAY
EL=34.5 (EMERGENCY OVERFLOW)

COMMONWEALTH ELECTRIC EASEMENT (APPROXIMATE)

100.00' BUFFER ZONE

50' NO ACTIVITY ZONE

PYLON SIGN

PP#883/4

PP#883/3

S46°35'36"E
141.98

S67°23'46"E
209.57

Δ=007°39'41"
L=104.30
R=780.00

Δ=005°28'30"
L=200.67
R=2100.00

S61°55'18"E
272.86

Δ=023°32'
L=134.7
R=327.8'

DOTY STREET

MAP 103 LOT 1015 B
N/F
WALDON ROBY

SCALE IN FEET
0 40 80

COMMONWEALTH OF MASSACHUSETTS
Brian E. Potvin
CIVIL
NO. 47251
REGISTERED PROFESSIONAL ENGINEER
3/27/24

REVISION RECORD

NO	DATE	DESCRIPTION
1.	8/8/2024	RESPONSE TO PEER REVIEW COMMENTS
2.	10/27/2024	RESPONSE TO SECOND PEER REVIEW COMMENTS

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Ph: 774.501.2176 · 866.312.2024 · Fax: 774.501.2669
www.cecinc.com

EVERSOURCE ENERGY
37 DOTY STREET
WAREHAM, MASSACHUSETTS

GRADING/DRAINAGE PLAN

DATE:	OCTOBER 11, 2023
DRAWN BY:	KJ
DWG SCALE:	1" = 40'
CHECKED BY:	CJV
PROJECT NO:	323-322
APPROVED BY:	BEP

DRAWING NO.: C300
SHEET 7 OF 13

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NORTH

ROUTE 58

POINT OF BEGINNING

CRUSHED STONE

BLUE STAR MEMORIAL HIGHWAY
INTERSTATE-495

L=694.76
R=5850.01
Δ=006°48'16"

642.91
S30°15'56"W

ELECTRICAL OUTLET
BUILDING GENERATOR

ELECTRICAL BOX

EMH

ELECTRIC METER

EMH
RIM=37.74

GAS METER

3" DOMESTIC WATER

6" FIRE PROTECTION WATER

METAL COVER

SMH
RIM=37.56

METAL COVER

TMH

SMH
RIM=37.22

SMH
RIM=37.14

SEPTIC TANK

SMH
RIM=37.44

SMH
RIM=37.38

ELECTRICAL BOX

COORDINATE WITH ELECTRICAL DOCUMENTS FOR CONDUIT LAYOUT

SEPTIC SYSTEM LEACH FIELD

SMH
RIM=37.72

ACCESS PANEL
RIM=37.66

SEPTIC SYSTEM LEACH FIELD

10" CLEANOUT (TYP)

EMH

TMH
RIM=36.60

8" CLASS 52 D.I. WATER MAIN

6" WATER

8" SERVICE

NEW PULL BOX

NEW PULL BOX

NEW PULL BOX

PYLON SIGN

PP#883/4

PP#883/3

EX. TREELINE

S46°35'38"E
141.98

CB/FND

CB/FND

S67°23'46"E
209.57

Δ=007°39'41"
L=104.30
R=780.00

PROP. ELECTRICAL CONNECTION FROM EX. U.P #883

8" CLASS 52 D.I. WATER MAIN

MAP 103 LOT 1015 B
N/F
WALDON ROBY

BOT. OF VAULT AREA EL=25.0±

RESIZED DETENTION BASIN

Δ=005°28'30"
L=200.67
R=2100.00

S61°55'16"E
272.86

DOTY STREET

50'

100.00' BUFFER ZONE

50' NO ACTIVITY ZONE

COMMONWEALTH ELECTRIC EASEMENT (APPROXIMATE)

SMH
RIM=30.36

100BZ

50BZ

280Z

SEPTIC CALCULATIONS FROM 02/03/02 CINEMA DESIGN PLAN

DESIGN FLOW 1400 SEATS X 5 GPD/SEAT = 7,000 GPD

SYSTEM DESIGN
DESIGN PERC RATE: 2 MINUTES PER INCH USE LEACHING BED: 100' LONG, 100' WIDE BOTTOM: 100' X 100' X (0.74 G/SF)=7,400 GPD PROVIDED

SEPTIC TANK DESIGN
200% x 7,000 GPD=14,000 GPD USE 1, 23,000 GALLON 2 COMPARTMENT H-20 SEPTIC TANK

PUMP CHAMBER DESIGN
7,000 GPD / 4 DOSES PER DAY=1750 GAL. PER DOSE/2=875 GAL PER DOSE DOSING SHALL ALTERNATE FROM SYSTEM "A" TO SYSTEM "B"

1750 GALLONS + 7,000 GALLON STORAGE=8,750 GALLON TANK REQUIRED USE 9,000 GALLON PUMP CHAMBER

DESIGN DATA

1. ESTIMATED DAILY FLOW:
   SECONDARY EDUCATION W/ CAFETERIA, GYMNASIUM AND SHOWERS: 20 GPD/PERSON
   20 GPD X 350 STUDENTS (MAX.) =7,000 GALLONS PER DAY.

PER THE ABOVE CALCULATIONS, THE EXISTING ON-SITE SEPTIC SYSTEM CAN ACCOMMODATE A MAXIMUM OF 350 STUDENTS, WHICH IS MORE THAN THE ANTICIPATED AMOUNT.

LEGEND

- EXISTING SUBJECT PROPERTY LINE
- 100BZ — 100-FT WETLAND BUFFER ZONE LINE
- 50BZ — 50-FT WETLAND BUFFER ZONE LINE
- EXISTING CHAIN LINK FENCE LINE
- EXISTING ROADWAY CENTERLINE
- EXISTING CURB
- EXISTING EDGE OF PAVEMENT
- EXISTING ASPHALT PAVEMENT
- ST — EXISTING STORM PIPE
- W — EXISTING WATER LINE
- SAN — EXISTING SANITARY SEWER LINE
- G — EXISTING GAS LINE
- OH-E — EXISTING OVERHEAD WIRE
- UG-E — EXISTING UNGERGROUND ELECTRIC LINE
- FO — EXISTING FIBER OPTIC LINE
- WV — EXISTING WATER VALVE
- EXISTING FIRE HYDRANT
- EXISTING SEWER MANHOLE
- EXISTING DRAIN MANHOLE
- EXISTING WATER SHUT OFF/WATER GATE
- EXISTING GAS SHUT OFF/GAS GATE
- EXISTING CATCH BASIN
- EXISTING UTILITY POLE
- EXISTING SIGN
- EXISTING DECIDUOUS TREE
- PROPOSED CURB & GUTTER
- PROPOSED SIDEWALK
- PROPOSED FENCE
- PROPOSED SIGN
- EN — PROPOSED ENTRANCE
- OH — PROPOSED OVERHEAD DOOR
- 595.68 — PROPOSED SPOT ELEVATION
- PROPOSED STORM PIPE
- PROPOSED CATCH BASIN
- E — PROPOSED UNDERGROUND ELECTRIC

NOTES

1. PROPERTY LINE INFORMATION TAKEN FROM PLAN TITLED "APPROVAL NOT REQUIRED PLAN OF LAND ON WAREHAM, MA" PREPARED FOR CLOVER LEAF GROUP NOMINEE TRUST. BY LAMONY R. HEALY LAND SURVEYORS, SCALE: 1"=80' DATED: MARCH 7, 1989 RECORDED IN THE PLYMOUTH COUNTY REGISTRY OF DEEDS AS BOOK 32 PAGE 204.
2. EXISTING SITE TOPOGRAPHIC INFORMATION COMPILED FROM A FIELD SURVEY BY CIVIL AND ENVIRONMENTAL CONSULTANTS IN JULY AND AUGUST 2022. EXISTING CONDITIONS AS DEPICTED ON THESE PLANS ARE GENERAL AND ILLUSTRATIVE IN NATURE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO EXAMINE THE SITE AND BE FAMILIAR WITH EXISTING CONDITIONS PRIOR TO BIDDING ON THIS PROJECT. IF CONDITIONS ENCOUNTERED DURING EXAMINATION ARE SIGNIFICANTLY DIFFERENT FROM THOSE SHOWN, THE CONTRACTOR SHALL NOTIFY THE ENGINEER AND TOWN OF WAREHAM IMMEDIATELY.
3. VERTICAL ELEVATIONS REFER TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).
4. THE SITE IS NOT WITHIN A FLOOD ZONE AS SHOWN ON THE FEDERAL EMERGENCY AGENCY (FEMA) FLOOD INSURANCE RATE MAP (FIRM) FOR THE TOWN OF WAREHAM, MAP# 25023C0467KK, EFFECTIVE JULY 6, 2021.
5. NO TREES SHALL BE REMOVED, NOR VEGETATION DISTURBED BEYOND THE LIMITS OF CONSTRUCTION WITHOUT THE EXPRESS WRITTEN APPROVAL OF THE OWNER'S REPRESENTATIVE.
6. PROTECTION OF EXISTING TREES AND VEGETATION: PROTECT EXISTING TREES AND OTHER VEGETATION INDICATED TO REMAIN IN PLACE AGAINST UNNECESSARY CUTTING, BREAKING OR SKINNING OF ROOTS, SKINNING OR BRUISING OF BARK, SMOTHERING OF TREES BY STOCKPILING CONSTRUCTION MATERIALS OR EXCAVATED MATERIALS WITHIN DRIP LINE, EXCESS FOOT OR VEHICULAR TRAFFIC, OR PARKING OF VEHICLES WITHIN DRIP LINE. PROVIDE TEMPORARY GUARDS TO PROTECT TREES AND VEGETATION TO BE LEFT STANDING.
7. ALL UTILITY DISCONNECTION, REMOVAL, RELOCATION, CUTTING, CAPPING AND/OR ABANDONMENT SHALL BE COORDINATED WITH THE APPROPRIATE UTILITY COMPANY / AGENCY. UTILITY CONTACTS ARE LISTED ON THE COVER SHEET.
8. EROSION & SEDIMENT CONTROL MEASURES SHALL BE IMPLEMENTED IN ACCORDANCE WITH EROSION & SEDIMENT CONTROL PLAN. A STORMWATER POLLUTION PREVENTION CONTROL PLAN SHALL BE PREPARED.
9. FINAL LOCATION(S) OF PROPOSED UNDERGROUND ELECTRICAL LINES AND INFRASTRUCTURE SHALL BE INSTALLED PER EVERSOURCE REQUIREMENTS AND SHALL BE COORDINATED WITH CONTRACTOR/DEVELOPER.
10. SEE SHEET C001 FOR UTILITY NOTES.

SCALE IN FEET
0 40 80

Brian E. Pohrh
CIVIL
NO. 47231
REGISTERED PROFESSIONAL ENGINEER
COMMONWEALTH OF MASSACHUSETTS
5/27/24

REVISION RECORD

NO	DATE	DESCRIPTION
1.	3/8/2024	RESPONSE TO PEER REVIEW COMMENTS
2.	05/27/2024	RESPONSE TO SECOND PEER REVIEW COMMENTS

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EVERSOURCE ENERGY
37 DOTY STREET
WAREHAM, MASSACHUSETTS

UTILITIES PLAN

DATE:	OCTOBER 11, 2023
DRAWN BY:	KJ
CHECKED BY:	CJV
DWG SCALE:	1" = 40'
PROJECT NO:	323-322
APPROVED BY:	BEP

DRAWING NO.:
C500
SHEET 8 OF 13

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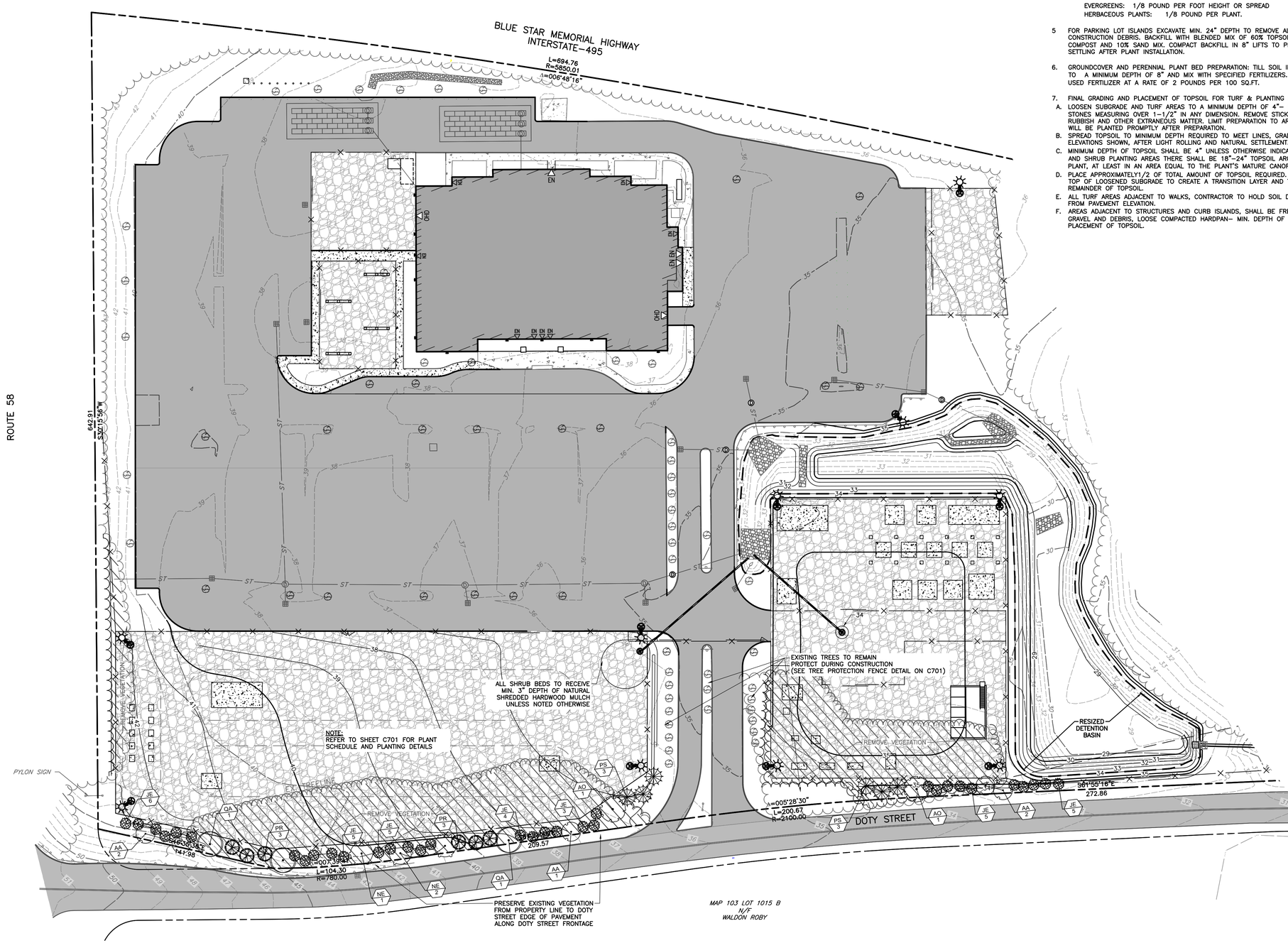
NORTH



SCALE IN FEET
0 40 80

PLANTING SOIL PREPARATION NOTES:

1. BEFORE MIXING, CLEAN TOPSOIL OF ROOTS, PLANTS, SODS, STONES, CLAY LUMPS, AND OTHER EXTRANEOUS MATERIALS HARMFUL TO PLANT GROWTH.
2. MIX SOIL AMENDMENTS AS SPECIFIED ON PLANTING DETAILS. MIX FERTILIZERS WITH TOPSOIL AT RATES INDICATED. DELAY MIXING FERTILIZER IF PLANTING DOES NOT FOLLOW PLACING OF PLANTING SOIL WITHIN A FEW DAYS.
3. FOR TREE PIT OR TRENCH BACKFILL, MIX PLANTING SOIL BEFORE BACKFILLING AND STOCKPILE AT SITE.
   PROVIDE SPECIFIED FERTILIZER AT A RATE OF:
   SHADE TREE: 2 POUNDS PER INCH OF CALIPER
   SMALL TREE: 1 POUND PER INCH OF CALIPER
4. FOR PLANTING BEDS, MIX PLANTING SOIL EITHER PRIOR TO PLANTING OR APPLY ON SURFACE OF TOPSOIL AND MIX THOROUGHLY BEFORE PLANTING.
   APPLY SPECIFIED COMMERCIAL FERTILIZER AT RATES AND THOROUGHLY MIX INTO UPPER 2" OF TOPSOIL. PROVIDE SPECIFIED FERTILIZER AT THE FOLLOWING RATES:
   SHRUBS: 1/4 POUND PER FOOT HEIGHT OR SPREAD
   EVERGREENS: 1/8 POUND PER FOOT HEIGHT OR SPREAD
   HERBACEOUS PLANTS: 1/8 POUND PER PLANT.
5. FOR PARKING LOT ISLANDS EXCAVATE MIN. 24" DEPTH TO REMOVE ALL CONSTRUCTION DEBRIS. BACKFILL WITH BLENDED MIX OF 60% TOPSOIL, 30% COMPOST AND 10% SAND MIX. COMPACT BACKFILL IN 8" LIFTS TO PREVENT SETTLING AFTER PLANT INSTALLATION.
6. GROUNDCOVER AND PERENNIAL PLANT BED PREPARATION: TILL SOIL IN BEDS TO A MINIMUM DEPTH OF 8" AND MIX WITH SPECIFIED FERTILIZERS. USED FERTILIZER AT A RATE OF 2 POUNDS PER 100 SQ.FT.
7. FINAL GRADING AND PLACEMENT OF TOPSOIL FOR TURF & PLANTING AREAS:
   A. LOOSEN SUBGRADE AND TURF AREAS TO A MINIMUM DEPTH OF 4"– 6". REMOVE STONES MEASURING OVER 1–1/2" IN ANY DIMENSION. REMOVE STICKS, ROOTS, RUBBISH AND OTHER EXTRANEOUS MATTER. LIMIT PREPARATION TO AREAS WHICH WILL BE PLANTED PROMPTLY AFTER PREPARATION.
   B. SPREAD TOPSOIL TO MINIMUM DEPTH REQUIRED TO MEET LINES, GRADES, AND ELEVATIONS SHOWN. AFTER LIGHT ROLLING AND NATURAL SETTLEMENT.
   C. MINIMUM DEPTH OF TOPSOIL SHALL BE 4" UNLESS OTHERWISE INDICATED. IN TREE AND SHRUB PLANTING AREAS THERE SHALL BE 18"–24" TOPSOIL AROUND THE PLANT, AT LEAST IN AN AREA EQUAL TO THE PLANT'S MATURE CANOPY SIZE.
   D. PLACE APPROXIMATELY1/2 OF TOTAL AMOUNT OF TOPSOIL REQUIRED. WORK INTO TOP OF LOOSENED SUBGRADE TO CREATE A TRANSITION LAYER AND THEN PLACE REMAINDER OF TOPSOIL.
   E. ALL TURF AREAS ADJACENT TO WALKS, CONTRACTOR TO HOLD SOIL DOWN 1" FROM PAVEMENT ELEVATION.
   F. AREAS ADJACENT TO STRUCTURES AND CURB ISLANDS, SHALL BE FREE OF GRAVEL AND DEBRIS, LOOSE COMPACTED HARDPAN– MIN. DEPTH OF 18" BEFORE PLACEMENT OF TOPSOIL.

GENERAL LANDSCAPE NOTES:

1. ALL CONSTRUCTION METHODS AND MATERIALS MUST CONFORM TO CURRENT STANDARDS AND SPECIFICATIONS OF THE FEDERAL, STATE, COUNTY, CITY, OR LOCAL REQUIREMENTS, WHICHEVER HAS JURISDICTION.
2. CONTRACTOR SHALL REQUEST UTILITY LOCATIONS PRIOR TO COMMENCEMENT OF WORK. VERIFY ALL EXISTING UTILITIES AND CONDITIONS PRIOR TO ANY EXCAVATION AT LEAST 72 HOURS PRIOR TO LANDSCAPE INSTALLATION.
3. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS IN THE FIELD PRIOR TO THE START OF CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL FIELD DIMENSIONS AND ELEVATIONS DURING THE ENTIRE CONSTRUCTION SCHEDULE. IF ANY DISCREPANCIES ARE FOUND IN THESE PLANS FROM ACTUAL FIELD DIMENSIONS, THE CONTRACTOR SHALL CONTACT THE ENGINEER IMMEDIATELY.
4. CONTRACTOR SHALL MAINTAIN ONE SET OF AS–BUILT/RECORD DRAWINGS ON THE JOB SITE DURING CONSTRUCTION FOR DISTRIBUTION TO THE OWNER AND/OR OWNER'S REPRESENTATIVE UPON COMPLETION.
5. NO CHANGES TO THE SITE LANDSCAPE LAYOUT ARE ALLOWED WITHOUT THE WRITTEN APPROVAL OF THE LANDSCAPE ARCHITECT.
6. CONTRACTOR SHALL SUPPLY ALL PLANT MATERIALS IN QUALITY AND QUANTITIES SUFFICIENT TO COMPLETE THE PLANTING AS SHOWN ON DRAWINGS. PLANT MATERIAL DELIVERED TO SITE SHALL BE STORED IN SECURE LOCATION NOT IN CONFLICT WITH OTHER CONSTRUCTION OPERATIONS AND MAINTAINED UNTIL TIME OF INSTALLATION.
7. ALL MATERIAL SHALL CONFORM TO THE GUIDELINES ESTABLISHED BY THE CURRENT "AMERICAN ASSOCIATION OF NURSERY STOCK. ANSI Z60.1–2004", PUBLISHED BY THE AMERICAN ASSOCIATION OF NURSERYMAN.
8. NO PLANT SHALL BE PUT INTO THE GROUND BEFORE ROUGH GRADING HAS BEEN FINISHED AND APPROVED.
9. ALL PLANTS SHALL BE PLANTED SO THAT THE ROOT CROWN IS PLANTED AT GRADE LEVEL.
10. ALL PLANTS SHALL BE BALLED AND WRAPPED OR CONTAINER GROWN AS SPECIFIED. NO CONTAINER GROWN STOCK WILL BE ACCEPTED IF IT IS ROOT BOUND. ALL ROOT WRAPPING MATERIAL MADE OF SYNTHETICS OR PLASTICS SHALL BE REMOVED AT THE TIME OF PLANTING. TWINE OR ROPE SHALL BE REMOVED FROM AROUND CROWN OF TRUNK TO PREVENT GIRDLING OF TREE OR SHRUB.
11. WITH CONTAINER GROWN STOCK, THE CONTAINER SHALL BE REMOVED AND THE CONTAINER BALL SHALL BE CUT THROUGH THE SURFACE IN TWO VERTICAL LOCATIONS.
12. CONTRACTOR TO REMOVE ALL LANDSCAPE DEBRIS FROM PLANTING OPERATIONS FROM THE PROJECT SITE. NO OPEN BURNING SHALL BE PERMITTED ON SITE.
13. THE DAY PRIOR TO PLANTING, THE LOCATIONS OF ALL TREES AND SHRUBS SHALL BE STAKED AND PLANTING BEDLINE OUTLINES DEFINED FOR APPROVAL BY OWNER(S).
14. THE LANDSCAPE CONTRACTOR SHALL REFER TO CONTRACT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
15. THE LANDSCAPE CONTRACTOR SHALL GUARANTEE NEW PLANT MATERIAL THROUGH TWO (2) CALENDAR YEARS FROM THE TIME OF SUBSTANTIAL COMPLETION OF PROJECT. AT THE END OF THIS PERIOD, ANY PLANT MATERIAL DEEMED UNSATISFACTORY BY LANDSCAPE ARCHITECT SHALL BE REPLACED AT NO ADDITIONAL COST TO OWNER.
16. IF THERE IS A DISCREPANCY BETWEEN THE PLANS AND THE PLANT SCHEDULE, THE PLANS SHALL TAKE PRECEDENCE.
17. CONTRACTOR SHALL REPAIR ANY DAMAGE TO PROPERTY FROM PLANTING OPERATIONS AT NO COST TO OWNER.
18. STAKES AND OR GUY WIRES SHALL BE REMOVED AFTER ONE (1) YEAR OF INSTALLATION.
19. ALL EXISTING LANDSCAPING SHALL BE MAINTAINED DURING CONSTRUCTION. ANY MATERIAL DEEMED DEAD OR UNSATISFACTORY BY LANDSCAPE ARCHITECT, WILL BE REPLACED EQUIVALENT IN SIZE AND SHAPE AT NO COST TO OWNER.
20. IF PLANT SPECIES SPECIFIED ARE FOUND TO BE UNAVAILABLE OR NOT IN SUFFICIENT QUANTITIES AT TIME OF PLANTING, THE CONTRACTOR MAY SUBSTITUTE SPECIES UPON WRITTEN APPROVAL BY LANDSCAPE ARCHITECT.
21. LANDSCAPING SHALL BE SUBSTANTIALLY INSTALLED PRIOR TO THE ISSUANCE OF OCCUPANCY PERMIT FOR THE PRINCIPAL STRUCTURE ON LOT, WEATHER PERMITTING, OR, IN THE EVENT OF ADVERSE WEATHER CONDITIONS, WITHIN NINETY (90) DAYS FOLLOWING THE COMMENCEMENT OF THE NEXT SUCCESSIVE PLANTING SEASON FOLLOWING ISSUANCE OF OCCUPANCY PERMIT.
22. ALL LANDSCAPE PLANTINGS TO BE MAINTAINED BY CONTRACTOR FOR 60 DAYS FOLLOWING FINAL INSPECTION BY LANDSCAPE ARCHITECT. MAINTENANCE TO INCLUDE WATERING, WEED REMOVAL, MULCHING, MOWING AND ALL OTHER NECESSARY OPERATIONS REQUIRED FOR PROPER ESTABLISHMENT OF PLANTINGS AND TURF AREAS.

REVISION RECORD

NO	DATE	DESCRIPTION
1.	03/08/2024	RESPONSE TO PEER REVIEW COMMENTS
2.	05/27/2024	RESPONSE TO SECOND PEER REVIEW COMMENTS

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EVERSOURCE ENERGY
37 DOTY STREET
WAREHAM, MASSACHUSETTS

LANDSCAPING PLAN

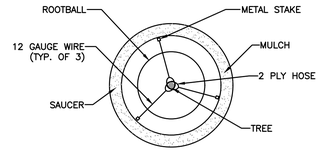
DATE:	OCTOBER 11, 2023
DWG SCALE:	1" = 40'
PROJECT NO:	323-322
APPROVED BY:	DRAFT
DRAWN BY:	DRAFT
CHECKED BY:	DRAFT

COMMONWEALTH OF MASSACHUSETTS
REBECCA TAYLOR
NO. 4352
REGISTERED LANDSCAPE ARCHITECT

DRAWING NO.:
C700
SHEET 9 OF 13

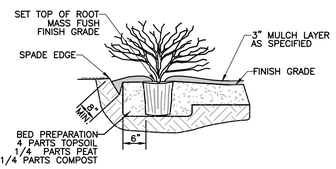
P:\320-001\323-322\1-0401\dwg\C101\323322-C101-C700.dwg(C700) LS (3/27/2024 - cwindenburger) - LD: 3/27/2024 2:41 PM

PLANT SCHEDULE							
TREES	CODE	QTY	BOTANICAL / COMMON NAME	CONTAINER	CALIPER	SIZE	
	AO	2	ACER RUBRUM 'OCTOBER GLORY' OCTOBER GLORY RED MAPLE	B&B	2.5" CAL.		
	AA	5	AMELANCHIER ARBOREA DOWNY SERVICEBERRY	B&B	2.5" CAL.		
	JE	33	JUNIPERUS VIRGINIANA EASTERN REDCEDAR	B&B		6'-7" HT.	
	NE	3	NYSSA SYLVATICA 'JFS-RED' FIRESTARTER® TUPELO	B&B	2.5" CAL.		
	PR	6	PINUS RIGIDA PITCH PINE	B&B		6'-7" HT.	
	PS	6	PINUS STROBUS WHITE PINE	B&B		6'-7" HT.	
	QA	3	QUERCUS ALBA WHITE OAK	B&B	2.5" CAL.		

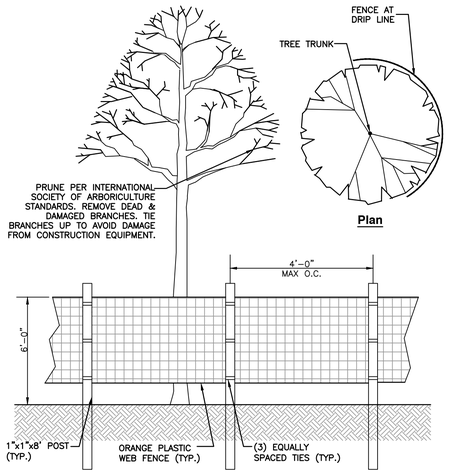


NOTE: LOCATE 1 STAKE DIRECTLY SOUTHWEST OF TREE TRUNK

DETAIL 703 - TREE STAKING DETAIL
NOT TO SCALE



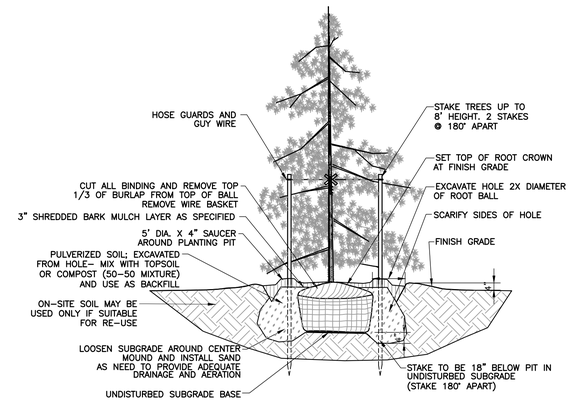
DETAIL 702- SHRUB PLANTING DETAIL
NOT TO SCALE



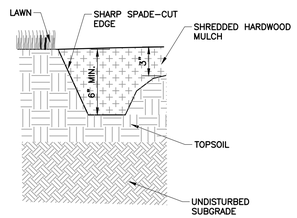
CONSTRUCTION NOTES

1. INSTALL TREE PROTECTION FENCE AT THE DRIP LINE OF EXISTING TREES TO REMAIN
2. IF DRIP LINES OVERLAP, INSTALL CONTINUOUS PROTECTION FENCE FOR MULTIPLE EXISTING TREE TO REMAIN.

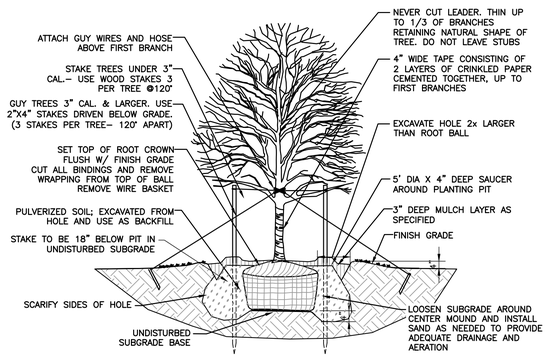
TREE PROTECTION FENCE
N.T.S.



DETAIL 701 - EVERGREEN TREE PLANTING DETAIL
NOT TO SCALE



DETAIL 704 - SPADE EDGING DETAIL
NOT TO SCALE



DETAIL 700- SHADE TREE PLANTING DETAIL
NOT TO SCALE

REVISION RECORD

NO	DATE	DESCRIPTION
1.	03/08/2024	RESPONSE TO PESRP REVIEW COMMENTS
2.	05/27/2024	RESPONSE TO SECOND PESRP REVIEW COMMENTS

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31 Bellows Road · Raynham, MA 02767
Ph: 774.501.2176 · 866.312.2024 · Fax: 774.501.2669
www.cecinc.com

EVERSOURCE ENERGY
37 DOTY STREET
WAREHAM, MASSACHUSETTS

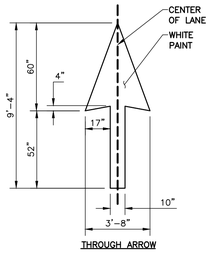
LANDSCAPE DETAILS

DATE:	OCTOBER 11, 2023	DRAWN BY:	KJ
DWG SCALE:	AS SHOWN	CHECKED BY:	CJV
PROJECT NO:			323-322
APPROVED BY:			DRAFT

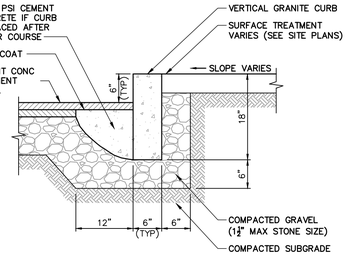
DRAWING NO.:
C701
SHEET 10 OF 13

MASSACHUSETTS REBECCA FELDER NO. 4352 REGISTERED LANDSCAPE ARCHITECT

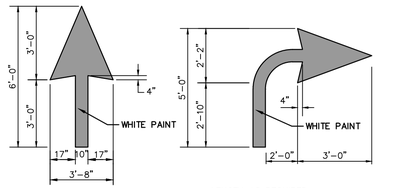
P:\320-000\323-322\-CAD\Dwg\CIVIL\323322-CIVIL-C700.dwg[C701] LS(3/27/2024 - cvandenburgh) - LP: 3/27/2024 2:41 PM

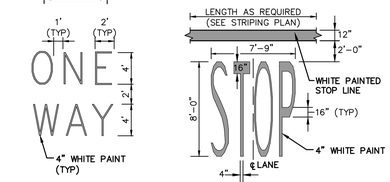


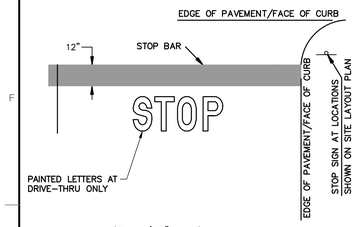
PAVEMENT STRIPING
N.T.S.

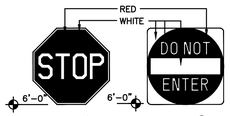


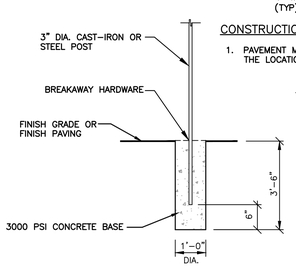
VERTICAL GRANITE CURB
N.T.S.





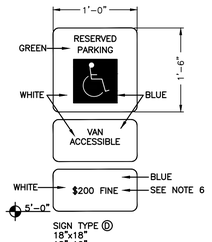






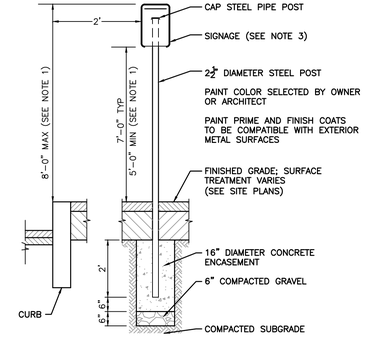
CONSTRUCTION NOTES
1. PAVEMENT MARKINGS TO BE INSTALLED FOR ON-SITE WORK IN THE LOCATIONS SHOWN ON THE LAYOUT AND MATERIALS PLAN.

PAINTED PAVEMENT MARKINGS
N.T.S.



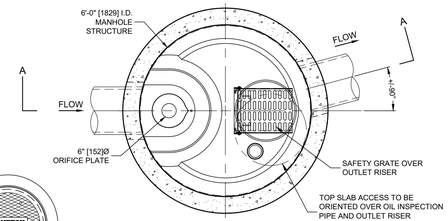
CONSTRUCTION NOTES:
1. SET POSTS IN CONCRETE TO A MINIMUM DEPTH OF 36". SIGN PANELS SHALL BE 0.100 ALUMINUM WITH RAISED OR SILKSCREEN COPY.
2. FOR POST MOUNTING, USE NON-CORROSIVE 3/8" 2. MACHINE BOLTS W/ WASHERS, 2 PER SIGN; OR IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.
3. FOR WALL MOUNTING, USE NON-CORROSIVE 3/8" LAG BOLTS W/ LEAD EXPANSION SHIELD, 4 PER SIGN.
4. ALL ACCESSIBLE PARKING SIGNAGE IS TO BE INSTALLED WITH THE BOTTOM EDGE OF THE LOWEST SIGN AT LEAST 60" ABOVE FINISHED GRADE.
5. FINE AMOUNT SHALL BE DISPLAYED IN ACCORDANCE WITH LOCAL LAW. THE AMOUNT SHOWN HEREON IS REPRESENTATIVE ONLY. COORDINATE POSTING OF FINE AMOUNT WITH LOCAL LAW ENFORCEMENT AGENCY.
6. STOP BAR MUST BE PAINTED WHITE
7. FOR ACCESSIBLE SIGNAGE, DIMENSIONS SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN THE MASSACHUSETTS ARCHITECTURAL ACCESS BOARD (ACB) RULES AND REGULATIONS (521 CMR).
8. SIGNAGE SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (M.U.T.C.D.).

SIGNS
N.T.S.

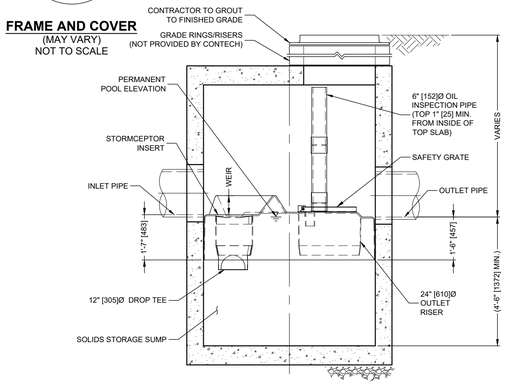


CONSTRUCTION NOTES
1. FOR ACCESSIBLE SIGNAGE, DIMENSION SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN THE THE MASSACHUSETTS ARCHITECTURAL ACCESS BOARD (AAB) RULES AND REGULATIONS (521 CMR).
2. SIGNS AND POSTS SHALL BE PER CITY OF QUINCY STANDARDS IF THE STANDARDS DIFFER FROM THE DETAIL DEPICTED HEREON.
3. SIGNAGE SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (M.U.T.C.D.).

SIGN POST
N.T.S.



PLAN VIEW
TOP SLAB NOT SHOWN



FRAME AND COVER
(MAY VARY)
NOT TO SCALE

SECTION A-A

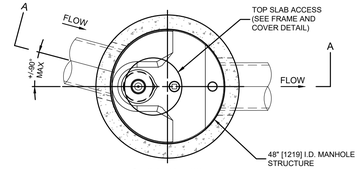
GENERAL NOTES
1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
2. FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS LLC REPRESENTATIVE. www.ContechES.com
3. STORMCEPTOR WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS OF PROJECT.
4. STORMCEPTOR STRUCTURE SHALL MEET AASHTO HS20 LOAD RATING, ASSUMING EARTH COVER OF 0' - 2' [610], AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 AND BE CAST WITH THE CONTECH LOGO.
5. STORMCEPTOR STRUCTURE SHALL BE PRECAST CONCRETE CONFORMING TO ASTM C478 AND AASHTO LOAD FACTOR DESIGN METHOD.
6. ALTERNATE UNITS ARE SHOWN IN MILLIMETERS [mm].

INSTALLATION NOTES
A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STORMCEPTOR MANHOLE STRUCTURE.
C. CONTRACTOR TO INSTALL JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS AND ASSEMBLE STRUCTURE.
D. CONTRACTOR TO PROVIDE, INSTALL, AND GROUT INLET AND OUTLET PIPE(S). MATCH PIPE INVERTS WITH ELEVATIONS SHOWN. ALL PIPE CENTERLINES TO MATCH PIPE OPENING CENTERLINES.
E. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ASSURE UNIT IS WATER TIGHT, HOLDING WATER TO FLOWLINE INVERT MINIMUM. IT IS SUGGESTED THAT ALL JOINTS BELOW PIPE INVERTS ARE GROUTED.

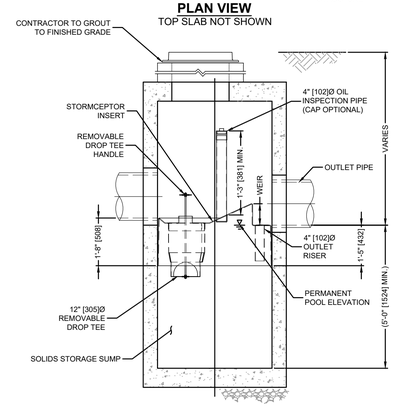
STORMCEPTOR STC 900 DETAIL
(NOT TO SCALE)

FRAME AND COVER
(MAY VARY)
NOT TO SCALE

FRAME AND GRATE
(MAY VARY)
NOT TO SCALE

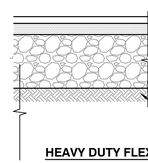


PLAN VIEW
TOP SLAB NOT SHOWN



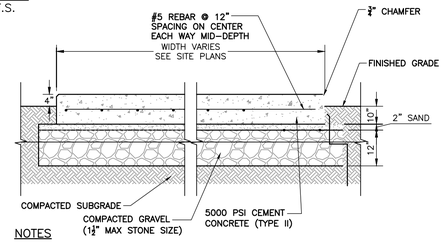
SECTION A-A

STORMCEPTOR STC 450i DETAIL
(NOT TO SCALE)



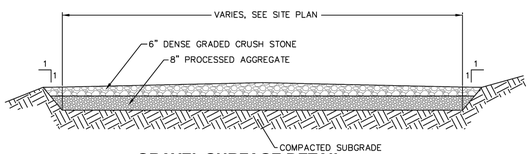
HEAVY DUTY FLEXIBLE PAVEMENT

BITUMINOUS CONCRETE PAVEMENT
N.T.S.

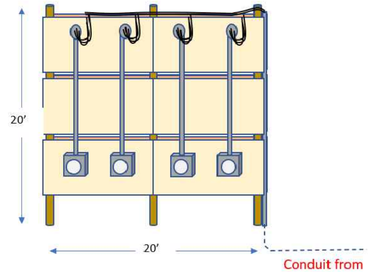


NOTES
1. SIZE (LENGTH AND WIDTH) OF GENERATOR PAD TO BE AS INDICATED ON SITE PLANS.
2. ALL EXPOSED TOP SURFACE EDGES SHALL HAVE 3/4" CHAMFER.

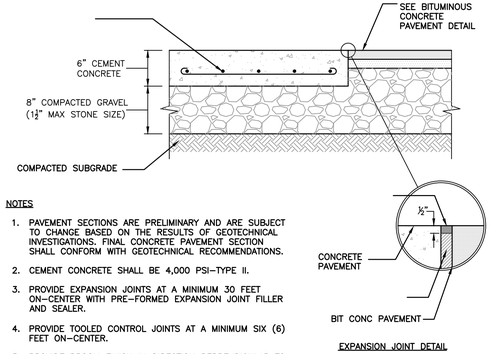
CONCRETE PAD GENERATOR
N.T.S.



GRAVEL SURFACE DETAIL
N.T.S.

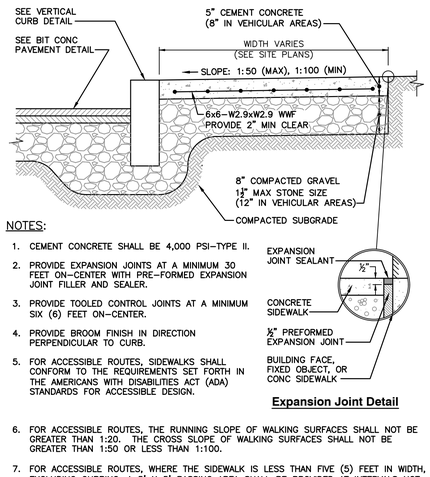


3-POLE STRUCTURE DETAIL
N.T.S.



NOTES
1. PAVEMENT SECTIONS ARE PRELIMINARY AND ARE SUBJECT TO CHANGE BASED ON THE RESULTS OF GEOTECHNICAL INVESTIGATIONS. FINAL CONCRETE PAVEMENT SECTION SHALL CONFORM WITH GEOTECHNICAL RECOMMENDATIONS.
2. CEMENT CONCRETE SHALL BE 4,000 PSI-TYPE II.
3. PROVIDE EXPANSION JOINTS AT A MINIMUM 30 FEET ON-CENTER WITH PRE-FORMED EXPANSION JOINT FILLER AND SEALER.
4. PROVIDE TOOLED CONTROL JOINTS AT A MINIMUM SIX (6) FEET ON-CENTER.
5. PROVIDE BROOM FINISH IN DIRECTION PERPENDICULAR TO CURB.

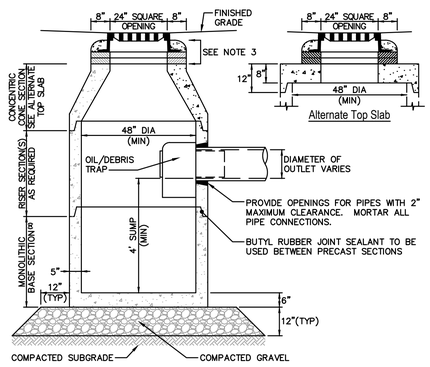
CONCRETE PAVEMENT
N.T.S.



NOTES:
1. CEMENT CONCRETE SHALL BE 4,000 PSI-TYPE II.
2. PROVIDE EXPANSION JOINTS AT A MINIMUM 30 FEET ON-CENTER WITH PRE-FORMED EXPANSION JOINT FILLER AND SEALER.
3. PROVIDE TOOLED CONTROL JOINTS AT A MINIMUM SIX (6) FEET ON-CENTER.
4. PROVIDE BROOM FINISH IN DIRECTION PERPENDICULAR TO CURB.
5. FOR ACCESSIBLE ROUTES, SIDEWALKS SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN THE AMERICANS WITH DISABILITIES ACT (ADA) STANDARDS FOR ACCESSIBLE DESIGN.
6. FOR ACCESSIBLE ROUTES, THE RUNNING SLOPE OF WALKING SURFACES SHALL NOT BE GREATER THAN 1:20. THE CROSS SLOPE OF WALKING SURFACES SHALL NOT BE GREATER THAN 1:50 OR LESS THAN 1:100.
7. FOR ACCESSIBLE ROUTES, WHERE THE SIDEWALK IS LESS THAN FIVE (5) FEET IN WIDTH, EXCLUDING CURBING, A 5' X 5' PASSING AREA SHALL BE PROVIDED AT INTERVALS NOT TO EXCEED 200 FEET.
8. FOR ACCESSIBLE ROUTES, A MINIMUM CLEARANCE OF THREE (3) FEET SHALL BE MAINTAINED AT ANY PERMANENT OBSTACLE; HYDRANTS, UTILITY POLES, SIGN POSTS, ETC.

Expansion Joint Detail

CONCRETE SIDEWALK
N.T.S.



CONSTRUCTION NOTES
1. STRUCTURE TO BE PRECAST CONCRETE, MINIMUM 4,000 PSI. ALL SECTIONS TO BE DESIGNED TO MEET OR EXCEED HS-20 LOADING.
2. BASE TO BE SINGLE POUR MONOLITHIC SECTION.
3. FRAME AND GRATE TO BE SET IN FULL MORTAR BED. ADJUST TO GRADE WITH CLAY BRICK AND MORTAR. MAXIMUM OF FIVE BRICK COURSES.

SINGLE CATCH BASIN (SCB) WITH OIL / DEBRIS TRAP
N.T.S.

COMMONWEALTH OF MASSACHUSETTS
Brian E. Paves
CIVIL
NO. 47251
REGISTERED PROFESSIONAL ENGINEER
5/27/24

REVISION RECORD

NO	DATE	DESCRIPTION
1.	3/8/2024	RESPONSE TO PESP1 REVIEW COMMENTS
2.	05/27/2024	RESPONSE TO SECOND PESP1 REVIEW COMMENTS

CEC
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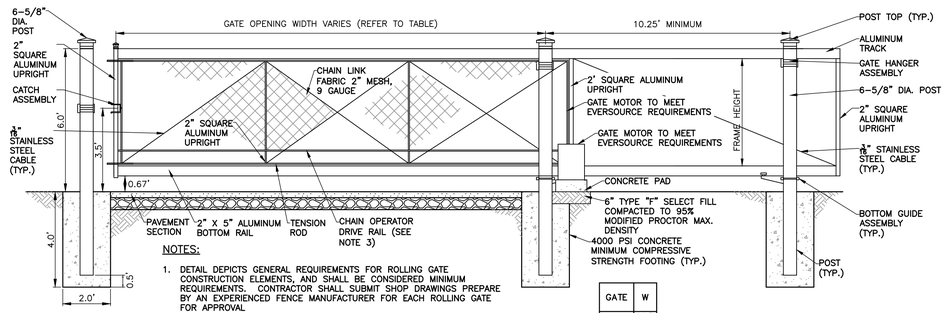
www.cecinc.com

EVERSOURCE ENERGY
37 DOTY STREET
WAREHAM, MASSACHUSETTS

DETAIL SHEET 1

DATE:	OCTOBER 11, 2023
DWG SCALE:	NOT TO SCALE
DRAWN BY:	KJ
CHECKED BY:	CJV
PROJECT NO:	323-322
APPROVED BY:	BEP

DRAWING NO.:
C800
SHEET 11 OF 13



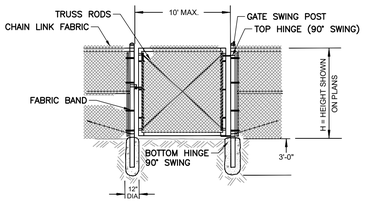
NOTES:

1. DETAIL DEPICTS GENERAL REQUIREMENTS FOR ROLLING GATE CONSTRUCTION ELEMENTS, AND SHALL BE CONSIDERED MINIMUM REQUIREMENTS. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS PREPARE BY AN EXPERIENCED FENCE MANUFACTURER FOR EACH ROLLING GATE FOR APPROVAL
2. FENCING AND GATES TO COMPLY WITH MASSDOT SPECIFICATIONS.
3. CONTRACTOR TO COORDINATE GATE OPERATOR DRIVE RAIL WITH GATE OPERATOR DRIVE MANUFACTURER.
4. CONTRACTOR SHALL COORDINATE ELECTRIC SERVICE REQUIREMENTS FOR GATE WITH MOTOR MANUFACTURER.

GATE	W
A	25'
B	30'
C	20'
D	20'
E	20'
F	20'
G	20'
H	20'

SLIDING FENCE GATE DETAIL

N.T.S.

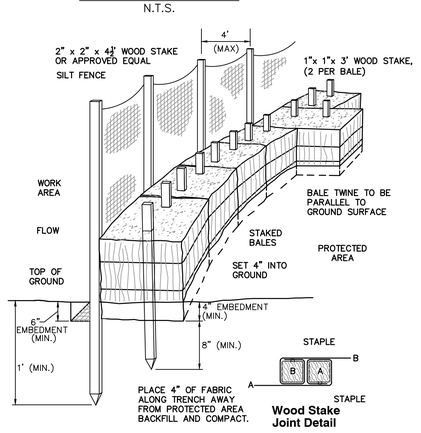


SINGLE GATE

GATE SWING POST DIA.	
GATE SIZE	PIPE DIA.
6' AND LESS	3"O.D. – 5.79 LBS./FT.
6' – 10'	4"O.D. – 9.10 LBS./FT.

CHAIN LINK GATES

N.T.S.



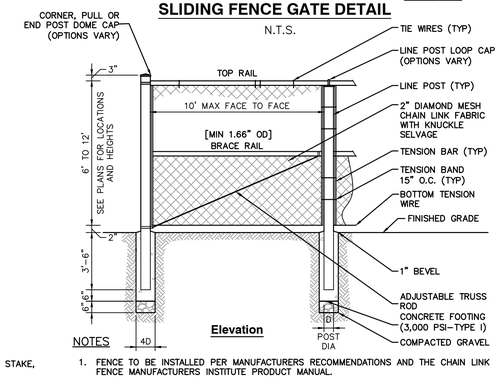
Wood Stake Joint Detail

CONSTRUCTION NOTES

1. PLACE ONE STRAW BALE PERPENDICULAR ALONG STRAW BALE BARRIER (100' O.C.)

SILT FENCE/STRAW BALE BARRIER

N.T.S.



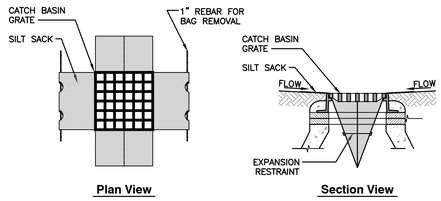
Elevation

NOTES

1. FENCE TO BE INSTALLED PER MANUFACTURERS RECOMMENDATIONS AND THE CHAIN LINK FENCE MANUFACTURERS INSTITUTE PRODUCT MANUAL.
2. POSTS SHALL MAINTAIN A MINIMUM DEPTH OF 3'–6" IN GROUND AND SHALL NOT BE RACKED TO ACCOMMODATE CHANGES IN GRADE.
3. LINE OF FENCE, TOP AND BOTTOM, SHALL BE INSTALLED STRAIGHT AND TRUE. POSTS SHALL BE INSTALLED PARALLEL AND PLUMB. RAILS SHALL BE INSTALLED PARALLEL TO GROUND SURFACE AND EACH OTHER.

6'-12' CHAIN LINK FENCE

N.T.S.



Plan View **Section View**

CONSTRUCTION NOTES

1. INSTALL SILT SACKS IN ALL CATCH BASINS WHERE INDICATED ON THE SITE PLANS BEFORE COMMENCING WORK OR IN PAVED AREAS AFTER THE BINDER COURSE IS PLACED AND EROSION CONTROL BARRIERS HAVE BEEN REMOVED.
2. GRATE TO BE PLACED OVER SILT SACK.
3. SILT SACKS SHALL BE INSPECTED PERIODICALLY AND AFTER ALL STORM EVENTS. CLEANING OR REPLACEMENT SHALL BE PERFORMED AS NEEDED. MAINTAIN SILT SACKS UNTIL UPSTREAM AREAS HAVE BEEN PERMANENTLY STABILIZED.

SILT SACK INLET PROTECTION

N.T.S.

GRAVEL FILTER

NOT TO SCALE

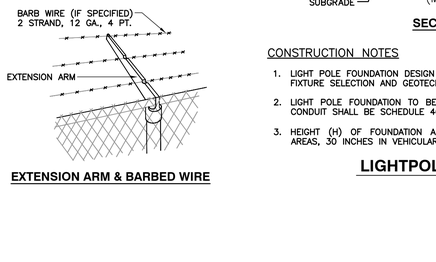
STORM ELEVATIONS:
2 YEAR STORM ELEVATION=32.23
10 YEAR STORM ELEVATION=33.26
25 YEAR STORM ELEVATION=34.00
100YEAR STORM ELEVATION=34.76

CROSS SECTION

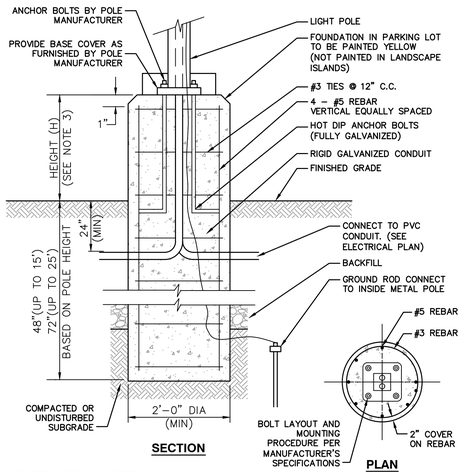
WET BASIN DETAIL

NOT TO SCALE

NOTE: RE–GRADE BASIN SIDE SLOPES AS SHOWN ON THE SITE PLANS. BASIN BOTTOM AREA TO BE EXPANDED AS SHOWN ON THE SITE PLANS AND SHALL BE COMPRISED OF NEW ENGLAND EROSION CONTROL/ RESTORATION MIX FOR DETENTION BASIN AND MOIST SITES.



EXTENSION ARM & BARBED WIRE



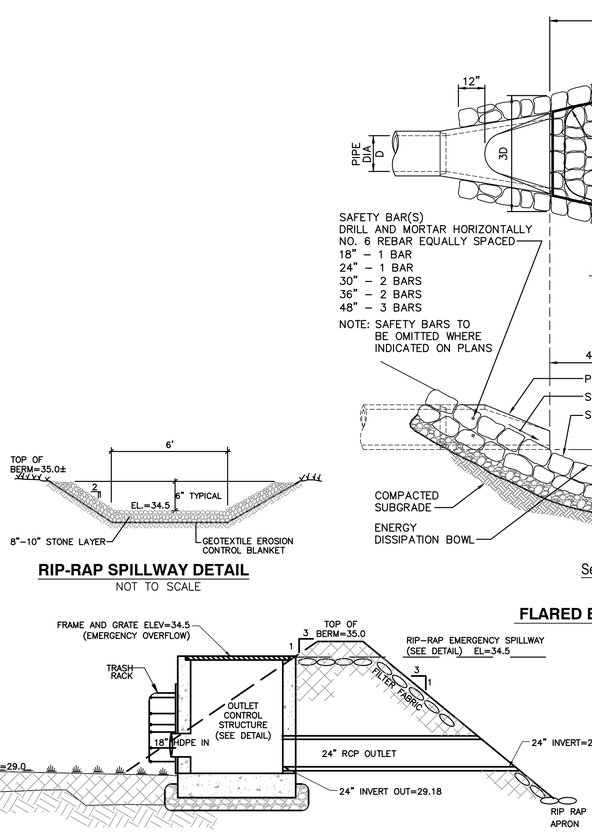
SECTION **PLAN**

CONSTRUCTION NOTES

1. LIGHT POLE FOUNDATION DESIGN IS SUBJECT TO CHANGE BASED ON FINAL POLE HEIGHT AND FIXTURE SELECTION AND GEOTECHNICAL SITE INVESTIGATIONS.
2. LIGHT POLE FOUNDATION TO BE PRECAST CONCRETE, MINIMUM 4,000 PSI. UNDERGROUND CONDUIT SHALL BE SCHEDULE 40 PVC.
3. HEIGHT (H) OF FOUNDATION ABOVE FINISHED GRADE TO BE 6 INCHES IN LANDSCAPED AREAS, 30 INCHES IN VEHICULAR AREAS, AND FLUSH IN SIDEWALKS.

LIGHTPOLE FOUNDATION DETAIL

N.T.S.



Plan

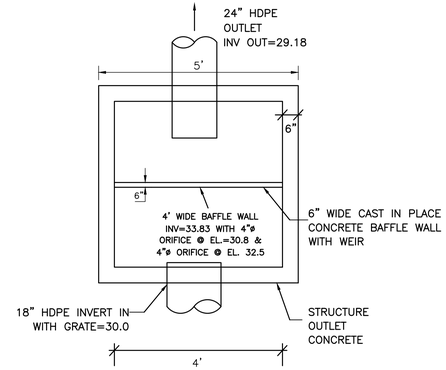
Section

FLARED END SECTION

N.T.S.

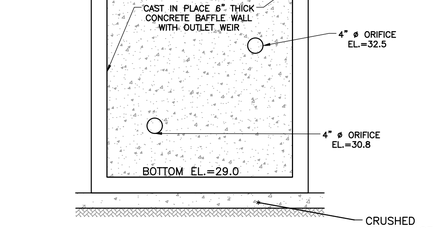
RIP-RAP SPILLWAY DETAIL

NOT TO SCALE



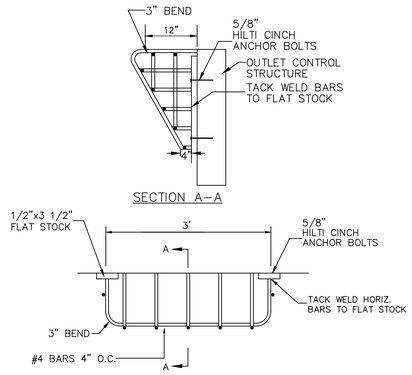
OUTLET CONTROL STRUCTURE PLAN VIEW

(NOT TO SCALE)



OUTLET CONTROL STRUCTURE

(NOT TO SCALE)



ALL COMPONENTS TO BE HOT DIPPED GALVANIZED OR STAINLESS STEEL 304

TRASH RACK

NOT TO SCALE

REVISION RECORD

NO	DATE	DESCRIPTION
1.	3/8/2024	RESPONSE TO PESP1 REVIEW COMMENTS
2.	05/27/2024	RESPONSE TO SECOND PESP1 REVIEW COMMENTS

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www.cecinc.com

EVERSOURCE ENERGY
37 DOTY STREET
WAREHAM, MASSACHUSETTS

DETAIL SHEET 2

DRAWN BY:	KJ
CHECKED BY:	CJV
DATE:	OCTOBER 11, 2023
DWG SCALE:	NOT TO SCALE
PROJECT NO.:	323-322
APPROVED BY:	BEP

Brian E. Polvin CIVIL NO. 47231 REGISTERED PROFESSIONAL ENGINEER COMMONWEALTH OF MASSACHUSETTS
5/27/24

DRAWING NO.: C801

SHEET 12 OF 13

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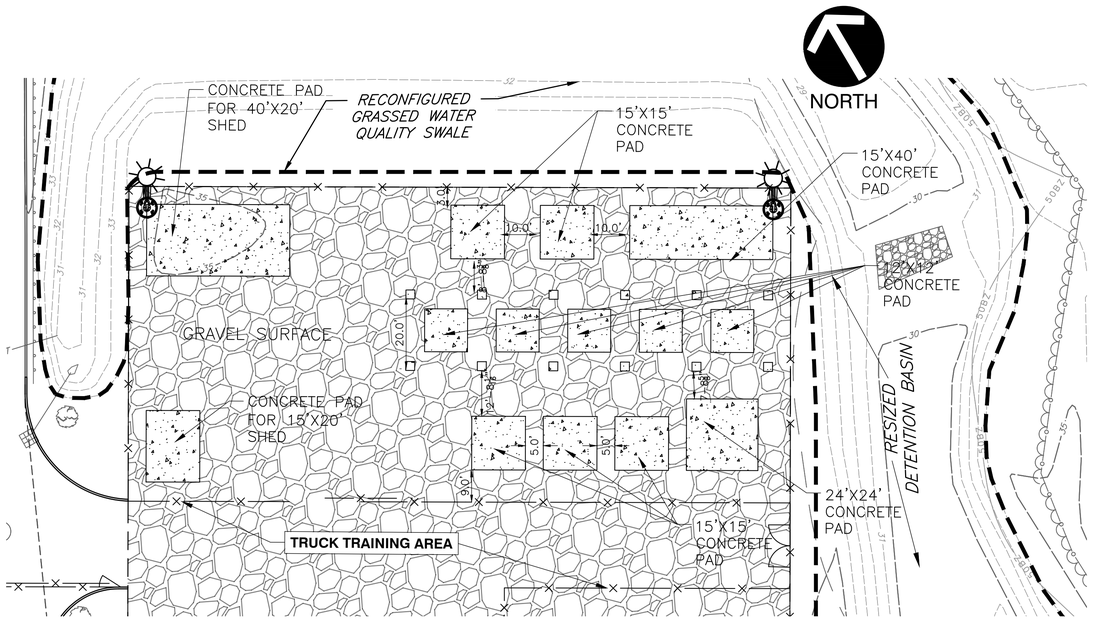
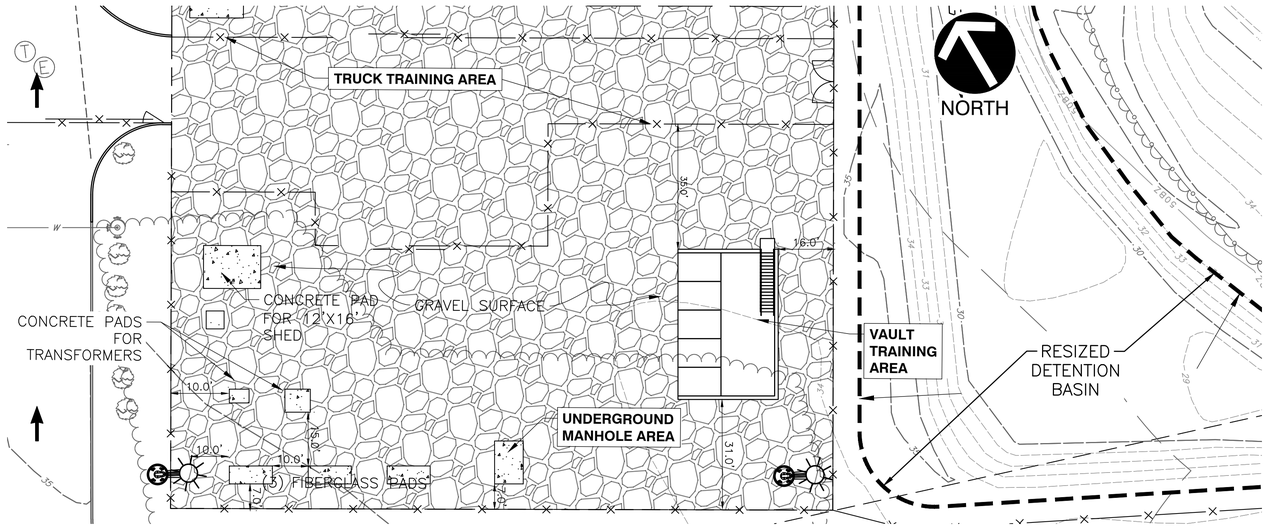
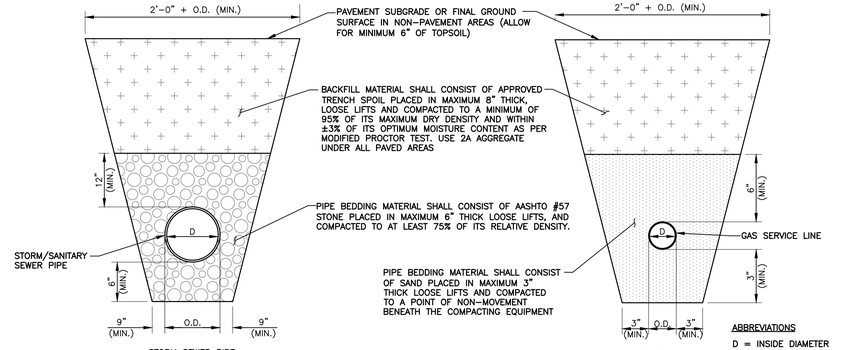


SUBSTATION TRAINING AREA DETAIL
SCALE: 1"=20'

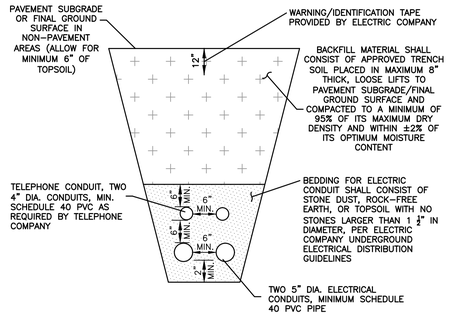


UNDERGROUND TRAINING AREA DETAIL
SCALE: 1"=20'



ABBREVIATIONS

D = INSIDE DIAMETER
O.D. = OUTSIDE DIAMETER



ELECTRIC AND TELEPHONE LINES (TYP.)

COVER REQUIREMENTS

UTILITY	MINIMUM COVER REQUIRED *
STORM SEWER	2.0 FT.
SANITARY SEWER	4.0 FT.
WATERLINE	4.0 FT.
GAS LINE	2.0 FT.
TELEPHONE LINE	2.5 FT.
ELECTRIC LINE	3.75 FT.

* AS MEASURED FROM TOP OF PIPE TO FINAL GROUND SURFACE VEGETATED AREA

CONSTRUCTION NOTES:

1. FINAL CONDUIT CONFIGURATION TO BE SPECIFIED BY THE TELEPHONE AND ELECTRIC COMPANY DESIGNS FOR SITE.
2. A ½" HIGH STRENGTH PULL ROPE SHALL BE INSTALLED FOR BOTH ELECTRIC AND TELEPHONE.

UTILITY TRENCH DETAILS
N.T.S.

CONSTRUCTION NOTES:

1. ALL MATERIALS EXCAVATED FROM THE UTILITY TRENCH SHALL BE STOCKPILED A MINIMUM SUFFICIENT DISTANCE FROM ALL TRENCHES TO PREVENT SLIDES OR CAVE-INS.
2. ALL BACKFILL MATERIALS SHALL BE APPROVED BY THE GENERAL CONTRACTOR OR HIS REPRESENTATIVE BEFORE BEING PLACED.
3. THE AASHTO #57 STONE SHALL BE CRUSHED LIMESTONE THAT SATISFIES THE REQUIREMENTS OF PADOT PUBLICATION 408, SECTION 703.
4. THE MAXIMUM DRY DENSITY AND OPTIMUM MOISTURE CONTENT FOR THE BACKFILL MATERIALS SHALL BE DETERMINED BY ASTM D1557, AND THE RELATIVE DENSITY OF THE AASHTO #57 STONE SHALL BE DETERMINED BY ASTM D4253 AND ASTM D4254.
5. THE CONTRACTOR SHALL CONSTRUCT THE UTILITY TRENCHES AND PROVIDE ADEQUATE SHORING (WHERE NECESSARY) IN CONFORMANCE WITH THE LATEST REQUIREMENTS FOR CONSTRUCTION STANDARD FOR EXCAVATIONS (29 CFR PART 1926.650-.652 SUBPART P) PROMULGATED BY OSHA.
6. THE CONTRACTOR SHALL VERIFY THAT THE MINIMUM SPECIFIED PIPE COVER IS PROVIDED BETWEEN THE FINAL GROUND SURFACE AND TOP OF PIPE BEFORE LAYING PIPE. PROVIDE A MINIMUM OF 3 FT. OF COVER ABOVE ALL PIPES DURING CONSTRUCTION.
7. THE TELEPHONE AND ELECTRIC CONDUIT WILL NOT SHARE THE SAME UTILITY TRENCH WHERE THEY ARE SEPARATED AT THE MANHOLE/VAULT LOCATIONS.
8. INCREASE TRENCH WIDTH AS NECESSARY TO ALLOW FOR PROPER COMPACTION OF BEDDING/BACKFILL.
9. HDPE STORM SEWER PIPE SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS' RECOMMENDATIONS.

REVISION RECORD

NO	DATE	DESCRIPTION
1.	3/8/2024	RESPONSE TO PESER REVIEW COMMENTS
2.	03/27/2024	RESPONSE TO SECOND PESER REVIEW COMMENTS

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EVERSOURCE ENERGY
37 DOTY STREET
WAREHAM, MASSACHUSETTS

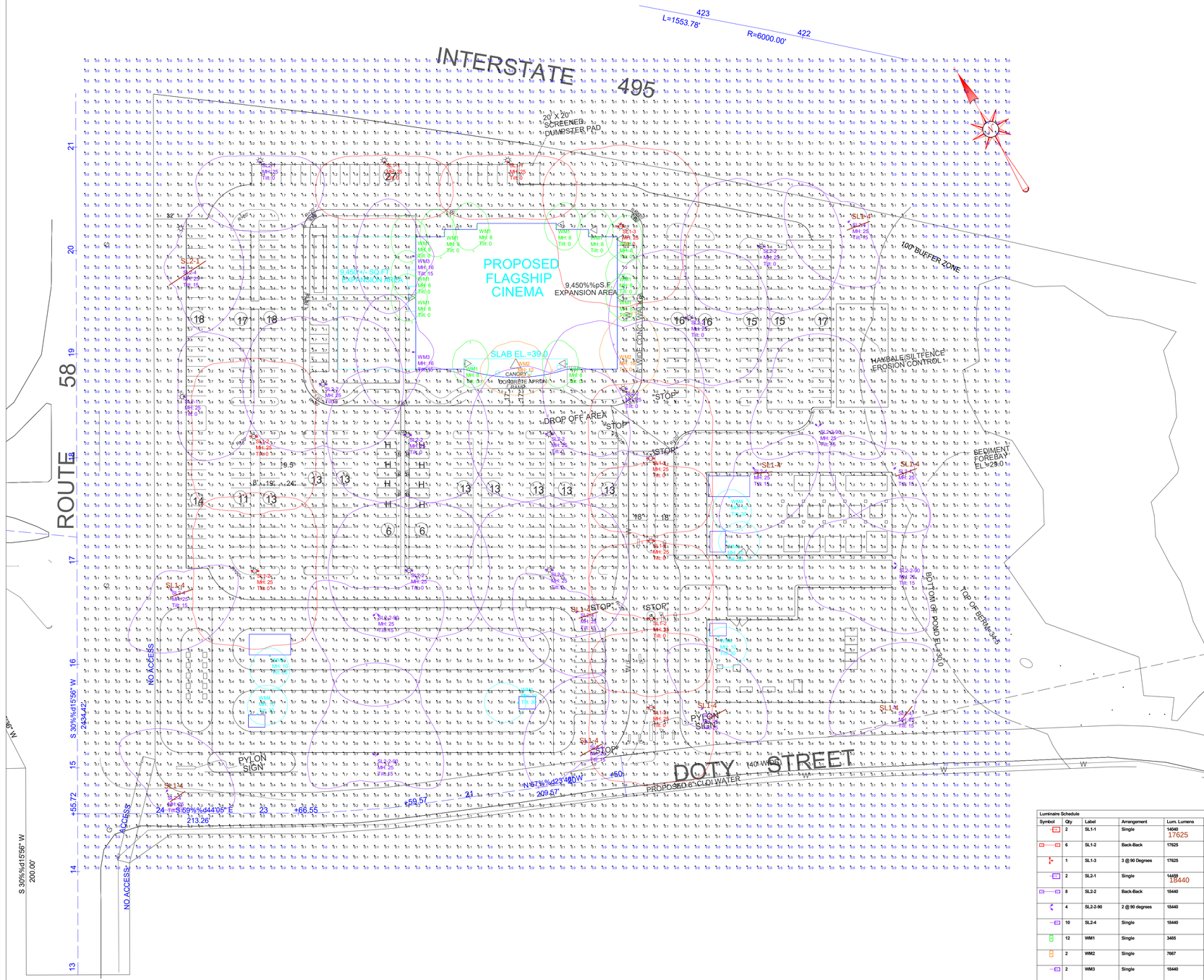
DETAIL SHEET 3

DATE:	OCTOBER 11, 2023
DWG SCALE:	AS SHOWN
PROJECT NO.	323-322
APPROVED BY:	DRAFT
DRAWN BY:	CJV
CHECKED BY:	BEP

DRAWING NO.:
C802
SHEET 13 OF 13

COMMONWEALTH OF MASSACHUSETTS
Brian E. Potvin
CIVIL
NO. 47231
REGISTERED PROFESSIONAL ENGINEER
3/27/24

P:\320-000\323-322\CADD\Dwg\Civil\323322-Civil-C800.dwg[C802] 15:13:27/2024 - cvondenbergh - LP: 3/27/2024 2:42 PM



Scale: 1 inch= 50 Ft.

Luminaire Schedule								
Symbol	Qty	Label	Arrangement	Lum. Lumens	Lum. Watts	LLF	Description	[MANUFAC]	Filename
	2	SL1-1	Single	14040 17625	135.1 135	0.850	ECF-S-48L-900-NW-G2-AR-3-UNV-BL-IMR7-PCB-HIS-FINISH / SSS-CB-5-7-25-D1-DTX-FINISH	Gardco	ecf-s-48l-900-nw-g2-3-his.ies
	6	SL1-2	Back-Back	17625	135.1	0.850	ECF-S-48L-900-NW-G2-AR-3-UNV-BL-IMR7-PCB-FINISH / SSS-CB-5-7-25-D3-DTX-FINISH	SIGNIFY GARDCO	ecf-s-48l-900-nw-g2-3.ies
	1	SL1-3	3 @ 90 Degrees	17625	135.1	0.850	ECF-S-48L-900-NW-G2-AR-3-UNV-BL-IMR7-PCB-FINISH / SSS-CB-5-7-25-D3-DTX-FINISH	SIGNIFY GARDCO	ecf-s-48l-900-nw-g2-3.ies
	2	SL2-1	Single	14400 18440	135.1	0.850	ECF-S-48L-900-NW-G2-AR-4-UNV-BL-IMR7-PCB-HIS-FINISH / SSS-CB-5-7-25-TDH-DTX-FINISH	Gardco	ecf-s-48l-900-nw-g2-4-his.ies
	8	SL2-2	Back-Back	18440	135.1	0.850	ECF-S-48L-900-NW-G2-AR-4-UNV-BL-IMR7-PCB-FINISH / SSS-CB-5-7-25-D2-DTX-FINISH	SIGNIFY GARDCO	ecf-s-48l-900-nw-g2-4.ies
	4	SL2-2-90	2 @ 90 degrees	18440	135.1	0.850	ECF-S-48L-900-NW-G2-SF-4-UNV-BL-IMR7-PCB-FINISH / SBRKT-RBH-L2.35-T2D4L-FINISH / SSS-CB-5-7-25-T2D4L-FINISH	SIGNIFY GARDCO	ecf-s-48l-900-nw-g2-4.ies
	10	SL2-4	Single	18440	135.1	0.850	ECF-S-48L-900-NW-G2-SF-4-UNV-BL-IMR7-PCB-FINISH / SSS-CB-5-7-25-T2D4L-FINISH	SIGNIFY GARDCO	ecf-s-48l-900-nw-g2-4.ies
	12	WM1	Single	3485	23	0.850	GWM-A07-630-T4M-VOLT-PCB-FINISH	SIGNIFY GARDCO	GWM-A07-630-T4M.ies
	2	WM2	Single	7667	51.6	0.850	GWM-A11-840-T4M-VOLT-PCB-FINISH	SIGNIFY GARDCO	GWM-A11-840-T4M.ies
	2	WM3	Single	18440	135.1	0.850	ECF-S-48L-900-NW-G2-SF-4-UNV-BL-IMR7-PCB-FINISH / SBRKT-RAH-L1-14/8-WA-FINISH	SIGNIFY GARDCO	ecf-s-48l-900-nw-g2-4.ies
	6	WM4	Single	7339	52.6	0.850	FLDS-A04-740-WFL-SLF-VOLT-PCB-FINISH / SBRKT-RAH-L1-14/8-WA-FINISH	GARDCO	FLDS-A04-740-WFL.ies

Calculation Summary								
Label	CalcType	Units	Avg	Max	Min	Avg/Min	Max/Min	Description
Site	Illuminance	Fc	1.23	26.4	0.0	N.A.	N.A.	10ft Grid
Entrance Drive	Illuminance	Fc	4.12	6.3	1.3	3.17	4.85	10ft Grid
Parking Lot	Illuminance	Fc	2.22	8.3	0.3	7.40	27.67	10ft Grid
Pole Training Area	Illuminance	Fc	1.59	26.4	0.3	5.30	88.00	10ft Grid
Substation Training Area	Illuminance	Fc	2.18	22.6	0.6	3.63	37.67	10ft Grid
Underground Training Area	Illuminance	Fc	1.29	24.6	0.4	3.23	61.50	10ft Grid

Revisions			
Rev	Date	Comments	
A	07/28/23	Initial Layout	
B	08/16/23	Revised per markups	
..	.....	.....	
..	.....	.....	
..	.....	.....	

Project Name: Eversource
Wareham, MA

Drawn by: PMD

Date: 8/16/2023

Project ID#: APX-11231

Rev: B

Prepared by
LID LIGHTING DESIGNS BY DUGAN

C803

STORMWATER REPORT

STORMWATER REPORT

EVERSOURCE

37 DOTY STREET
WAREHAM, MASSACHUSETTS 02576

Applicant:

NSTAR ELECTRIC COMPANY
DBA EVERSOURCE ENERGY
247 STATION DRIVE
WESTWOOD, MA 02090

Prepared By:

CIVIL & ENVIRONMENTAL CONSULTANTS, INC.
31 BELLOWS ROAD
RAYNHAM, MASSACHUSETTS 02767

CEC Project 323-322

October 2023
Rev. March 2024

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FIGURES

APPENDICES

1.0 PROJECT NARRATIVE

1.1 INTRODUCTION

On behalf of NSTAR Electric Company (DBA Eversource Energy), (the "Applicant"), Civil & Environmental Consultants, Inc. (CEC) has prepared this stormwater report and analysis to demonstrate compliance with the Massachusetts Department of Environmental Protection (MassDEP) Stormwater Management Standards

The Applicant is proposing to redevelop the existing Flagship Cinema site at 37 Doty St. (Assessors Map 103 Lots A1, B1, C1, and D1) as an Eversource Training Facility and will serve as a staging area for emergency response vehicles on an as needed basis. The proposed project will include new gravel areas to serve as various electrified/non-electrified training zones with associated permanent equipment, poles, and structures. In addition, the existing cinema building will be redeveloped into an indoor training area with classrooms and offices (the "Project").

1.2 EXISTING CONDITIONS

The former Flagship Cinemas previously operated on the site. The site is bounded by Doty Street to the south, Route 58 to the west and Route 495 to the north. The existing grounds of the facility consist mostly of pavement, with sidewalks and some vegetation along the north and south property boundaries, with a wetland and well established wooded area to the east.

The former cinema was served by water, electric and gas and a private septic system.

Stormwater is currently collected in an existing stormwater management system, which consists of a series of catch basins and manholes that ultimately discharge into a stormwater detention basin. The stormwater basin discharges overland, eventually reaching the bordering vegetated wetland. See Figure 1 for a Site Locus Map.

1.3 PROPOSED PROJECT

The proposed project will redevelop the cinema into an Eversource Training Facility that will also serve as a staging area for emergency response vehicles on an as-needed basis. The proposed improvements at the site will include new gravel area to serve as various electrified/non-electrified training zones with associated permanent equipment, poles, and structures. In addition, the existing cinema on site will be redeveloped into an indoor training area with classrooms and offices. Other improvements include new light poles with security cameras, installation of new fencing, enlarging the existing stormwater basin removing the existing grassed parking islands and curbing, and installing new landscaping.

The redevelopment will result in a net increase in impervious areas. Stormwater management improvements will be constructed as part of the proposed redevelopmet to manage the stormwater runoff generated from the additional impervious areas. The improvements will be incorporated into the existing stormwater management system while maintaining the overall site stormwater drainage patterns.

2.0 STORMWATER MANAGEMENT SYSTEM

2.1 DESCRIPTION OF RUNOFF CONTROLS

The stormwater management improvements consist of components designed to manage runoff from the Site. These components attenuate runoff discharge peaks, minimize erosion, minimize the transport of sediments, improve water quality, and prevent impacts to the municipal drainage system and any downstream resource areas.

The stormwater management system implements a treatment drain of the Best Management Practices designed to provide 90% TSS (Total Suspended Solids) removal and 50% phosphorus removal for stormwater runoff from all impervious areas. The proposed stormwater management system will use the following specific control measures:

- Deep Sump/Hooded Catch Basin: Deep sump hooded catch basins are designed to remove trash, debris, and coarse sediment from stormwater runoff. Sheet flow from paved areas will be directed toward the deep sump hooded catch basins. Existing catch basins at the site will be inspected to confirm they include sumps and will be replaced with sumped catch basins, if required.

- Proprietary particle separators (Stormceptor water quality units): The proposed Stormceptor water quality units provide efficient removal of free oils, debris, and total suspended solids (TSS). Although not the main objective of the water quality unit, some removal of heavy metals and other nutrients is also achieved. Water quality units allow for safe and easy removal of collected material and should be inspected and cleaned in accordance with the Operations and Maintenance (O&M) Plan and per manufacturer's recommendations.

  The use of these units for treatment of stormwater is accepted as a good practice and is in accordance with sound professional standards. Testing was performed by a third party in order to determine the maximum treatment flow rates for both 80% and 50% TSS removal. The testing was also verified by the New Jersey Department of Environmental Protection and the results were verified under the NJCAT program. See Appendix C for supporting information.

- Sediment Forebay: The proposed sediment forebays will improve the water quality of stormwater discharging to the wet basin. The sediment forebays have been sized to hold 0.1 inches per contributing impervious acre resulting in a forebay with a capacities of and 1,650 cubic feet (CF) and 535 (CF), respectively, and will be constructed within the footprint of the existing detention basin.

- Wet Basin: Wet basins allow sediments to settle and remove soluble pollutants in a permanent pool as well as provide storage capacity above the permanent water level to control peak discharges rates. The bottom of the basin will contain vegetation allow for vegetative uptake, reduce soil erosion, and scouring of the basin.

  The existing detention basin will be converted to function as a wet basin. Water quality pre-treatment will be provided by the deep sump hooded catch basins, the proprietary water quality units, and the sediment forebay prior to discharge to the infiltration basin. The bottom of the basin is located at the estimated seasonal high groundwater elevation with will promote the basin having a permanent pool. The basin has been sized to contain all storm events up to and including the 100-year storm event with all runoffs discharging through a new outlet control structure.

- Riprap Outlet Protection/Lever Spreaders: Riprap outlet protection will be placed at all stormwater outfalls in order to reduce flows to non-erosive velocities to prevent erosion and conform to natural topography where appropriate.

- Stormwater Infiltration Chambers: Stormwater recharge for the site is provided through an existing underground stormwater infiltration chamber system that receives clean runoff from the building's roof areas. The chambers are located beneath the paved parking and circulation areas.

All runoff controls should be inspected and cleaned in accordance with the Operations and Maintenance (O&M) Plan included in Section 6.

2.2 CONSTRUCTION SEQUENCE PLAN

The purpose of the Construction Sequence Plan is to develop a working schedule for the implementation of the proposed stormwater improvements. Prior to initiating work, the siltation control barriers will be installed along the limit of work. Once the appropriate permits are obtained, the construction project will commence in the following sequence:

1. Notify all appropriate town departments prior to construction commencement in accordance with all approvals
2. Flag limits of construction necessary to facilitate a pre-construction meeting.
3. Hold a pre-construction meeting. Remember to notify "Call Before You Dig" (1-800-922-4455)
4. Install all necessary siltation barriers and inlet protection as shown on the design drawings.
5. Placed crushed stoned stabilized construction pad and set up construction trailers and fence
6. Demo existing grassed parking islands/ remove existing light poles and install new binder asphalt within demoed parking island areas per Erosion & Sedimentation Control/Demolition Plan

7. Clear and grub/remove trees, as shown on the design drawings.
8. Perform excavation and install new drainage structures and any subsurface utilities
9. Resize and reshape existing grassed water quality swale and detention basin area as shown on the design drawings.
10. Construct training zone areas with associated permanent equipment, poles, and structures.
11. Loam and seed all disturbed areas and install proposed final landscaping.
12. Remove existing erosion control measures upon site stabilization.

All construction water will be collected and treated in accordance with the Erosion and Sediment Control Plan included in Section 5.0.

3.0 STORMWATER ANALYSIS

3.1 METHOD OF ANALYSIS

A hydrologic analysis has been performed for the Site comparing existing conditions and post-development conditions using a software program developed by HydroCAD. This program analyzes site hydrology by the graphic peak discharge method documented in Technical Release No. 20 and Technical Release No. 55 published by the United States Department of Agriculture (USDA) Soil Conservation Service.

The following variables were developed for the contributing watersheds (drainage areas) in order to complete the analysis:

- **Rainfall Depth:** A hydrologic analysis was performed for the 24-hour 2-year, 10-year, 25-year, and 100-year, Type III 24-hr storm events (3.35, 4.95, 6.19, and 8.68 inches respectively) for each drainage area. The rainfall depths for the study area were obtained from available charts published in Atlas-14 for the Site's address..

- **Runoff Curve Number (RCN):** The RCN is a hydrologic characteristic that contributes to the peak rate of runoff and volume from a given storm event. It is dependent upon soil conditions and land use. Generally, higher curve numbers are associated with less pervious soils and, hence, greater amounts of runoff. Per NRCS, the majority of the site consist of Hydrologic Soil Groups (HSG) A and this HSG was used in the HydroCAD analysis. See Appendix B for NRCS custom Soil Resource Report.

- **Time of Concentration:** The time of concentration is defined as the time it takes runoff to travel from the hydraulically most distant part of the watershed to the downstream point of interest. This parameter is dependent on the characteristics of the ground surface and condition of the travel path. Times of concentration were calculated for the various sub catchments using the HydroCAD program, with a minimum time of concentration of six (6) minutes used in accordance with the protocol outlined in Technical Release No. 55.

3.2 DRAINAGE AREAS

In order to perform the analysis, the contributing drainage areas for pre-development, existing, and post-development conditions were delineated. The delineation of the drainage areas was determined by the topography depicted on the Existing Conditions plan. Brief descriptions of the existing conditions and proposed conditions drainage areas are as follows:

- **Existing Conditions:** The Site is divided into two (2) drainage areas and the stormwater runoff was evaluated for one (1) design point, Flow to Wetlands (Design Point 1). Refer to Figure

HYD-EX for the existing conditions drainage areas. For the purpose of the analysis, the times of concentration were calculated to the edge of the wetlands where present.

- **Proposed Conditions:** In the proposed condition, site hydraulic patterns were maintained from existing conditions. The Site is composed of two (2) drainage areas and the stormwater runoff will continue to flow to one (1) design point, Flow to Wetlands (Design Point 1). Refer to Figure HYD-PR for the proposed conditions drainage area.

3.3 RESULTS OF ANALYSIS

A stormwater analysis was performed for the 24-hour 2-year, 10-year, 25-year, and 100-year storm events to determine that there will be no increase in peak stormwater runoff discharge rates off-site once the proposed construction is complete. Detailed calculations are attached in Appendix C. Compliance for existing and post-development conditions was evaluated for the site as a whole. A summary of the peak stormwater runoff rates is provided below.

As shown below in Table 3.1, post-development runoff rates from the Site do not exceed existing runoff rates for the 2, 10, and 25 year storm events. The 100-year event does exceed the pre-development peak runoff rate. However, downstream flooding is not anticipated to be exacerbated as stormwater discharges to a large network of surrounding wetlands. The overall volume increase in the 100-year storm event increases from 2.93 af in the existing conditions to 3.11 af in the post condition. This 0.18 af (7,840 ft^3) of additional stormwater flows to an approx. 50,000 sq. ft wetland system on the site which equates to an overall increase of 0.16' or ≈1.9" in the 100-year storm event. Therefore, this minor increase within this wetland system is negligible and will not exacerbate flooding on the site or neighboring properties. See Supporting calculations provided in Appendix C.

TABLE 3.1
PROJECT STORMWATER RUNOFF RATES

Design Point	**Peak Runoff Rate (cfs)**							
	2-Year		10-Year		25-Year		100-Year	
	Ex.	Prop.	Ex.	Prop.	Ex.	Prop.	Ex.	Prop.
1	0.7	0.5	1.3	1.3	4.2	2.5	16.0	19.0

cfs = cubic feet per second

4.0 STORMWATER CONTROL SYSTEM DESIGN CRITERIA

4.1 MASSDEP STORMWATER MANAGEMENT POLICY

Stormwater discharge from the proposed Project is subject to the Massachusetts DEP Stormwater Management Policy (the Policy). The Policy is designed *"to protect the wetlands and waters of the Commonwealth from adverse impacts of storm water runoff."* To accomplish this goal, the Policy establishes ten (10) performance standards to control stormwater quantity and quality. These standards establish the level of required controls that can be achieved with site planning, structural and non-structural controls, and other best management practices (BMPs). The Stormwater Checklist is provided in Appendix A. Stormwater modeling methodology is discussed in detail in Section 3.0. Results of the stormwater modeling of the existing and proposed conditions are provided as Appendix C.

4.1.1 Stormwater Management Standards

The following section documents compliance with the MassDEP Stormwater Management Standards.

Standard 1
No new stormwater conveyances (e.g., outfalls) may discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.

The project is designed to limit to the extent possible new stormwater conveyances that could discharge untreated stormwater into, or cause erosion to, wetlands or waters of the Commonwealth. The proposed project captures and provides treatment for all new impervious paved areas and will discharge clean runoff to the existing offsite wetlands.

Stormwater runoff from the site will be conveyed through deep sump catch basins, that discharge into water quality units prior to discharging to a sediment forebay and, ultimately, the wet basin. Each outfall will have riprap installed at the outlet to prevent scour.

Standard 2
Stormwater management systems must be designed so that post-development peak discharge rates do not exceed pre-development peak discharge rates. This Standard may be waived for discharges to land subject to coastal storm flowage as defined in 310 CMR 10.04.

The post-development peak discharge rate to design point 1, does not exceed the pre-development rate. Stormwater modeling methodology is discussed in detail in Section 3.0. The model output is provided as Appendix C. A summary of the model results are provided above in Table 3.1.

Standard 3

Loss of annual recharge to groundwater should be eliminated or minimized through the use of infiltration measures.. The annual recharge from the post-development site should approximate the annual recharge from the pre-development or existing site conditions, based on soil types.

An existing underground stormwater infiltration system receives all the stormwater discharge from the building roof resulting in approximately 4,360 CF of infiltration. Additional infiltration is not practicable at this site due to a high groundwater level.

Standard 4

For new development, stormwater management systems must be designed to remove 80% of the average annual load (post-development conditions) of Total Suspended Solids (TSS). It is presumed that this standard is met when:

A. *Suitable nonstructural practices for source control and pollution prevention are implemented;*
B. *Stormwater management best practices (BMPs) are sized to capture the prescribed runoff volume; and*
C. *Stormwater management BMPs are maintained as designed.*

The proposed development proposes to utilize the existing deep sump catch basins that will discharge into new water quality units before ultimately discharging to the reconfigured detention basin. The detention basin will be converted to a wet basin that includes a sediment forebay and will maintain a permanent pool of water,

The estimated TSS removal rate from the proposed BMP pre-treatment train for the existing reconfigured detention basin exceeds the 80% requirement with approximately 92% TSS removal. Refer to Appendix C for the TSS removal spreadsheet. Supporting information is provided in Appendix C.

A comprehensive Operations and Maintenance Plan (O&M) has been developed and is included in Section 6.0 of this report.

Standard 5

Stormwater discharges from areas with higher potential pollutant loads require the use of specific stormwater management BMPs. The use of infiltration practices without pre-treatment is prohibited.

The site is not within areas with higher potential pollutant loads.

Standard 6

Stormwater discharges to critical areas must utilize certain stormwater management BMPs approved for critical areas. Critical areas are Outstanding Resources Waters (ORWs), shellfish beds, bathing beaches, cold water fisheries, and recharge areas for public water supplies.

The Site is not located within critical areas

Standard 7

Redevelopment of previously developed sites must meet the Stormwater Management Standards to the maximum extent practicable. Where it is not practicable to meet all the Standards, new (retrofitted or expanded) stormwater management systems must be designed to improve existing conditions.

This project is considered a new development due to the small increase in impervious coverage from existing conditions.

Standard 8

Erosion and sediment controls must be implemented to prevent impacts during construction, or land disturbance activities.

Erosion and sediment controls are integral to the project improvements. The plan includes Filter Fabric fence reinforced by staked straw bales, which will be installed down-gradient of the proposed work area. If necessary, a temporary stabilized construction exit will be constructed as well. Prior to, and during construction, the Site's Erosion and Sediment Control Plan, included in Section 5.0 of this report will be followed. These measures will be utilized throughout construction to prevent erosion, control sediments, and stabilize exposed soils as discussed in Section 5.0.

Standard 9

All stormwater management systems must have an operations and maintenance plan to ensure that systems function as designed.

A comprehensive Operations and Maintenance Plan (O&M) has been developed and is included in Section 6.0 of this report.

Standard 10

All illicit discharges to the stormwater management system are prohibited.

There are no known illicit discharges at the Site. If found, any illicit discharges will be eliminated, and the project will not be constructed with any illicit connections. An Illicit Discharge Statement is provided in Appendix D.

5.0 CONSTRUCTION SEDIMENTATION AND EROSION CONTROL PLAN

5.1 INTRODUCTION

The greatest potential for sediment generation will occur during the construction. An extensive erosion and sedimentation program is proposed and will be diligently implemented during construction of the project. The erosion control program will minimize erosion and sedimentation that could potentially impact resources areas. Water quality will be maintained by minimizing erosion of exposed soils and siltation. Erosion control barriers will be installed and exposed soil areas re-vegetated as soon as possible after work in an area is completed.

Responsible Party for Plan Compliance:

NSTAR ELECTRIC COMPANY
DBA EVERSOURCE ENERGY
247 STATION DRIVE
WESTWOOD, MA 02090

Contact: Jason St. Martin (Facilities Operation Manager)
Phone: 617-780-9365

5.2 SITE DESCRIPTION

The propsoed site will become an Eversource Training Facility and will serve as a staging area for emergency response vehicles on an as needed basis. The proposed improvements at the site will include approximately 2.3 acres of new gravel area to serve as various electrified/non-electrified training zones with associated permanent equipment, poles, and structures. In addition, the existing cinema on site will be redeveloped into an indoor training area with classrooms and offices.

Soil disturbing activities will include installing perimeter and other sediment controls, finish grading of the site, followed by the resizing of the stormwater detention system, pavement area, utilities, curbing and sidewalks. Upon completion of construction, landscaping will be installed and all disturbed areas will be stabilized.

5.3 SEQUENCE OF MAJOR ACTIVITIES

1. Notify all appropriate town departments prior to construction commencement in accordance with all approvals
2. Flag limits of construction necessary to facilitate a pre-construction meeting.

3. Hold a pre-construction meeting. Remember to notify "Call Before You Dig" (1-800-922-4455)
4. Install all necessary siltation barriers and inlet protection as shown on the design drawings.
5. Placed crushed stoned stabilized construction pad and set up construction trailers and fence
6. Demo existing parking islands/ remove existing light poles and install new binder asphalt within demoed parking island areas per Erosion & Sedimentation Control/Demolition Plan
7. Clear and grub/remove trees, as shown on the design drawings.
8. Perform excavation and install new drainage structures and any subsurface utilities
9. Resize and reshape existing grassed water quality swale and detention basin area as shown on the design drawings.
10. Construct training zone areas with associated permanent equipment, poles, and structures.
11. Loam and seed all disturbed areas and install proposed final landscaping.
12. Remove existing erosion control measures upon site stabilization.

5.4 EROSION AND SEDIMENT CONTROLS

In addition to the perimeter controls, erosion control will be accomplished using temporary measures such as tracking entrance, seeding or mulching, spraying of liquid stabilizers or any combination of these measures. Seeds should be applied at a rate of 2 lbs/ 1000 square feet at a depth of 1/2 inch. Soil netting or covering should be used in extreme conditions.

Only minor stockpiling of soils will be allowed on site. Soil stockpiles will be ringed with hay bales/ silt fencing or covered in extreme conditions.

Maintenance / Inspection Procedures for Erosion and Sediment Controls

- Construction to commence in a phased manner.
- All control measures will be inspected at least once each week and following any storm event of 0.5 inches of precipitation or greater.
- All measures will be maintained in good working order; if repair is necessary, it will be initiated within 24 hours of report.
- Built up sediment will be removed from erosion control when it has reached one-third the height of the fence or bale.
- Silt fence will be inspected for depth of sediment, tears and to see if fabric is securely attached to the fence post, are firmly in the ground.
- Any temporary sediment basin used will be inspected for depth of sediment. Any buildup of sediment will be removed when it reaches 10% of the design capacity or at the end of the project completion.
- Temporary and permanent seeding and planting will be inspected for bare spots, washouts and healthy growth.
- A maintenance and inspection report will be made after each inspection. A copy of the report form to be completed by the inspector and kept on site.

- Construction site supervisor will be responsible for training workers in all inspection and maintenance practices necessary for keeping erosion and sediment controls in good working order.

5.5 OTHER CONTROLS

5.5.1 Waste Disposal

All waste materials will be disposed of offsite in accordance with all applicable local, State, and Federal regulations. No construction waste is to be buried on site. All personnel will be instructed regarding the correct procedure for waste disposal. The individual who manages the day-to-day site operations will be responsible for seeing that these procedures are followed.

5.5.2 Hazardous Waste

All hazardous waste materials will be disposed of in a manner specified by local, State, and Federal regulations and in accordance with any manufacturer's recommendations.

5.5.3 Sanitary Waste

All sanitary waste will be collected in portable units installed on site. The portable units will be cleaned and emptied by a qualified licensed contractor.

5.5.4 Concrete Waste

All concrete washings will be disposed on in a designated area away from wetlands and any property line. When the concrete hardens it be removed from the site.

5.6 POLLUTION AND SPILL PREVENTION

5.6.1 Materials

The following materials are anticipated to be present onsite during construction:

- General construction materials
- Asphalt/concrete
- Paint
- Petroleum-based products
- Cleaning solvents

5.6.2 Material Management Practices

Good Housekeeping Practices

- Store only enough materials needed for current construction activities.
- All materials that are stored outside will be stored in a neat, orderly manner, in the original containers.
- Materials will be kept in their original containers with manufacturer's labels.
- Whenever possible, all materials should be used before disposing the container.
- The site contractor shall be responsible for daily inspections to ensure proper handling and disposal of materials on site.

Product Specific Practices

Petroleum Products:

- Refueling vehicles shall be DOT certified and shall contain SPCC Plans in place along with emergency equipment to contain and clean up spills.
- All on site construction vehicles shall be inspected for leaks and receive regular preventive maintenance to reduce the chance of leakage.
- Petroleum-based products will be stored in tightly sealed containers, which are properly marked.

Paints:

- All containers will be tightly sealed and stored when not required for use.
- All procedures will be followed to minimize spills and to keep products in the original containers.

Concrete Trucks:

- The site contractor is responsible for designating a safe area, away from abutting property and resource areas, for excess concrete disposal.

Product Specific Practices

In addition to the good housekeeping and material management practices discussed in the previous sections of this plan, the following practices will be followed for spill prevention and cleanup during construction:

- Manufacturer recommended methods for spill clean up will be clearly posted and site personnel will be made aware of the procedures and the location of the information and cleanup supplies.
- All spills will be cleaned up immediately after discovery.
- In any case or threat of explosion or life threatening condition, all personnel shall evacuate the area to safety and then contact the local fire department for assistance.

- The spill area will be ventilated and personnel will wear appropriate protective clothing to prevent injury from contact with a hazardous substance.
- The site contractor shall be responsible for spill prevention and cleanup and will designate at least three personnel who will receive spill prevention and cleanup training. The names of the assigned three personnel will be posted in the material storage area in the field office on site.

5.7 RECORD KEEPING

The following records will be maintained on the Site:

1. Dates when major grading activities occur,
2. Dates when construction activities temporarily or permanently cease on a portion of the Site,
3. Dates when stabilization measures are initiated, and
4. In addition, the following records will also be kept:
    - The Order of Conditions; and any additional permit conditions/approvals,
    - All inspection reports, and
    - Any spill reports.

6.0 OPERATIONS AND MAINTENANCE (O&M) PLAN

6.1 GENERAL

Stormwater management systems with multiple components, such as the one proposed for the project, assures the cleanest possible discharges of stormwater to the environment. However, these systems must be routinely maintained to keep them in good working order. Additionally, this plan identifies potential sources of pollution that may affect the quality of stormwater discharges and describes the implementation of Long-Term Pollution Prevention practices to reduce potential pollutants in stormwater discharge. The party identified below will be responsible for the operation and maintenance of the stormwater management system and Site. Schedules and procedures for inspection and maintenance of the existing and proposed stormwater management system components are provided in the following sections.

Responsible Party for Plan Compliance:

NSTAR ELECTRIC COMPANY
DBA EVERSOURCE ENERGY
247 STATION DRIVE
WESTWOOD, MA 02090

Contact: Jason St. Martin (Facilities Operation Manager)
Phone: 617-780-9365

Emergency Contact Information:
Civil & Environmental Consultants, Inc.
(774) 501-2176

Upon a transfer of ownership, if any, the future owner shall assume the responsibilities for compliance with this O&M Plan.

6.2 ROUTINE INSPECTIONS

Inspections of the stormwater management system as a whole, and of the individual components of the system, will be carried out on a routine basis in accordance with the schedule identified in Section 6.3. Components to be inspected include the catch basins and subsurface infiltration chambers. Each will be inspected for sediment buildup, presence of oil, color, and structural damage. The results of each inspection will be entered into an inspection log. Refer to Table 6.1 for the inspection log forms.

6.3 MAINTENANCE PLAN

The Responsible Party will incorporate a routine maintenance program to assure proper operation of the stormwater management system. The program will include the following maintenance activities:

Catch Basin Structures

- See the attached Manufacturer's instructions on operation and maintenance requirements and methodology.
- Inspect and clean four times per year or as required by manufacturer.
- Remove sediment and other trapped pollutants at whenever the depth of the deposits is greater than two feet.

Stormceptor Water Quality Units

- See the attached Manufacturer's instructions on operation and maintenance requirements and methodology.
- Inspect and clean twice per year or as required by manufacturer.
- Remove sediment and other trapped pollutants at the frequency or level specified by the manufacturer.

Sediment Forebay

- Inspect monthly for accumulated sediment, trash, and debris and remove it.
- Clean four times per year and when sediment depth is greater than 3 feet.

Wet Basin

- Inspect at least once per year to ensure it's operating as designed.
- Mow the embankments at least twice per year.
- Remove sediment from the basin as necessary, and at least once every 10 years.
- Emergency overflow pipes will be examined at least once each year and verified that no blockage has occurred.

Rip-Rap Outlet

- Inspect after the first several rainfall events and after any major storm events within the first year. After the first year, inspect regularly on an annual basis.
- Remove any sediment, trash, debris, leaves and grass clippings. Remove any tree seedlings before they become firmly established.
- Note and repair any erosion or low spots.

Infiltration Chamber

- See the attached Manufacturer's instructions on operation and maintenance requirements and methodology.
- Inspect and clean twice per year or as required by manufacturer.
- Remove sediment and other trapped pollutants at the frequency or level specified by the manufacturer.

6.4 LONG TERM POLLUTION PREVENTION MAINTENANCE

The Responsible Party will incorporate a routine maintenance program to ensure the continued effectiveness of the structural water quality controls. Maintenance will be performed based on the results of inspections in accordance with the schedules identified below. The program will include the following maintenance activities:

Maintenance of Pavement Systems

Regular maintenance of pavement surfaces will prevent pollutants such as oil and grease, trash, and sediments from entering the stormwater management system. The following practices should be performed:

- Sweep or vacuum asphalt pavement areas annually with a commercial cleaning unit and dispose of removed material.
- Routinely pick up and remove litter from the parking areas, islands, and perimeter landscaping.

Maintenance of Vegetated Areas

Proper maintenance of vegetated areas can prevent the pollution of stormwater runoff by controlling the source of pollutants such as suspended sediments, excess nutrients, and chemicals from landscape care products. Practices that should be followed under the regular maintenance of the vegetated landscape include:

- Inspect planted areas on a semi-annual basis and remove any litter.
- Maintain planted areas adjacent to pavement to prevent soil washout.
- Immediately clean any soil deposited on pavement.
- Re-seed bare areas: install appropriate erosion control measures when native soil is exposed, or erosion channels are forming.
- Plant alternative mixture of grass species in the event of unsuccessful establishment.
- Grass vegetation should not be cut to a height less than four inches.
- Pesticide/Herbicide Usage – No pesticides are to be used unless a single spot treatment is required for a specific control application.
- Fertilizer usage should be avoided. If deemed necessary, slow-release fertilizer should be used. Fertilizer may be used to begin the establishment of vegetation in bare or damaged areas but should not be applied on a regular basis unless necessary.

Management of Snow and Ice

Should significant snow fall events occur, which result in stockpiled snow impacting the operation of the Project Site, through the temporary loss of parking or limiting access in any way, the property manager may choose to have snow removed from the site. All snow removal operations will be done in accordance with Massachusetts DEP guidelines BRPG01-01, effective date March 8, 2001.

Salt and Deicing Chemicals

The amount of salt and deicing chemicals to be used on the site shall be reduced to the minimum amount needed to provide safe pedestrian and vehicle travel. The following practices should be followed to control the amount of salt and deicing materials that come into contact with stormwater runoff:

- Devices used for spreading salt and deicing chemicals should be capable of varying the rate of application based on the site-specific conditions.
- Sand and salt should be stockpiled under covered storage facilities that prevent precipitation and adjacent runoff from coming in contact with the deicing materials.

6.5 EMPLOYEE TRAINING

Training of personnel is essential to achieving proper operation and maintenance of the stormwater management system. Therefore, those Facility personnel who are responsible for operation and maintenance will be trained on the following subjects:

- Environmental laws and regulations relating to stormwater,
- The components and goals of the current Erosion and Sediment Control Plan,
- Site specific permit conditions and requirements,
- General Facility spill response procedures,
- General good housekeeping procedures, and
- General material management procedures.

Refresher training sessions will be held once a year following the completion of the Site Compliance Evaluation.

6.6 RECORDKEEPING

Records of inspections and maintenance shall be up to date and available for review and inspection, if requested

FIGURES

FIGURES

REFERENCE

1. USGS TOPOGRAPHIC MAP QUADRANGLES q265838 and q261838, DATED 2011.
2. USGS MAPS ARE BASED ON GIS DATA PROVIDED BY THE BUREAU OF GEOGRAPHIC INFORMATION (MASS GIS), COMMONWEALTH OF MASSACHUSETTS EXECUTIVE OFFICE OF TECHNOLOGY AND SECURITY SERVICES.

SCALE IN FEET

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WAREHAM, MASSACHUSETTS

SITE LOCUS

DRAWN BY:	CJV	CHECKED BY:	BEP	APPROVED BY:	BEP	FIGURE NO.:	1
DATE:	JULY 2023	DWG SCALE:	1"=2000'	PROJECT NO:	323-322		

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REFERENCE

1. AERIAL PHOTOGRAPHY BY EARTHSTAR GEOGRAPHICS SIO, PROVIDED BY AUTODESK, ACCESSED 05/10/2023.

SCALE IN FEET

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WAREHAM, MASSACHUSETTS

AERIAL EXHIBIT

DRAWN BY:	CJV	CHECKED BY:	BEP	APPROVED BY:	BEP	FIGURE NO.:	2
DATE:	JULY 2023	DWG SCALE:	1"=500'	PROJECT NO:	323-322		

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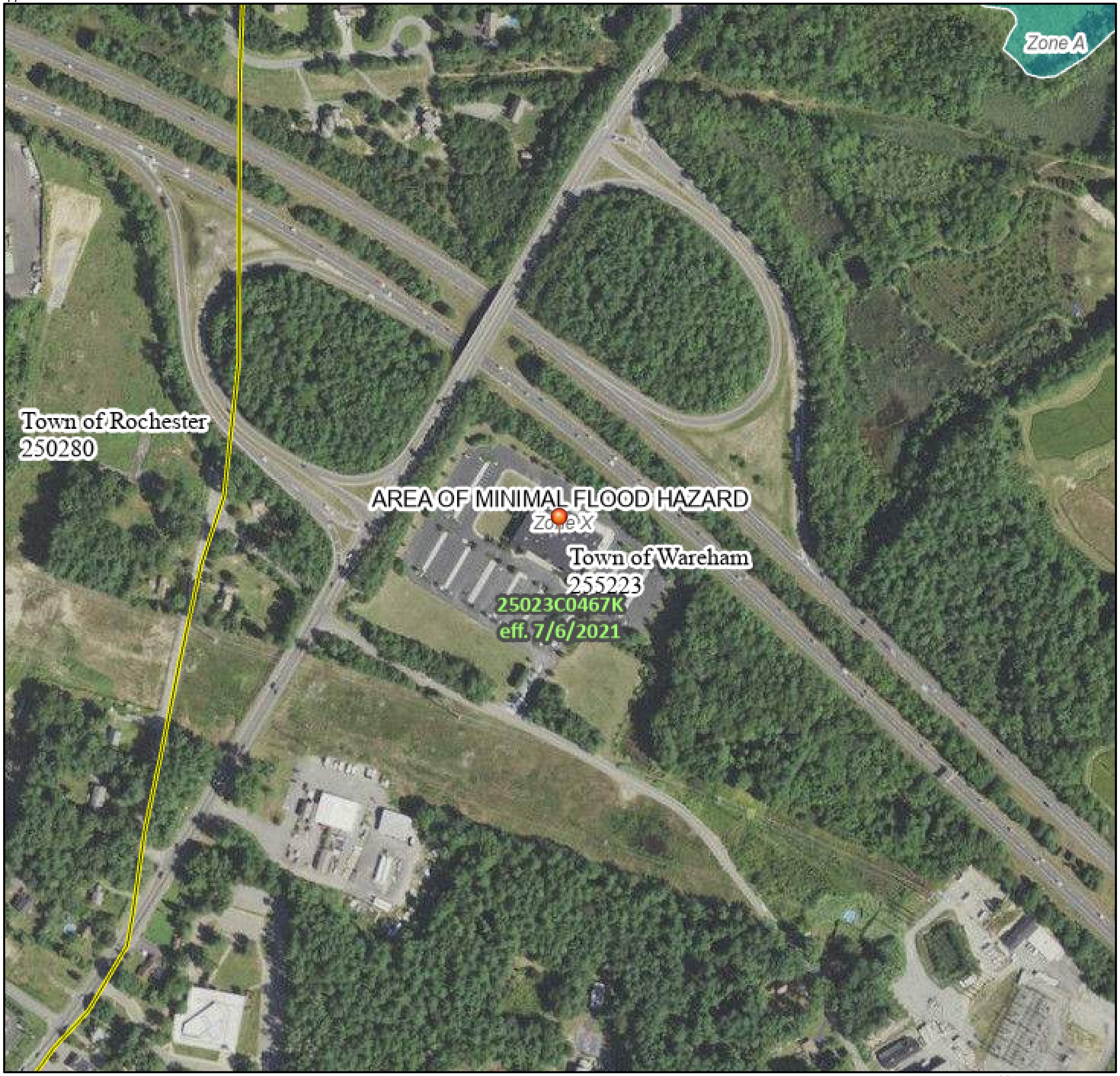
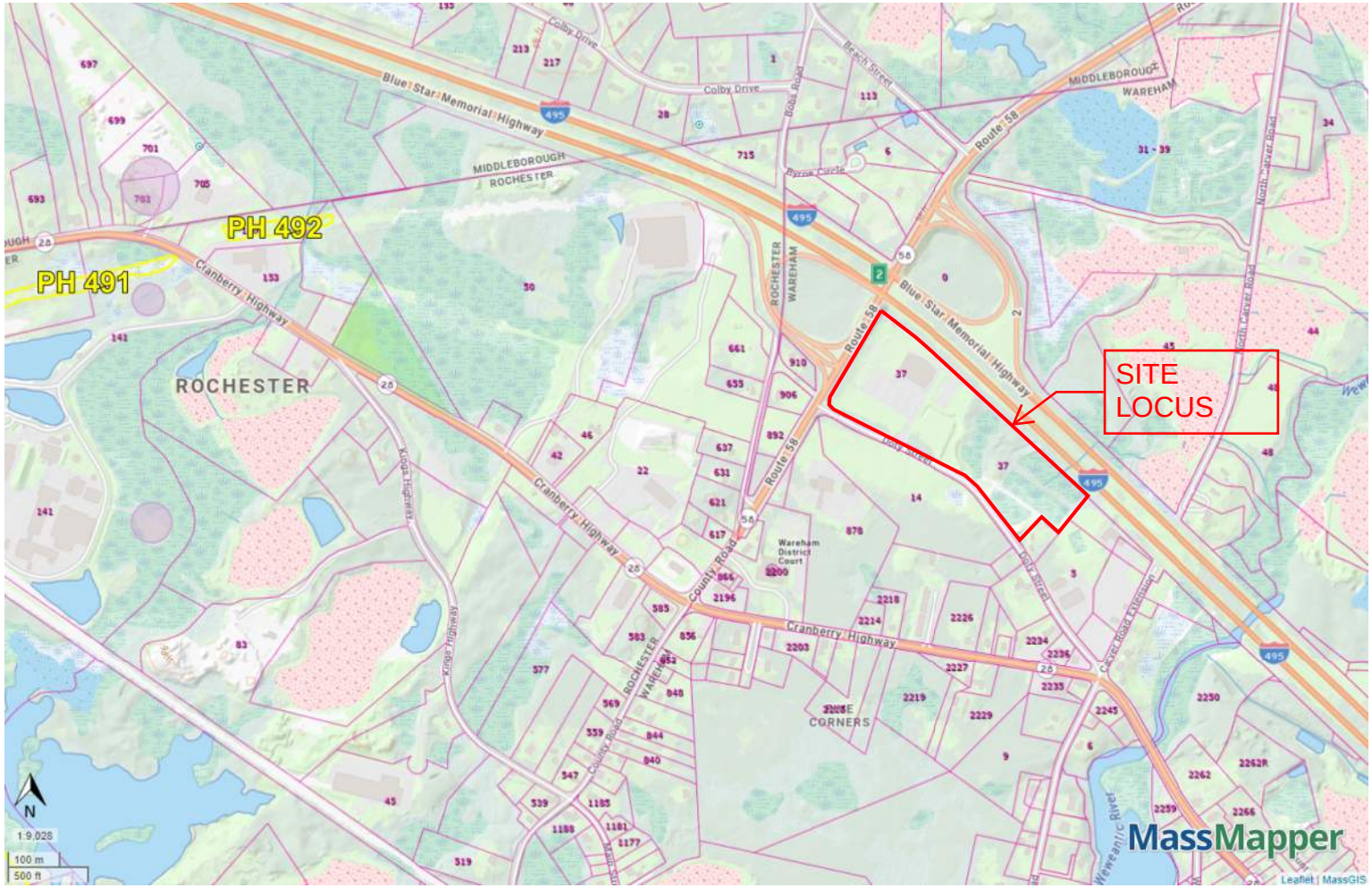


FIGURE 3

Critical Areas Map



Areas of Critical Environmental Concern ACECs

NHESP Priority Habitats of Rare Species

NHESP Estimated Habitats of Rare Wildlife

Zone C

Zone B

Zone A

Zone Is

Zone IIs

NHESP Certified Vernal Pools

Potential Vernal Pools

Property Tax Parcels

FIGURE 4

NORTH

LEGEND

- A — DESIGN POINT
- A1 — SUBCATCHMENT AREA
- P1 — POND/DETENTION AREA
- SUBCATCHMENT BOUNDARY
- TIME OF CONCENTRATION PATH
- VEGETATED AREA
- WOODED AREA
- PAVED AREA
- ROOF AREA

SCALE IN FEET

0 100 200

REFERENCE

1. EXISTING CONDITIONS INFORMATION WAS COMPILED FROM AN ON THE GROUND SURVEY PERFORMED BY CIVIL & ENVIRONMENTAL CONSULTANTS, CONDUCTED IN JULY AND AUGUST 2021.

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DRAWN BY:	AMB	CHECKED BY:	DRAFT	APPROVED BY:	DRAFT	FIGURE NO.:
DATE:	MARCH 2023	DWG SCALE:	1"=100'	PROJECT NO:	323-322	HYD-PRE

EVERSOURCE ENERGY
37 DOTY STREET
WAREHAM. MASSACHUSETTS

DRAINAGE AREA MAP
EXISTING CONDITIONS

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NORTH

LEGEND

- A — DESIGN POINT
- A1 — SUBCATCHMENT AREA
- P1 — POND/DETENTION AREA
- SUBCATCHMENT BOUNDARY
- TIME OF CONCENTRATION PA
- VEGETATED AREA
- WOODED AREA
- PAVED AREA
- GRAVEL AREA
- ROOF AREA
- WATER AREA

REFERENCE

1. EXISTING CONDITIONS INFORMATION WAS COMPILED FROM AN ON THE GROUND SURVEY PERFORMED BY CIVIL & ENVIRONMENTAL CONSULTANTS, CONDUCTED IN JULY AND AUGUST 2022.

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DATE:	MARCH 2023	DWG SCALE:	1"=100'	PROJECT NO:	323-322

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DRAINAGE AREA MAP
PROPOSED CONDITIONS

FIGURE NO.: **HYD-POST**

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APPENDIX A

DEP STORMWATER CHECKLIST

Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.

A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.[1] This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8[2]
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

---

[1] The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

[2] For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.

Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature

3/8/24

Signature and Date

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

☐ New development

☐ Redevelopment

☒ Mix of New Development and Redevelopment

Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

☒ No disturbance to any Wetland Resource Areas

☐ Site Design Practices (e.g. clustered development, reduced frontage setbacks)

☐ Reduced Impervious Area (Redevelopment Only)

☐ Minimizing disturbance to existing trees and shrubs

☐ LID Site Design Credit Requested:

- ☐ Credit 1
- ☐ Credit 2
- ☐ Credit 3

☐ Use of "country drainage" versus curb and gutter conveyance and pipe

☐ Bioretention Cells (includes Rain Gardens)

☐ Constructed Stormwater Wetlands (includes Gravel Wetlands designs)

☐ Treebox Filter

☐ Water Quality Swale

☐ Grass Channel

☐ Green Roof

☐ Other (describe): _____

Standard 1: No New Untreated Discharges

☒ No new untreated discharges

☒ Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth

☐ Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.

Checklist for Stormwater Report

Checklist (continued)

Standard 2: Peak Rate Attenuation

☐ Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.

☒ Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.

☒ Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

☒ Soil Analysis provided.

☐ Required Recharge Volume calculation provided.

☐ Required Recharge volume reduced through use of the LID site Design Credits.

☐ Sizing the infiltration, BMPs is based on the following method: Check the method used.

☐ Static ☐ Simple Dynamic ☐ Dynamic Field[1]

☐ Runoff from all impervious areas at the site discharging to the infiltration BMP.

☐ Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.

☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume.

☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:

☐ Site is comprised solely of C and D soils and/or bedrock at the land surface

☐ M.G.L. c. 21E sites pursuant to 310 CMR 40.0000

☐ Solid Waste Landfill pursuant to 310 CMR 19.000

☒ Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.

☐ Calculations showing that the infiltration BMPs will drain in 72 hours are provided.

☐ Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

[1] 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.

Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

☐ The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.

☐ Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.

☒ A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.

☐ Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:

- ☐ is within the Zone II or Interim Wellhead Protection Area
- ☐ is near or to other critical areas
- ☐ is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
- ☐ involves runoff from land uses with higher potential pollutant loads.

☐ The Required Water Quality Volume is reduced through use of the LID site Design Credits.

☒ Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.

Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

☒ The BMP is sized (and calculations provided) based on:

- ☒ The ½" or 1" Water Quality Volume or
- ☒ The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.

☒ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.

☐ A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

☐ The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.

☐ The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted ***prior to*** the discharge of stormwater to the post-construction stormwater BMPs.

☒ The NPDES Multi-Sector General Permit does ***not*** cover the land use.

☐ LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.

☐ All exposure has been eliminated.

☐ All exposure has ***not*** been eliminated and all BMPs selected are on MassDEP LUHPPL list.

☐ The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

☐ The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.

☐ Critical areas and BMPs are identified in the Stormwater Report.

Checklist for Stormwater Report

Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

☒ The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:

- ☐ Limited Project
- ☐ Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
- ☐ Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
- ☐ Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
- ☐ Bike Path and/or Foot Path
- ☐ Redevelopment Project
- ☒ Redevelopment portion of mix of new and redevelopment.

☒ Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.

☒ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.

☒ A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.

Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has ***not*** been included in the Stormwater Report but will be submitted ***before*** land disturbance begins.

☐ The project is ***not*** covered by a NPDES Construction General Permit.

☐ The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.

☒ The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

☒ The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:

- ☒ Name of the stormwater management system owners;
- ☒ Party responsible for operation and maintenance;
- ☒ Schedule for implementation of routine and non-routine maintenance tasks;
- ☐ Plan showing the location of all stormwater BMPs maintenance access areas;
- ☐ Description and delineation of public safety features;
- ☒ Estimated operation and maintenance budget; and
- ☒ Operation and Maintenance Log Form.

☐ The responsible party is ***not*** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:

- ☐ A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
- ☐ A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

☒ The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;

☒ An Illicit Discharge Compliance Statement is attached;

☐ NO Illicit Discharge Compliance Statement is attached but will be submitted ***prior to*** the discharge of any stormwater to post-construction BMPs.

APPENDIX B

GEOTECHNICAL INFORMATION

NRCS Soil Resource Report

NRCS Soil Resource Report

United States Department of Agriculture

NRCS

Natural Resources Conservation Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Plymouth County, Massachusetts

June 1, 2023

Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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Contents

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report
Soil Map

Map Scale: 1:2,860 if printed on A landscape (11" x 8.5") sheet.

N

Meters: 0 40 80 160 240

Feet: 0 100 200 400 600

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 19N WGS84

MAP LEGEND

Area of Interest (AOI)
- Area of Interest (AOI)

Soils
- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

Special Point Features
- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

Water Features
- Streams and Canals

Transportation
- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background
- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Plymouth County, Massachusetts
Survey Area Data: Version 15, Sep 9, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 10, 2022—Jun 30, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
253A	Hinckley loamy sand, 0 to 3 percent slopes	4.1	26.2%
253B	Hinckley loamy sand, 3 to 8 percent slopes	2.3	14.8%
256A	Deerfield loamy fine sand, 0 to 3 percent slopes	1.9	12.2%
259A	Carver loamy coarse sand, 0 to 3 percent slopes	3.2	20.6%
259B	Carver loamy coarse sand, 3 to 8 percent slopes	1.9	12.5%
321A	Birchwood sand, 0 to 3 percent slopes, very stony	1.9	11.9%
430C	Barnstable loamy sand, 8 to 15 percent slopes	0.0	0.0%
656B	Udorthents - Urban land complex, 0 to 8 percent slopes	0.3	1.7%
Totals for Area of Interest		**15.6**	**100.0%**

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas

are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Plymouth County, Massachusetts

253A—Hinckley loamy sand, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 2svm7
Elevation: 0 to 1,420 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: Farmland of statewide importance

Map Unit Composition

Hinckley and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hinckley

Setting

Landform: Outwash terraces, outwash plains, kame terraces, outwash deltas
Landform position (three-dimensional): Tread
Down-slope shape: Concave, convex, linear
Across-slope shape: Convex, linear, concave
Parent material: Sandy and gravelly glaciofluvial deposits derived from gneiss and/or granite and/or schist

Typical profile

Oe - 0 to 1 inches: moderately decomposed plant material
A - 1 to 8 inches: loamy sand
Bw1 - 8 to 11 inches: gravelly loamy sand
Bw2 - 11 to 16 inches: gravelly loamy sand
BC - 16 to 19 inches: very gravelly loamy sand
C - 19 to 65 inches: very gravelly sand

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 99.90 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 3.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3s
Hydrologic Soil Group: A
Ecological site: F144AY022MA - Dry Outwash
Hydric soil rating: No

Minor Components

Windsor

Percent of map unit: 5 percent
Landform: Outwash deltas, kame terraces, outwash terraces
Landform position (three-dimensional): Tread
Down-slope shape: Concave, convex, linear
Across-slope shape: Convex, linear, concave
Hydric soil rating: No

Sudbury

Percent of map unit: 5 percent
Landform: Outwash deltas, outwash terraces, kame terraces
Landform position (three-dimensional): Tread
Down-slope shape: Concave, convex, linear
Across-slope shape: Convex, linear, concave
Hydric soil rating: No

Merrimac

Percent of map unit: 5 percent
Landform: Outwash deltas, outwash terraces, kame terraces
Landform position (three-dimensional): Tread
Down-slope shape: Concave, convex, linear
Across-slope shape: Convex, linear, concave
Hydric soil rating: No

253B—Hinckley loamy sand, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2svm8
Elevation: 0 to 1,430 feet
Mean annual precipitation: 36 to 53 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 250 days
Farmland classification: Farmland of statewide importance

Map Unit Composition

Hinckley and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hinckley

Setting

Landform: Outwash deltas, outwash terraces, kames, kame terraces, moraines, eskers, outwash plains
Landform position (two-dimensional): Summit, shoulder, backslope, footslope
Landform position (three-dimensional): Nose slope, side slope, base slope, crest, riser, tread
Down-slope shape: Concave, convex, linear

Across-slope shape: Convex, linear, concave
Parent material: Sandy and gravelly glaciofluvial deposits derived from gneiss and/or granite and/or schist

Typical profile

Oe - 0 to 1 inches: moderately decomposed plant material
A - 1 to 8 inches: loamy sand
Bw1 - 8 to 11 inches: gravelly loamy sand
Bw2 - 11 to 16 inches: gravelly loamy sand
BC - 16 to 19 inches: very gravelly loamy sand
C - 19 to 65 inches: very gravelly sand

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 99.90 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Very low (about 3.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3s
Hydrologic Soil Group: A
Ecological site: F144AY022MA - Dry Outwash
Hydric soil rating: No

Minor Components

Windsor

Percent of map unit: 8 percent
Landform: Outwash deltas, outwash terraces, moraines, eskers, kames, outwash plains, kame terraces
Landform position (two-dimensional): Summit, shoulder, backslope, footslope
Landform position (three-dimensional): Nose slope, side slope, base slope, crest, riser, tread
Down-slope shape: Concave, convex, linear
Across-slope shape: Convex, linear, concave
Hydric soil rating: No

Sudbury

Percent of map unit: 5 percent
Landform: Outwash deltas, outwash terraces, moraines, outwash plains, kame terraces
Landform position (two-dimensional): Backslope, footslope
Landform position (three-dimensional): Head slope, side slope, base slope, tread
Down-slope shape: Concave, linear
Across-slope shape: Concave, linear
Hydric soil rating: No

Agawam

Percent of map unit: 2 percent

Landform: Outwash deltas, outwash terraces, moraines, eskers, kames, outwash plains, kame terraces
Landform position (two-dimensional): Summit, shoulder, backslope, footslope
Landform position (three-dimensional): Nose slope, side slope, base slope, crest, riser, tread
Down-slope shape: Concave, convex, linear
Across-slope shape: Convex, linear, concave
Hydric soil rating: No

256A—Deerfield loamy fine sand, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 2xfg8
Elevation: 0 to 1,100 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 145 to 240 days
Farmland classification: Farmland of statewide importance

Map Unit Composition

Deerfield and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Deerfield

Setting

Landform: Outwash terraces, outwash deltas, outwash plains, kame terraces
Landform position (three-dimensional): Tread
Down-slope shape: Concave, convex, linear
Across-slope shape: Convex, linear, concave
Parent material: Sandy outwash derived from granite, gneiss, and/or quartzite

Typical profile

Ap - 0 to 9 inches: loamy fine sand
Bw - 9 to 25 inches: loamy fine sand
BC - 25 to 33 inches: fine sand
Cg - 33 to 60 inches: sand

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Moderately well drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 99.90 in/hr)
Depth to water table: About 15 to 37 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)

Sodium adsorption ratio, maximum: 11.0
Available water supply, 0 to 60 inches: Moderate (about 6.5 inches)

Interpretive groups
Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 2w
Hydrologic Soil Group: A
Ecological site: F144AY027MA - Moist Sandy Outwash
Hydric soil rating: No

Minor Components

Windsor
Percent of map unit: 7 percent
Landform: Outwash terraces, kame terraces, outwash deltas, outwash plains
Landform position (three-dimensional): Tread
Down-slope shape: Concave, convex, linear
Across-slope shape: Convex, linear, concave
Hydric soil rating: No

Wareham
Percent of map unit: 5 percent
Landform: Drainageways, depressions
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

Sudbury
Percent of map unit: 2 percent
Landform: Outwash plains, kame terraces, outwash deltas, outwash terraces
Landform position (three-dimensional): Tread
Down-slope shape: Concave, convex, linear
Across-slope shape: Convex, linear, concave
Hydric soil rating: No

Ninigret
Percent of map unit: 1 percent
Landform: Outwash terraces, kame terraces, outwash plains
Landform position (three-dimensional): Tread
Down-slope shape: Linear, convex
Across-slope shape: Concave, convex
Hydric soil rating: No

259A—Carver loamy coarse sand, 0 to 3 percent slopes

Map Unit Setting
National map unit symbol: 2y07s
Elevation: 0 to 990 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days

Farmland classification: Not prime farmland

Map Unit Composition

Carver, loamy coarse sand, and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Carver, Loamy Coarse Sand

Setting

Landform: Moraines, outwash plains
Landform position (two-dimensional): Summit, shoulder
Landform position (three-dimensional): Side slope, crest, tread
Down-slope shape: Convex, linear
Across-slope shape: Linear
Parent material: Sandy glaciofluvial deposits

Typical profile

Oi - 0 to 2 inches: slightly decomposed plant material
Oe - 2 to 3 inches: moderately decomposed plant material
A - 3 to 7 inches: loamy coarse sand
E - 7 to 10 inches: coarse sand
Bw1 - 10 to 15 inches: coarse sand
Bw2 - 15 to 28 inches: coarse sand
BC - 28 to 32 inches: coarse sand
C - 32 to 67 inches: coarse sand

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 14.17 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 4.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3s
Hydrologic Soil Group: A
Ecological site: F149BY005MA - Dry Outwash
Hydric soil rating: No

Minor Components

Deerfield

Percent of map unit: 10 percent
Landform: Outwash terraces, outwash deltas, outwash plains, kame terraces
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Concave
Hydric soil rating: No

Hinckley

Percent of map unit: 5 percent
Landform: Outwash deltas, outwash terraces, moraines, eskers, kames, outwash plains, kame terraces
Landform position (two-dimensional): Summit, shoulder, backslope, footslope, toeslope
Landform position (three-dimensional): Head slope, nose slope, side slope, crest, riser, tread
Down-slope shape: Convex
Across-slope shape: Convex
Hydric soil rating: No

Merrimac

Percent of map unit: 3 percent
Landform: Outwash terraces, kame terraces, outwash deltas
Landform position (three-dimensional): Riser, tread
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

Mashpee

Percent of map unit: 2 percent
Landform: Depressions, drainageways, terraces
Landform position (three-dimensional): Tread
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

259B—Carver loamy coarse sand, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2y07t
Elevation: 0 to 240 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: Not prime farmland

Map Unit Composition

Carver, loamy coarse sand, and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Carver, Loamy Coarse Sand

Setting

Landform: Moraines, outwash plains
Landform position (two-dimensional): Summit, shoulder, backslope, footslope, toeslope

Landform position (three-dimensional): Head slope, nose slope, side slope, crest, tread
Down-slope shape: Convex, linear
Across-slope shape: Linear
Parent material: Sandy glaciofluvial deposits

Typical profile

Oi - 0 to 2 inches: slightly decomposed plant material
Oe - 2 to 3 inches: moderately decomposed plant material
A - 3 to 7 inches: loamy coarse sand
E - 7 to 10 inches: coarse sand
Bw1 - 10 to 15 inches: coarse sand
Bw2 - 15 to 28 inches: coarse sand
BC - 28 to 32 inches: coarse sand
C - 32 to 67 inches: coarse sand

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 14.17 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 4.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3s
Hydrologic Soil Group: A
Ecological site: F149BY005MA - Dry Outwash
Hydric soil rating: No

Minor Components

Deerfield

Percent of map unit: 10 percent
Landform: Outwash terraces, outwash plains, kame terraces, outwash deltas
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Concave
Hydric soil rating: No

Hinckley

Percent of map unit: 5 percent
Landform: Moraines, eskers, kames, outwash deltas, outwash terraces, outwash plains, kame terraces
Landform position (two-dimensional): Summit, shoulder, backslope, footslope, toeslope
Landform position (three-dimensional): Head slope, nose slope, side slope, crest, riser, tread
Down-slope shape: Convex
Across-slope shape: Convex
Hydric soil rating: No

Merrimac

Percent of map unit: 3 percent
Landform: Kame terraces, outwash deltas, outwash terraces
Landform position (three-dimensional): Riser, tread
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

Mashpee

Percent of map unit: 2 percent
Landform: Depressions, drainageways, terraces
Landform position (three-dimensional): Tread
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

321A—Birchwood sand, 0 to 3 percent slopes, very stony

Map Unit Setting

National map unit symbol: 9y46
Elevation: 0 to 400 feet
Mean annual precipitation: 41 to 54 inches
Mean annual air temperature: 43 to 54 degrees F
Frost-free period: 145 to 240 days
Farmland classification: Farmland of statewide importance

Map Unit Composition

Birchwood, very stony, and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Birchwood, Very Stony

Setting

Landform: Till plains, ground moraines, drumlins
Landform position (two-dimensional): Summit, footslope
Landform position (three-dimensional): Interfluve
Down-slope shape: Concave
Across-slope shape: Concave
Parent material: Sandy eolian deposits and/or sandy glaciofluvial deposits over coarse-loamy lodgment till

Typical profile

Oi - 0 to 1 inches: slightly decomposed plant material
Oe - 1 to 3 inches: moderately decomposed plant material
Oa - 3 to 4 inches: highly decomposed plant material
E - 4 to 5 inches: sand
Ap - 5 to 8 inches: loamy sand
Bs - 8 to 13 inches: loamy sand
Bw1 - 13 to 19 inches: loamy sand

Bw2 - 19 to 29 inches: loamy sand
BC - 29 to 40 inches: sand
Cd1 - 40 to 55 inches: gravelly sandy loam
Cd2 - 55 to 75 inches: gravelly sandy loam

Properties and qualities

Slope: 0 to 3 percent
Surface area covered with cobbles, stones or boulders: 1.0 percent
Depth to restrictive feature: 35 to 59 inches to densic material
Drainage class: Moderately well drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately high (0.00 to 0.20 in/hr)
Depth to water table: About 12 to 29 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 3.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 5s
Hydrologic Soil Group: B/D
Ecological site: F144AY037MA - Moist Dense Till Uplands
Hydric soil rating: No

Minor Components

Poquonock, very stony

Percent of map unit: 6 percent
Landform: Till plains, ground moraines, drumlins
Landform position (two-dimensional): Summit, shoulder
Landform position (three-dimensional): Interfluve
Down-slope shape: Convex
Across-slope shape: Convex
Hydric soil rating: No

Mattapoisett, extremely stony

Percent of map unit: 6 percent
Landform: Drainageways, depressions
Landform position (two-dimensional): Footslope, toeslope
Landform position (three-dimensional): Base slope
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

Scituate, very stony

Percent of map unit: 5 percent
Landform: Drumlins, ridges
Landform position (two-dimensional): Summit, footslope
Landform position (three-dimensional): Interfluve
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: No

Newfields, extremely stony

Percent of map unit: 3 percent
Landform: Till plains, hills, moraines

Landform position (two-dimensional): Summit, footslope
Landform position (three-dimensional): Interfluve
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: No

430C—Barnstable loamy sand, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 9y3r
Elevation: 10 to 400 feet
Mean annual precipitation: 41 to 54 inches
Mean annual air temperature: 43 to 54 degrees F
Frost-free period: 145 to 240 days
Farmland classification: Farmland of statewide importance

Map Unit Composition

Barnstable and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Barnstable

Setting

Landform: Moraines
Landform position (two-dimensional): Shoulder, backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Convex
Parent material: Coarse-loamy supraglacial meltout till over sandy and gravelly glaciofluvial deposits

Typical profile

Oi - 0 to 2 inches: slightly decomposed plant material
Oe - 2 to 3 inches: moderately decomposed plant material
Oa - 3 to 4 inches: highly decomposed plant material
E - 4 to 6 inches: loamy sand
Bs - 6 to 7 inches: gravelly sandy loam
Bw1 - 7 to 13 inches: stony sandy loam
Bw2 - 13 to 27 inches: very stony coarse sandy loam
2C1 - 27 to 40 inches: very gravelly coarse sand
2C2 - 40 to 64 inches: very gravelly coarse sand

Properties and qualities

Slope: 8 to 15 percent
Depth to restrictive feature: 23 to 27 inches to strongly contrasting textural stratification
Drainage class: Well drained
Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 1.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Very low (about 2.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: B
Ecological site: F149BY011MA - Well Drained Till Uplands
Hydric soil rating: No

Minor Components

Plymouth

Percent of map unit: 7 percent
Landform: Outwash plains, moraines
Landform position (two-dimensional): Shoulder, backslope
Landform position (three-dimensional): Side slope, riser
Down-slope shape: Linear
Across-slope shape: Convex
Hydric soil rating: No

Merrimac

Percent of map unit: 5 percent
Landform: Kames, terraces, outwash plains
Landform position (two-dimensional): Shoulder, backslope
Landform position (three-dimensional): Riser
Down-slope shape: Linear
Across-slope shape: Convex
Hydric soil rating: No

Canton

Percent of map unit: 5 percent
Landform: Till plains, ridges, hills
Landform position (two-dimensional): Shoulder, backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Convex
Hydric soil rating: No

Newfields

Percent of map unit: 3 percent
Landform: Moraines, hills, till plains
Landform position (two-dimensional): Shoulder, footslope
Landform position (three-dimensional): Interfluve
Down-slope shape: Linear
Across-slope shape: Concave
Hydric soil rating: No

656B—Udorthents - Urban land complex, 0 to 8 percent slopes

Map Unit Setting

National map unit symbol: bd08
Elevation: 0 to 390 feet
Mean annual precipitation: 41 to 54 inches
Mean annual air temperature: 43 to 54 degrees F
Frost-free period: 145 to 240 days
Farmland classification: Not prime farmland

Map Unit Composition

Udorthents, loamy, and similar soils: 45 percent
Urban land: 40 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Udorthents, Loamy

Setting

Landform position (two-dimensional): Summit, shoulder
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Coarse-loamy human transported material

Typical profile

^A - 0 to 5 inches: loam
^C1 - 5 to 21 inches: gravelly loam
^C2 - 21 to 80 inches: gravelly sandy loam

Properties and qualities

Slope: 0 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to very high (0.01 to 14.17 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Moderate (about 7.9 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 2s
Hydrologic Soil Group: B
Ecological site: F149BY100NY - Urban Site Complex
Hydric soil rating: No

Minor Components

Udipsamments, wet substratum

Percent of map unit: 5 percent
Landform: Dikes
Landform position (two-dimensional): Footslope
Landform position (three-dimensional): Tread
Down-slope shape: Linear, convex
Across-slope shape: Linear
Hydric soil rating: No

Udipsamments

Percent of map unit: 5 percent
Landform: Dikes
Landform position (two-dimensional): Summit
Landform position (three-dimensional): Tread
Down-slope shape: Linear, convex
Across-slope shape: Linear
Hydric soil rating: No

Udorthents, wet substratum

Percent of map unit: 5 percent
Landform position (two-dimensional): Footslope
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

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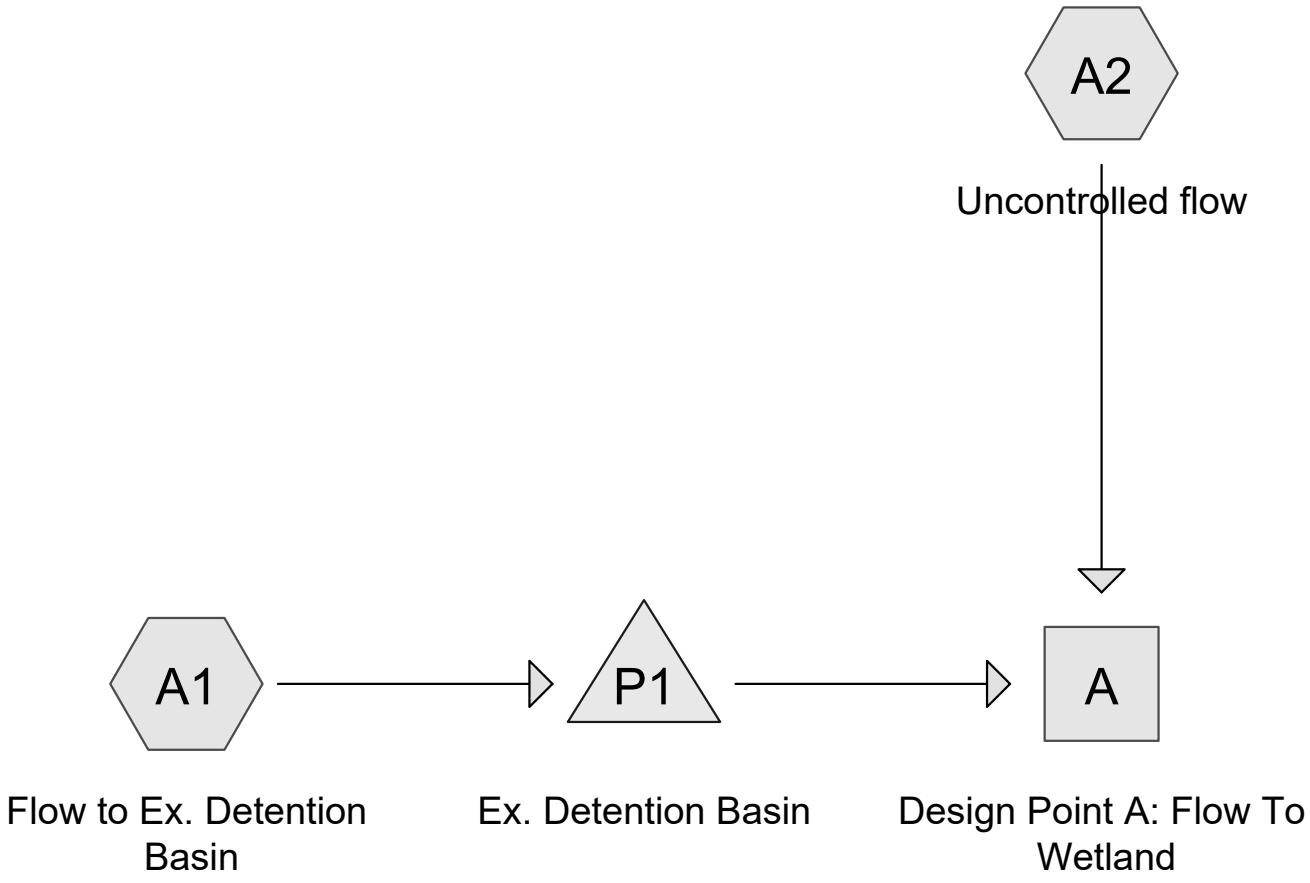
APPENDIX C

SUPPORTING CALCULATIONS

HydroCAD Drainage Analysis
TSS Calculations
Phosphorus Removal Calculations
Water Quality Volume Calculations
Drawdown Calculations
Rip-Rap Outlet Sizing Calculations
Sediment Forebay Sizing Calculations

HydroCAD Drainage Analysis



Subcat Reach Pond Link

Routing Diagram for Pre Drainage Calcs
Prepared by CEC Inc, Printed 3/8/2024

Project Notes

Rainfall events imported from "Atlas-14-Rain.txt" for 447 MA Plymouth

Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-Year	Type III 24-hr		Default	24.00	1	3.35	2
2	10-Year	Type III 24-hr		Default	24.00	1	4.95	2
3	25-Year	Type III 24-hr		Default	24.00	1	6.19	2
4	100-Year	Type III 24-hr		Default	24.00	1	8.68	2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
3.9	39	>75% Grass cover, Good, HSG A (A1)
0.4	39	Landscaped islands, Good, HSG A (A1)
3.4	98	Paved parking, HSG A (A1)
0.5	98	Water Surface, HSG A (A1)
2.8	45	Woods, Poor, HSG A (A1, A2)
0.2	98	paved sidewalks, HSG A (A1)
11.2	**62**	**TOTAL AREA**

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
11.2	HSG A	A1, A2
0.0	HSG B	
0.0	HSG C	
0.0	HSG D	
0.0	Other	
11.2		**TOTAL AREA**

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
3.9	0.0	0.0	0.0	0.0	3.9	>75% Grass cover, Good	A1
0.4	0.0	0.0	0.0	0.0	0.4	Landscaped islands, Good	A1
3.4	0.0	0.0	0.0	0.0	3.4	Paved parking	A1
0.5	0.0	0.0	0.0	0.0	0.5	Water Surface	A1
2.8	0.0	0.0	0.0	0.0	2.8	Woods, Poor	A1, A2
0.2	0.0	0.0	0.0	0.0	0.2	paved sidewalks	A1
11.2	**0.0**	**0.0**	**0.0**	**0.0**	**11.2**	**TOTAL AREA**	

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A1: Flow to Ex. Detention Runoff Area=450,410 sf 39.85% Impervious Runoff Depth>0.56"
Flow Length=690' Tc=9.6 min CN=64 Runoff=5.2 cfs 0.482 af

Subcatchment A2: Uncontrolled flow Runoff Area=38,860 sf 0.00% Impervious Runoff Depth>0.04"
Flow Length=103' Tc=8.7 min CN=45 Runoff=0.0 cfs 0.003 af

Reach A: Design Point A: Flow To Wetland Inflow=0.7 cfs 0.367 af
Outflow=0.7 cfs 0.367 af

Pond P1: Ex. Detention Basin Peak Elev=30.74' Storage=13,300 cf Inflow=5.2 cfs 0.482 af
Outflow=0.7 cfs 0.364 af

Total Runoff Area = 11.2 ac Runoff Volume = 0.485 af Average Runoff Depth = 0.52"
63.32% Pervious = 7.1 ac 36.68% Impervious = 4.1 ac

Summary for Subcatchment A1: Flow to Ex. Detention Basin

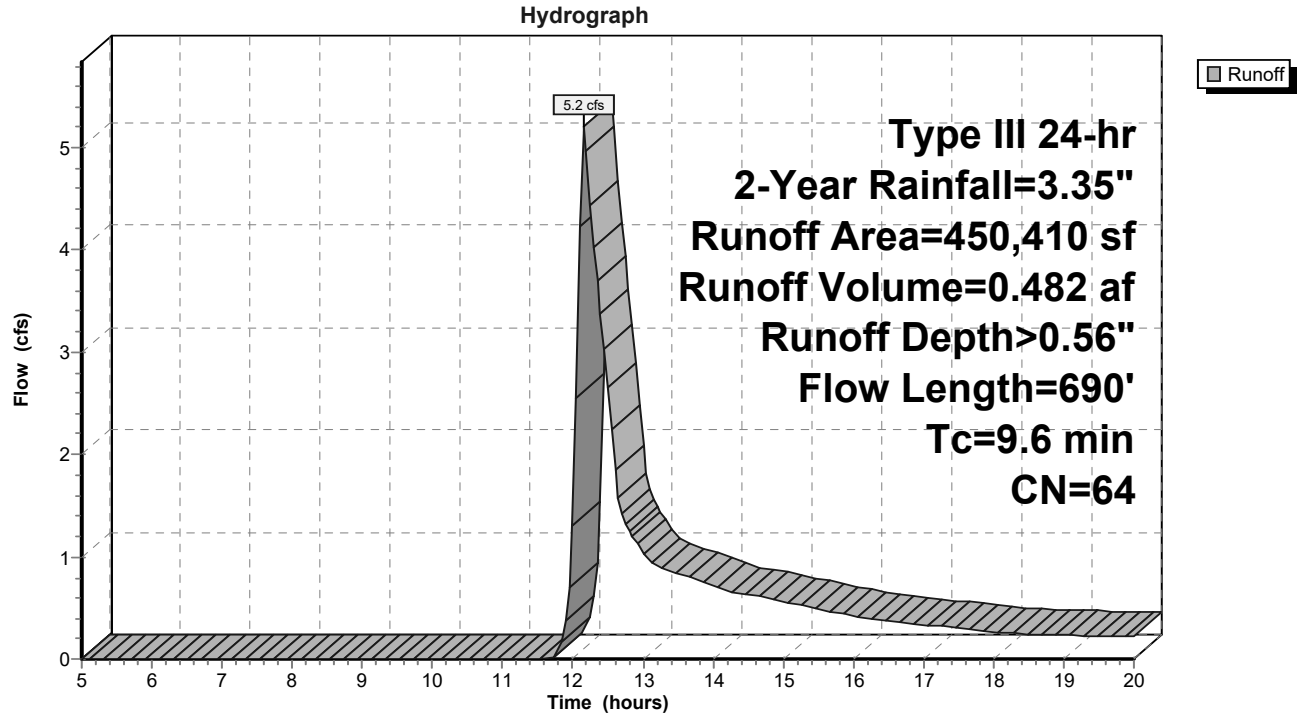
Runoff = 5.2 cfs @ 12.17 hrs, Volume= 0.482 af, Depth> 0.56"
Routed to Pond P1 : Ex. Detention Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.35"

	Area (sf)	CN	Description
*	18,731	39	Landscaped islands, Good, HSG A
*	7,841	98	paved sidewalks, HSG A
	82,000	45	Woods, Poor, HSG A
	149,846	98	Paved parking, HSG A
	170,212	39	>75% Grass cover, Good, HSG A
	21,780	98	Water Surface, HSG A
	450,410	64	Weighted Average
	270,943		60.15% Pervious Area
	179,467		39.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3	50	0.0400	0.20		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.20"
5.3	640	0.0100	2.03		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
9.6	690	Total			

Subcatchment A1: Flow to Ex. Detention Basin



Summary for Subcatchment A2: Uncontrolled flow

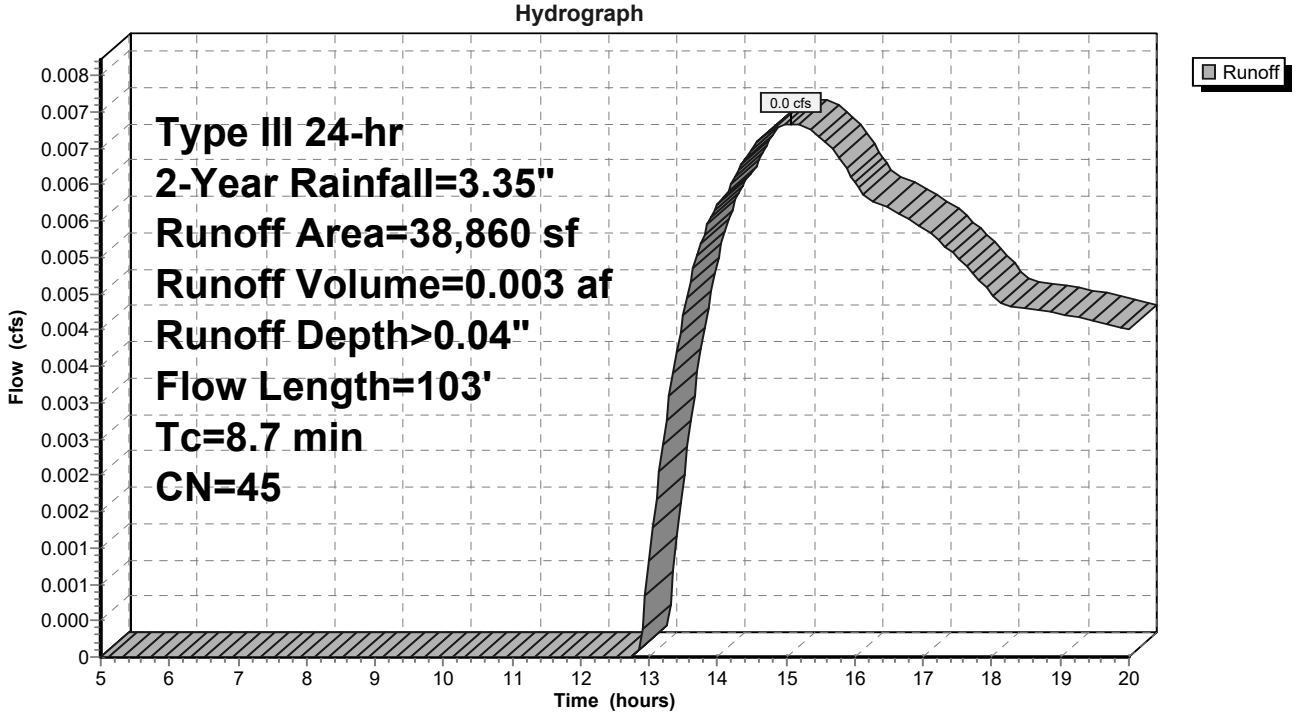
Runoff = 0.0 cfs @ 15.06 hrs, Volume= 0.003 af, Depth> 0.04"
Routed to Reach A : Design Point A: Flow To Wetland

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.35"

Area (sf)	CN	Description
38,860	45	Woods, Poor, HSG A
38,860		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.5	50	0.0500	0.10		**Sheet Flow, A-B** Woods: Light underbrush n= 0.400 P2= 3.20"
0.2	53	0.0900	4.83		**Shallow Concentrated Flow, B-C** Unpaved Kv= 16.1 fps
8.7	103	Total			

Subcatchment A2: Uncontrolled flow



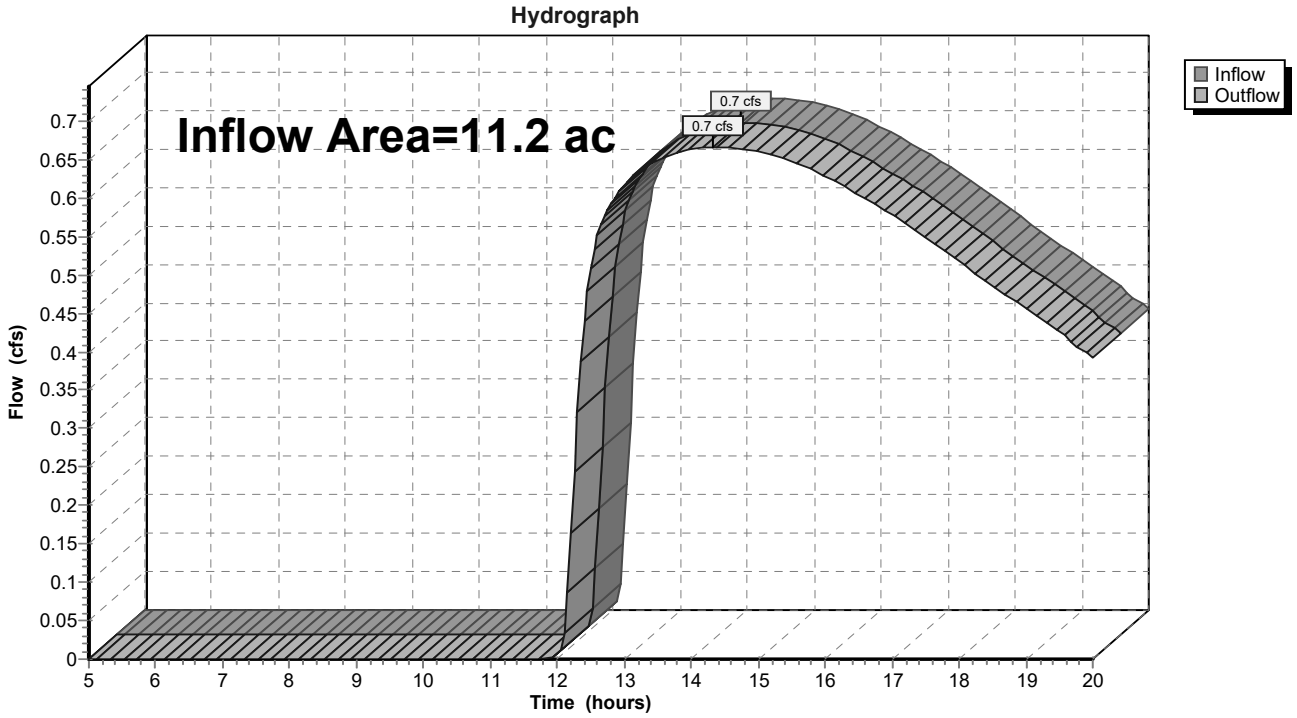
Summary for Reach A: Design Point A: Flow To Wetland

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 11.2 ac, 36.68% Impervious, Inflow Depth > 0.39" for 2-Year event
Inflow = 0.7 cfs @ 14.34 hrs, Volume= 0.367 af
Outflow = 0.7 cfs @ 14.34 hrs, Volume= 0.367 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach A: Design Point A: Flow To Wetland



Summary for Pond P1: Ex. Detention Basin

Inflow Area = 10.3 ac, 39.85% Impervious, Inflow Depth > 0.56" for 2-Year event
Inflow = 5.2 cfs @ 12.17 hrs, Volume= 0.482 af
Outflow = 0.7 cfs @ 14.27 hrs, Volume= 0.364 af, Atten= 87%, Lag= 126.2 min
Primary = 0.7 cfs @ 14.27 hrs, Volume= 0.364 af
Routed to Reach A : Design Point A: Flow To Wetland

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Starting Elev= 30.00' Surf.Area= 10,000 sf Storage= 4,261 cf
Peak Elev= 30.74' @ 14.27 hrs Surf.Area= 13,476 sf Storage= 13,300 cf (9,039 cf above start)

Plug-Flow detention time= 261.1 min calculated for 0.265 af (55% of inflow)
Center-of-Mass det. time= 108.3 min (953.4 - 845.2)

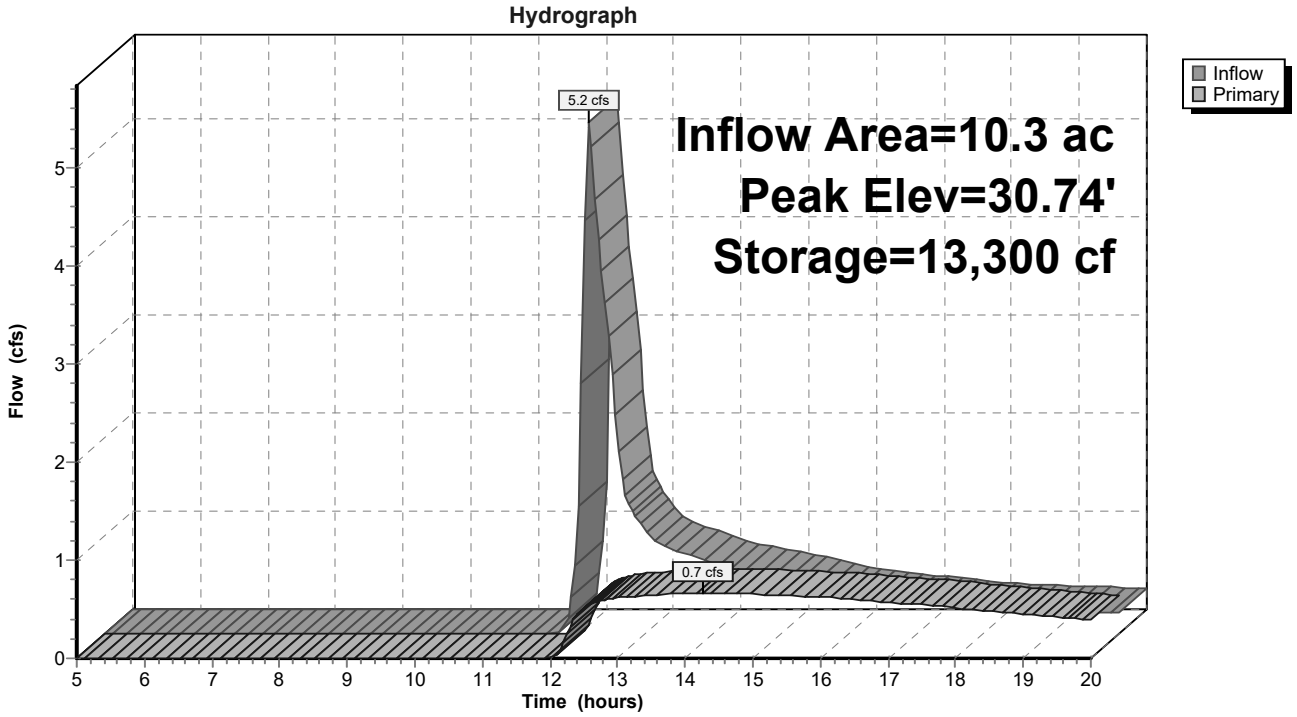
Volume	Invert	Avail.Storage	Storage Description
#1	29.00'	101,165 cf	**Custom Stage Data (Irregular)** Listed below

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
29.00	515	100.0	0	0	515
30.00	10,000	845.0	4,261	4,261	56,542
31.00	14,725	1,210.0	12,287	16,548	116,239
32.00	18,875	1,245.0	16,757	33,305	123,186
33.00	22,820	1,255.0	20,816	54,121	125,536
34.00	26,550	1,275.0	24,661	78,783	129,757
34.80	29,430	1,300.0	22,382	101,165	134,982

Device	Routing	Invert	Outlet Devices
#1	Device 3	32.00'	**2.5' long x 2.80' rise Sharp-Crested Rectangular Weir** 2 End Contraction(s) 3.0' Crest Height
#2	Device 3	30.00'	**6.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#3	Primary	28.91'	**24.0" Round Culvert** L= 40.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 28.91' / 28.51' S= 0.0100 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=0.7 cfs @ 14.27 hrs HW=30.74' (Free Discharge)
└**3=Culvert** (Passes 0.7 cfs of 10.9 cfs potential flow)
  ├**1=Sharp-Crested Rectangular Weir** (Controls 0.0 cfs)
  └**2=Orifice/Grate** (Orifice Controls 0.7 cfs @ 3.36 fps)

Pond P1: Ex. Detention Basin



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A1: Flow to Ex. Detention Runoff Area=450,410 sf 39.85% Impervious Runoff Depth>1.41"
Flow Length=690' Tc=9.6 min CN=64 Runoff=15.4 cfs 1.213 af

Subcatchment A2: Uncontrolled flow Runoff Area=38,860 sf 0.00% Impervious Runoff Depth>0.36"
Flow Length=103' Tc=8.7 min CN=45 Runoff=0.2 cfs 0.027 af

Reach A: Design Point A: Flow To Wetland Inflow=1.3 cfs 0.791 af
Outflow=1.3 cfs 0.791 af

Pond P1: Ex. Detention Basin Peak Elev=31.96' Storage=32,644 cf Inflow=15.4 cfs 1.213 af
Outflow=1.2 cfs 0.764 af

Total Runoff Area = 11.2 ac Runoff Volume = 1.240 af Average Runoff Depth = 1.32"
63.32% Pervious = 7.1 ac 36.68% Impervious = 4.1 ac

Summary for Subcatchment A1: Flow to Ex. Detention Basin

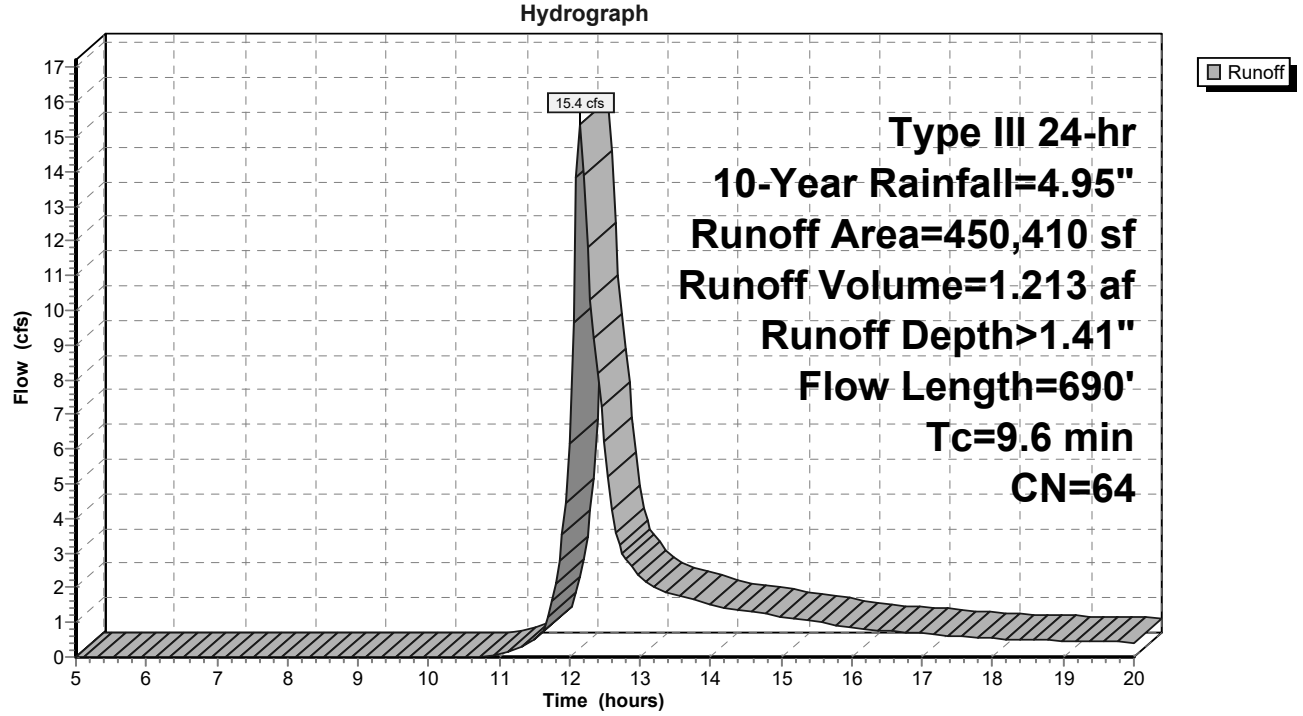
Runoff = 15.4 cfs @ 12.15 hrs, Volume= 1.213 af, Depth> 1.41"
Routed to Pond P1 : Ex. Detention Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=4.95"

	Area (sf)	CN	Description
*	18,731	39	Landscaped islands, Good, HSG A
*	7,841	98	paved sidewalks, HSG A
	82,000	45	Woods, Poor, HSG A
	149,846	98	Paved parking, HSG A
	170,212	39	>75% Grass cover, Good, HSG A
	21,780	98	Water Surface, HSG A
	450,410	64	Weighted Average
	270,943		60.15% Pervious Area
	179,467		39.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3	50	0.0400	0.20		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.20"
5.3	640	0.0100	2.03		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
9.6	690	Total			

Subcatchment A1: Flow to Ex. Detention Basin



Summary for Subcatchment A2: Uncontrolled flow

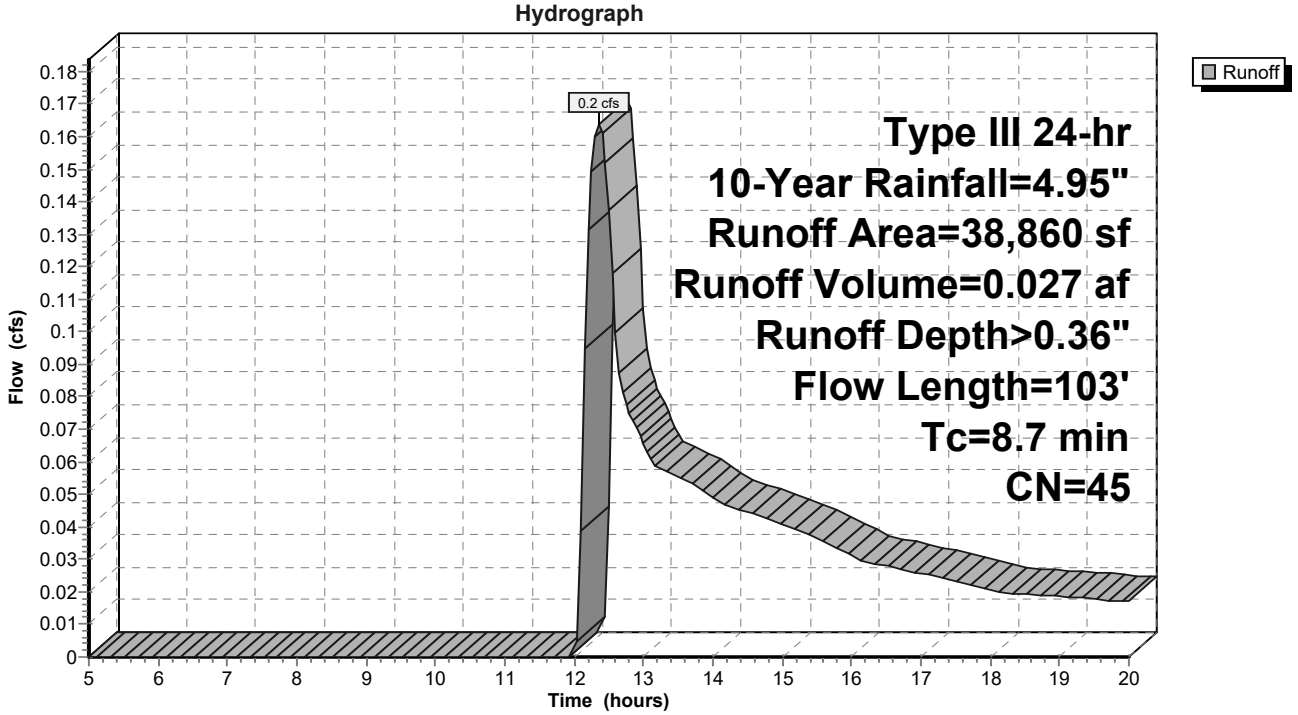
Runoff = 0.2 cfs @ 12.35 hrs, Volume= 0.027 af, Depth> 0.36"
Routed to Reach A : Design Point A: Flow To Wetland

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=4.95"

Area (sf)	CN	Description
38,860	45	Woods, Poor, HSG A
38,860		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.5	50	0.0500	0.10		**Sheet Flow, A-B** Woods: Light underbrush n= 0.400 P2= 3.20"
0.2	53	0.0900	4.83		**Shallow Concentrated Flow, B-C** Unpaved Kv= 16.1 fps
8.7	103	Total			

Subcatchment A2: Uncontrolled flow



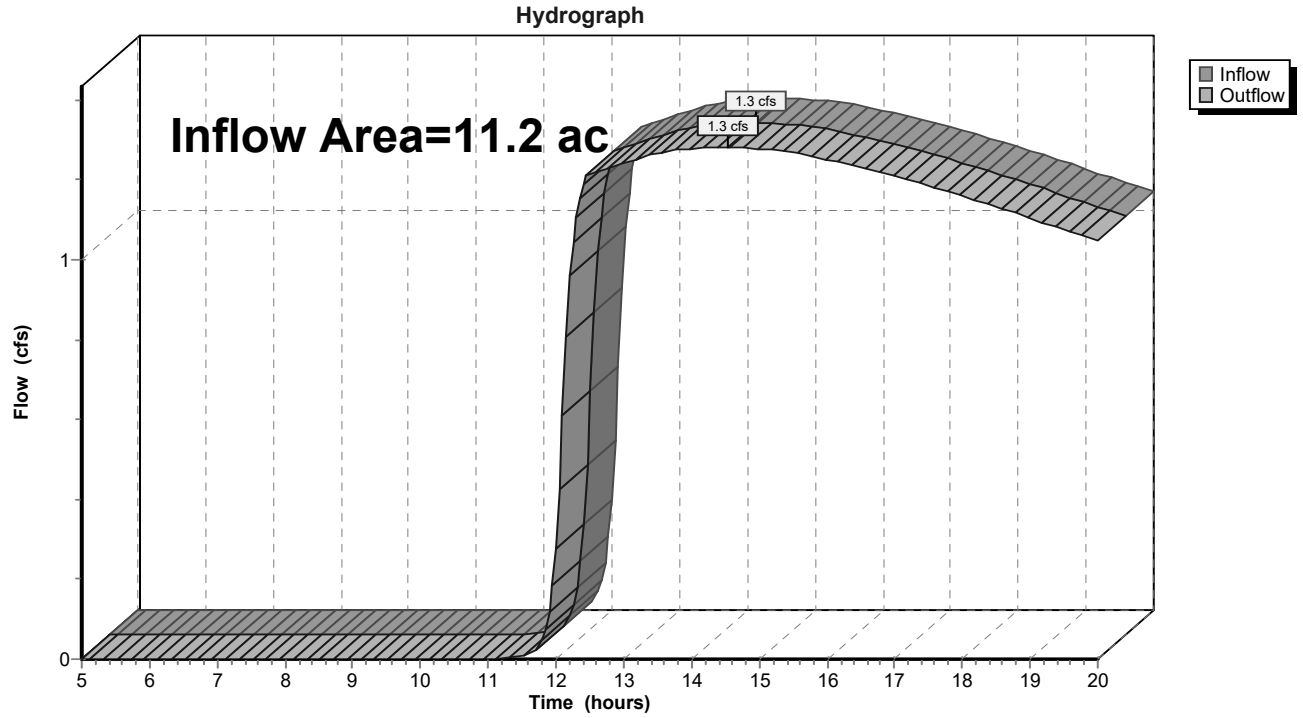
Summary for Reach A: Design Point A: Flow To Wetland

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 11.2 ac, 36.68% Impervious, Inflow Depth > 0.84" for 10-Year event
Inflow = 1.3 cfs @ 14.53 hrs, Volume= 0.791 af
Outflow = 1.3 cfs @ 14.53 hrs, Volume= 0.791 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach A: Design Point A: Flow To Wetland



Summary for Pond P1: Ex. Detention Basin

Inflow Area = 10.3 ac, 39.85% Impervious, Inflow Depth > 1.41" for 10-Year event
Inflow = 15.4 cfs @ 12.15 hrs, Volume= 1.213 af
Outflow = 1.2 cfs @ 14.81 hrs, Volume= 0.764 af, Atten= 92%, Lag= 159.3 min
Primary = 1.2 cfs @ 14.81 hrs, Volume= 0.764 af
Routed to Reach A : Design Point A: Flow To Wetland

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Starting Elev= 30.00' Surf.Area= 10,000 sf Storage= 4,261 cf
Peak Elev= 31.96' @ 14.81 hrs Surf.Area= 18,711 sf Storage= 32,644 cf (28,383 cf above start)

Plug-Flow detention time= 253.9 min calculated for 0.664 af (55% of inflow)
Center-of-Mass det. time= 136.8 min (959.5 - 822.7)

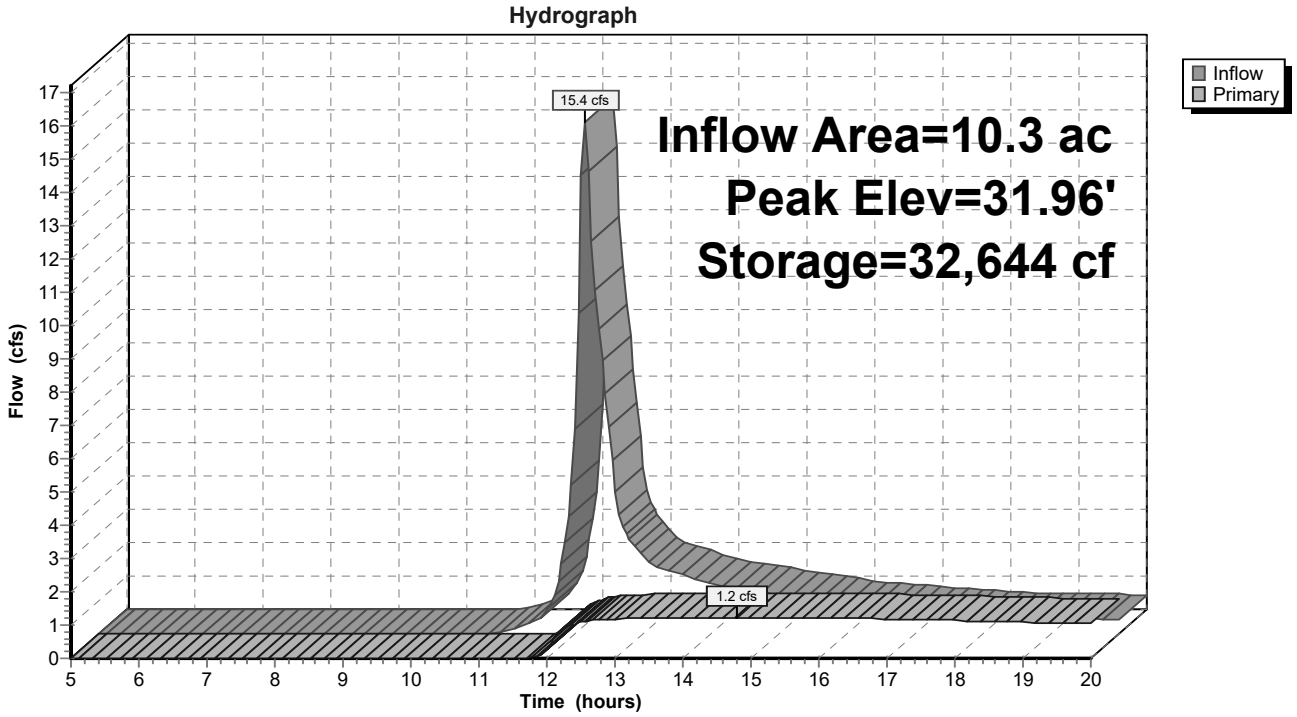
Volume	Invert	Avail.Storage	Storage Description
#1	29.00'	101,165 cf	**Custom Stage Data (Irregular)** Listed below

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
29.00	515	100.0	0	0	515
30.00	10,000	845.0	4,261	4,261	56,542
31.00	14,725	1,210.0	12,287	16,548	116,239
32.00	18,875	1,245.0	16,757	33,305	123,186
33.00	22,820	1,255.0	20,816	54,121	125,536
34.00	26,550	1,275.0	24,661	78,783	129,757
34.80	29,430	1,300.0	22,382	101,165	134,982

Device	Routing	Invert	Outlet Devices
#1	Device 3	32.00'	**2.5' long x 2.80' rise Sharp-Crested Rectangular Weir** 2 End Contraction(s) 3.0' Crest Height
#2	Device 3	30.00'	**6.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#3	Primary	28.91'	**24.0" Round Culvert** L= 40.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 28.91' / 28.51' S= 0.0100 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.2 cfs @ 14.81 hrs HW=31.96' (Free Discharge)
└**3=Culvert** (Passes 1.2 cfs of 17.1 cfs potential flow)
  ├**1=Sharp-Crested Rectangular Weir** (Controls 0.0 cfs)
  └**2=Orifice/Grate** (Orifice Controls 1.2 cfs @ 6.30 fps)

Pond P1: Ex. Detention Basin



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A1: Flow to Ex. Detention Runoff Area=450,410 sf 39.85% Impervious Runoff Depth>2.20"
Flow Length=690' Tc=9.6 min CN=64 Runoff=24.8 cfs 1.898 af

Subcatchment A2: Uncontrolled flow Runoff Area=38,860 sf 0.00% Impervious Runoff Depth>0.77"
Flow Length=103' Tc=8.7 min CN=45 Runoff=0.5 cfs 0.057 af

Reach A: Design Point A: Flow To Wetland Inflow=4.2 cfs 1.385 af
Outflow=4.2 cfs 1.385 af

Pond P1: Ex. Detention Basin Peak Elev=32.48' Storage=43,211 cf Inflow=24.8 cfs 1.898 af
Outflow=4.0 cfs 1.328 af

Total Runoff Area = 11.2 ac Runoff Volume = 1.955 af Average Runoff Depth = 2.09"
63.32% Pervious = 7.1 ac 36.68% Impervious = 4.1 ac

Summary for Subcatchment A1: Flow to Ex. Detention Basin

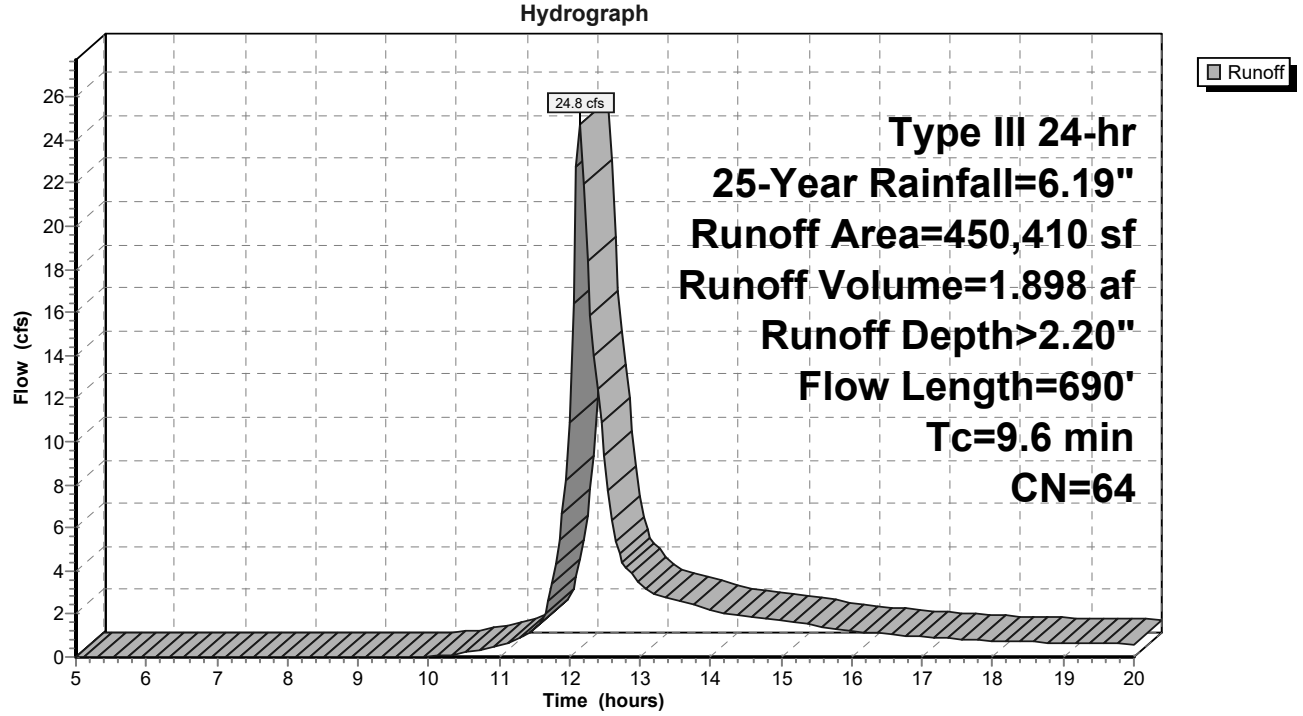
Runoff = 24.8 cfs @ 12.15 hrs, Volume= 1.898 af, Depth> 2.20"
Routed to Pond P1 : Ex. Detention Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.19"

	Area (sf)	CN	Description
*	18,731	39	Landscaped islands, Good, HSG A
*	7,841	98	paved sidewalks, HSG A
	82,000	45	Woods, Poor, HSG A
	149,846	98	Paved parking, HSG A
	170,212	39	>75% Grass cover, Good, HSG A
	21,780	98	Water Surface, HSG A
	450,410	64	Weighted Average
	270,943		60.15% Pervious Area
	179,467		39.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3	50	0.0400	0.20		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.20"
5.3	640	0.0100	2.03		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
9.6	690	Total			

Subcatchment A1: Flow to Ex. Detention Basin



Summary for Subcatchment A2: Uncontrolled flow

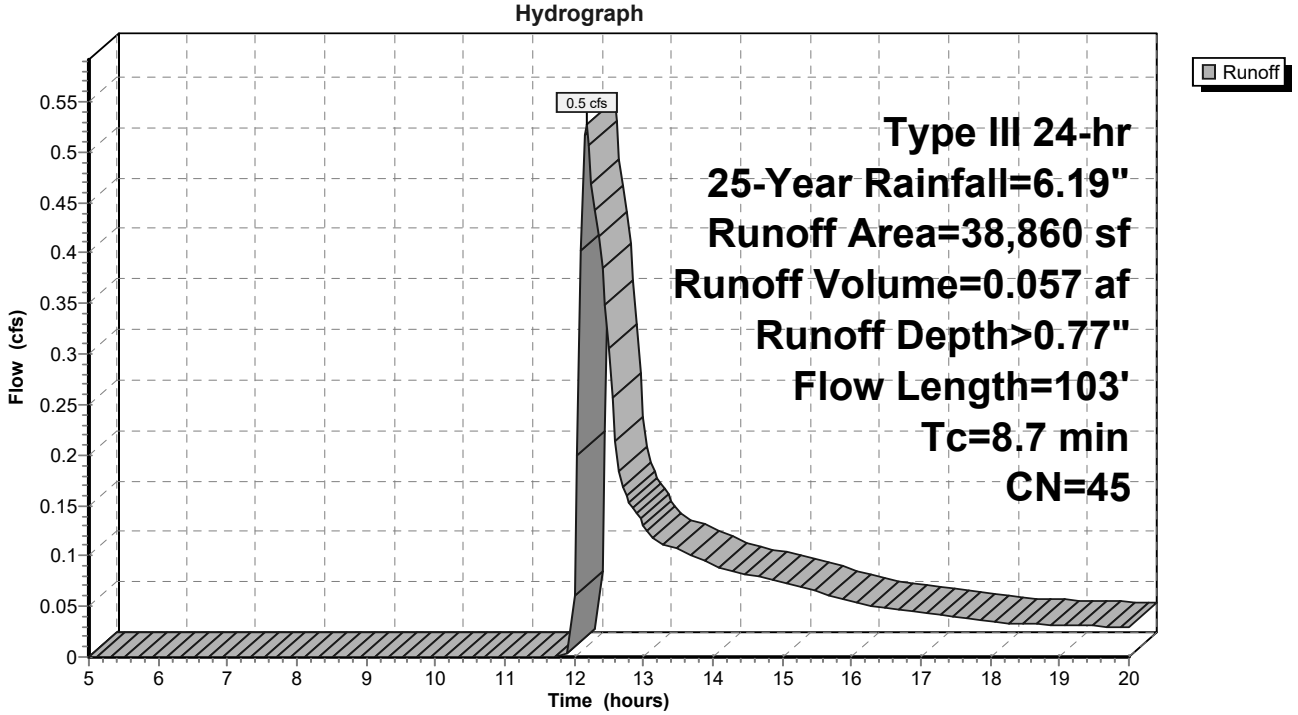
Runoff = 0.5 cfs @ 12.17 hrs, Volume= 0.057 af, Depth> 0.77"
Routed to Reach A : Design Point A: Flow To Wetland

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.19"

Area (sf)	CN	Description
38,860	45	Woods, Poor, HSG A
38,860		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.5	50	0.0500	0.10		**Sheet Flow, A-B** Woods: Light underbrush n= 0.400 P2= 3.20"
0.2	53	0.0900	4.83		**Shallow Concentrated Flow, B-C** Unpaved Kv= 16.1 fps
8.7	103	Total			

Subcatchment A2: Uncontrolled flow



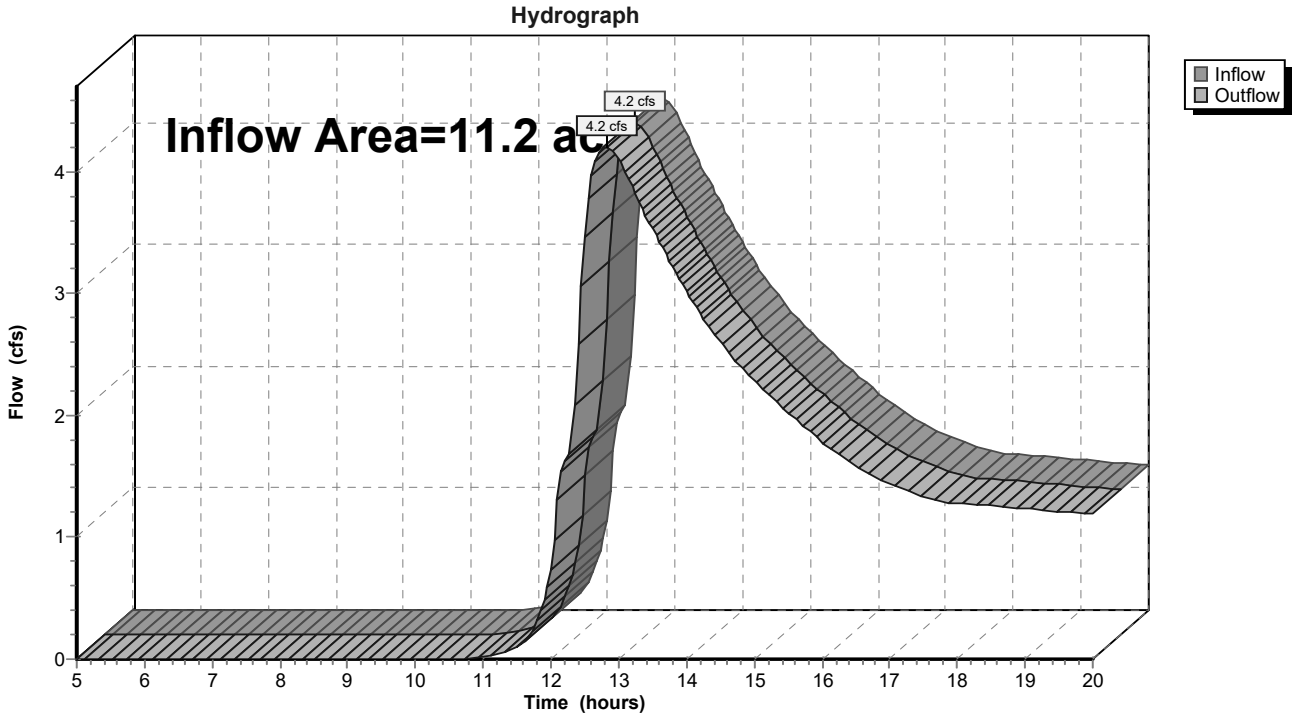
Summary for Reach A: Design Point A: Flow To Wetland

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 11.2 ac, 36.68% Impervious, Inflow Depth > 1.48" for 25-Year event
Inflow = 4.2 cfs @ 12.81 hrs, Volume= 1.385 af
Outflow = 4.2 cfs @ 12.81 hrs, Volume= 1.385 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach A: Design Point A: Flow To Wetland



Summary for Pond P1: Ex. Detention Basin

Inflow Area = 10.3 ac, 39.85% Impervious, Inflow Depth > 2.20" for 25-Year event
Inflow = 24.8 cfs @ 12.15 hrs, Volume= 1.898 af
Outflow = 4.0 cfs @ 12.83 hrs, Volume= 1.328 af, Atten= 84%, Lag= 41.2 min
Primary = 4.0 cfs @ 12.83 hrs, Volume= 1.328 af
Routed to Reach A : Design Point A: Flow To Wetland

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Starting Elev= 30.00' Surf.Area= 10,000 sf Storage= 4,261 cf
Peak Elev= 32.48' @ 12.83 hrs Surf.Area= 20,752 sf Storage= 43,211 cf (38,950 cf above start)

Plug-Flow detention time= 187.3 min calculated for 1.230 af (65% of inflow)
Center-of-Mass det. time= 97.5 min (910.1 - 812.7)

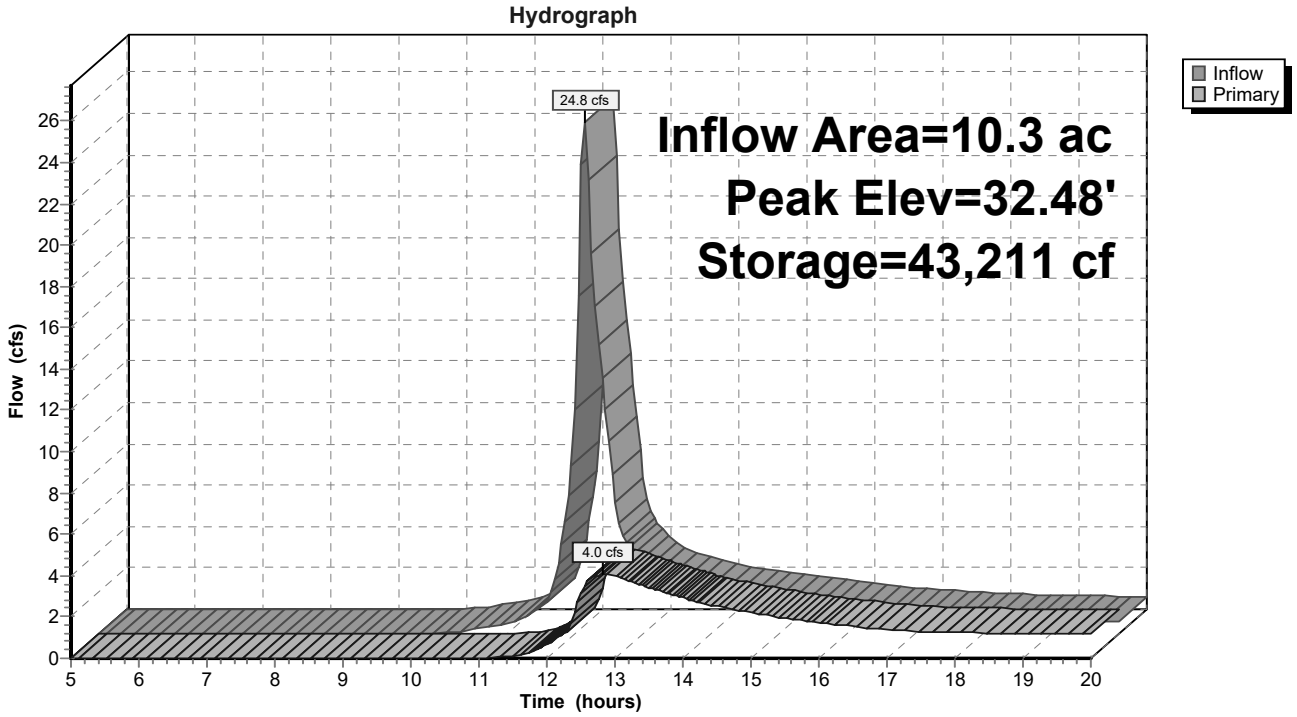
Volume	Invert	Avail.Storage	Storage Description
#1	29.00'	101,165 cf	**Custom Stage Data (Irregular)** Listed below

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
29.00	515	100.0	0	0	515
30.00	10,000	845.0	4,261	4,261	56,542
31.00	14,725	1,210.0	12,287	16,548	116,239
32.00	18,875	1,245.0	16,757	33,305	123,186
33.00	22,820	1,255.0	20,816	54,121	125,536
34.00	26,550	1,275.0	24,661	78,783	129,757
34.80	29,430	1,300.0	22,382	101,165	134,982

Device	Routing	Invert	Outlet Devices
#1	Device 3	32.00'	**2.5' long x 2.80' rise Sharp-Crested Rectangular Weir** 2 End Contraction(s) 3.0' Crest Height
#2	Device 3	30.00'	**6.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#3	Primary	28.91'	**24.0" Round Culvert** L= 40.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 28.91' / 28.51' S= 0.0100 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=4.0 cfs @ 12.83 hrs HW=32.48' (Free Discharge)
└**3=Culvert** (Passes 4.0 cfs of 19.1 cfs potential flow)
  ├**1=Sharp-Crested Rectangular Weir** (Weir Controls 2.6 cfs @ 2.30 fps)
  └**2=Orifice/Grate** (Orifice Controls 1.4 cfs @ 7.18 fps)

Pond P1: Ex. Detention Basin



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A1: Flow to Ex. Detention Runoff Area=450,410 sf 39.85% Impervious Runoff Depth>4.02"
Flow Length=690' Tc=9.6 min CN=64 Runoff=45.7 cfs 3.460 af

Subcatchment A2: Uncontrolled flow Runoff Area=38,860 sf 0.00% Impervious Runoff Depth>1.89"
Flow Length=103' Tc=8.7 min CN=45 Runoff=1.7 cfs 0.141 af

Reach A: Design Point A: Flow To Wetland Inflow=16.0 cfs 2.933 af
Outflow=16.0 cfs 2.933 af

Pond P1: Ex. Detention Basin Peak Elev=33.46' Storage=65,578 cf Inflow=45.7 cfs 3.460 af
Outflow=15.3 cfs 2.792 af

Total Runoff Area = 11.2 ac Runoff Volume = 3.601 af Average Runoff Depth = 3.85"
63.32% Pervious = 7.1 ac 36.68% Impervious = 4.1 ac

Summary for Subcatchment A1: Flow to Ex. Detention Basin

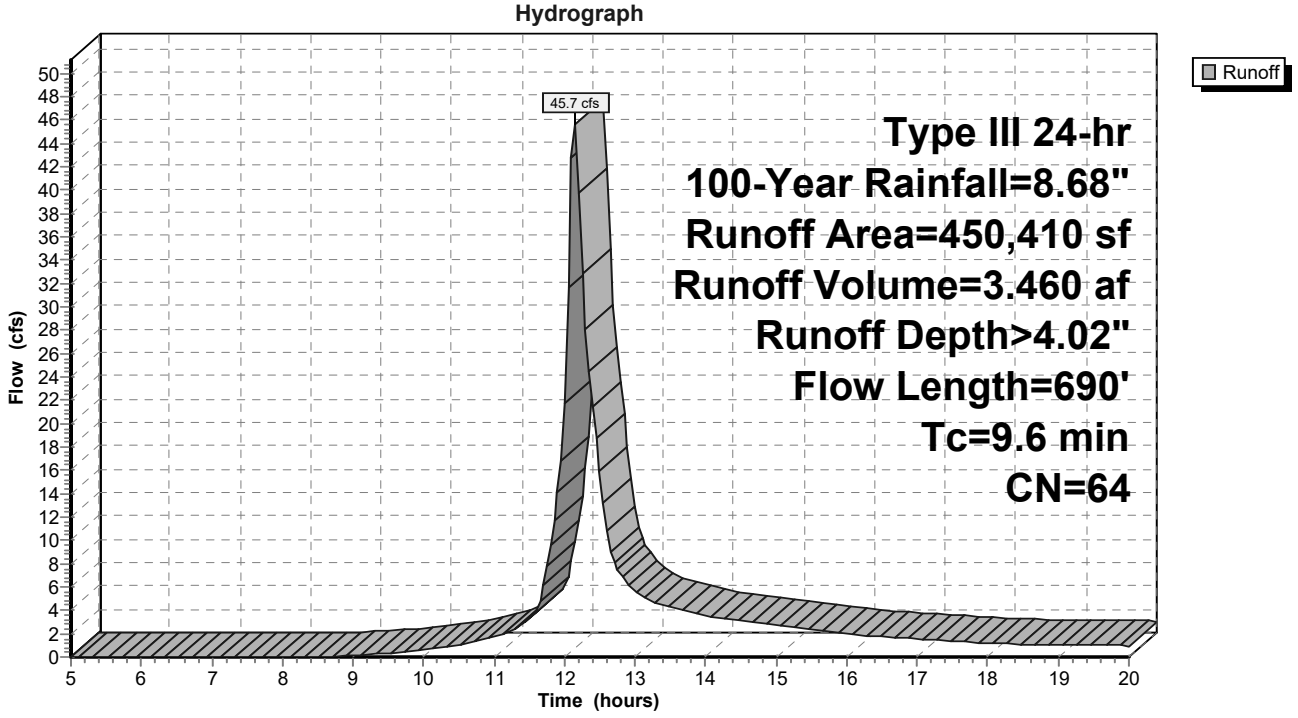
Runoff = 45.7 cfs @ 12.14 hrs, Volume= 3.460 af, Depth> 4.02"
Routed to Pond P1 : Ex. Detention Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=8.68"

	Area (sf)	CN	Description
*	18,731	39	Landscaped islands, Good, HSG A
*	7,841	98	paved sidewalks, HSG A
	82,000	45	Woods, Poor, HSG A
	149,846	98	Paved parking, HSG A
	170,212	39	>75% Grass cover, Good, HSG A
	21,780	98	Water Surface, HSG A
	450,410	64	Weighted Average
	270,943		60.15% Pervious Area
	179,467		39.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3	50	0.0400	0.20		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.20"
5.3	640	0.0100	2.03		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
9.6	690	Total			

Subcatchment A1: Flow to Ex. Detention Basin



Summary for Subcatchment A2: Uncontrolled flow

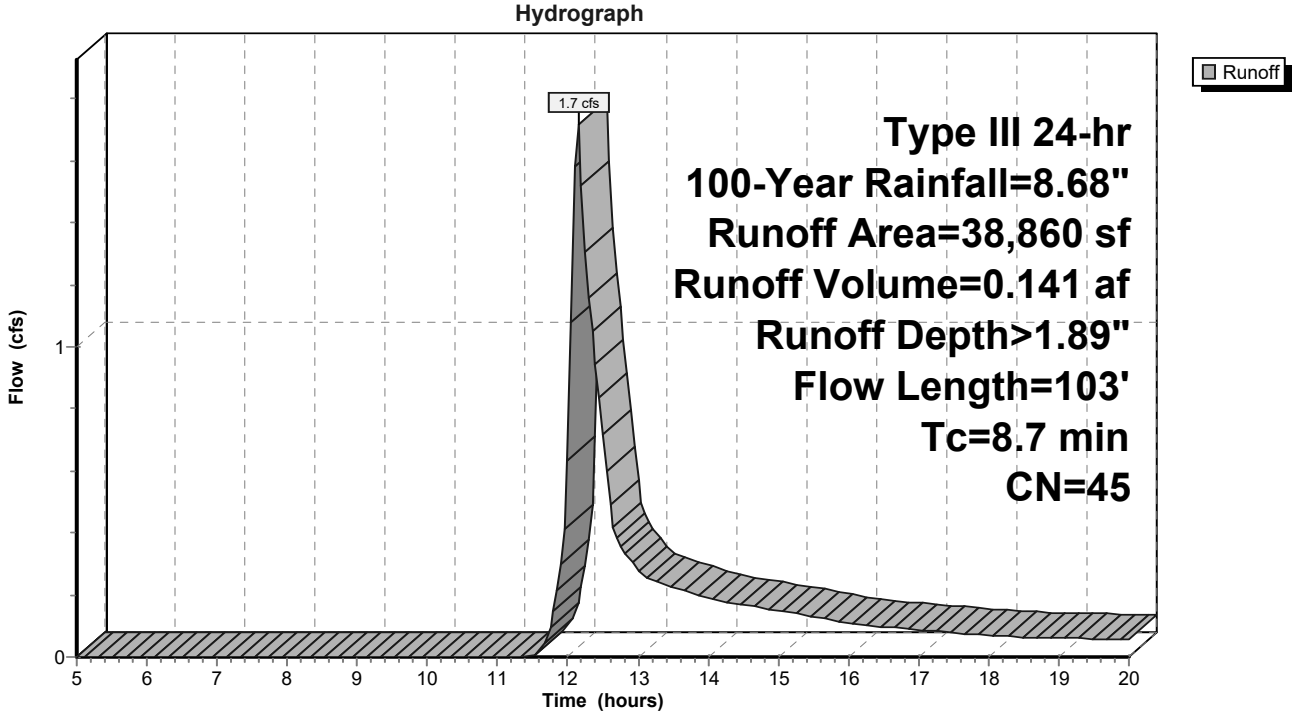
Runoff = 1.7 cfs @ 12.15 hrs, Volume= 0.141 af, Depth> 1.89"
Routed to Reach A : Design Point A: Flow To Wetland

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=8.68"

Area (sf)	CN	Description
38,860	45	Woods, Poor, HSG A
38,860		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.5	50	0.0500	0.10		**Sheet Flow, A-B** Woods: Light underbrush n= 0.400 P2= 3.20"
0.2	53	0.0900	4.83		**Shallow Concentrated Flow, B-C** Unpaved Kv= 16.1 fps
8.7	103	Total			

Subcatchment A2: Uncontrolled flow



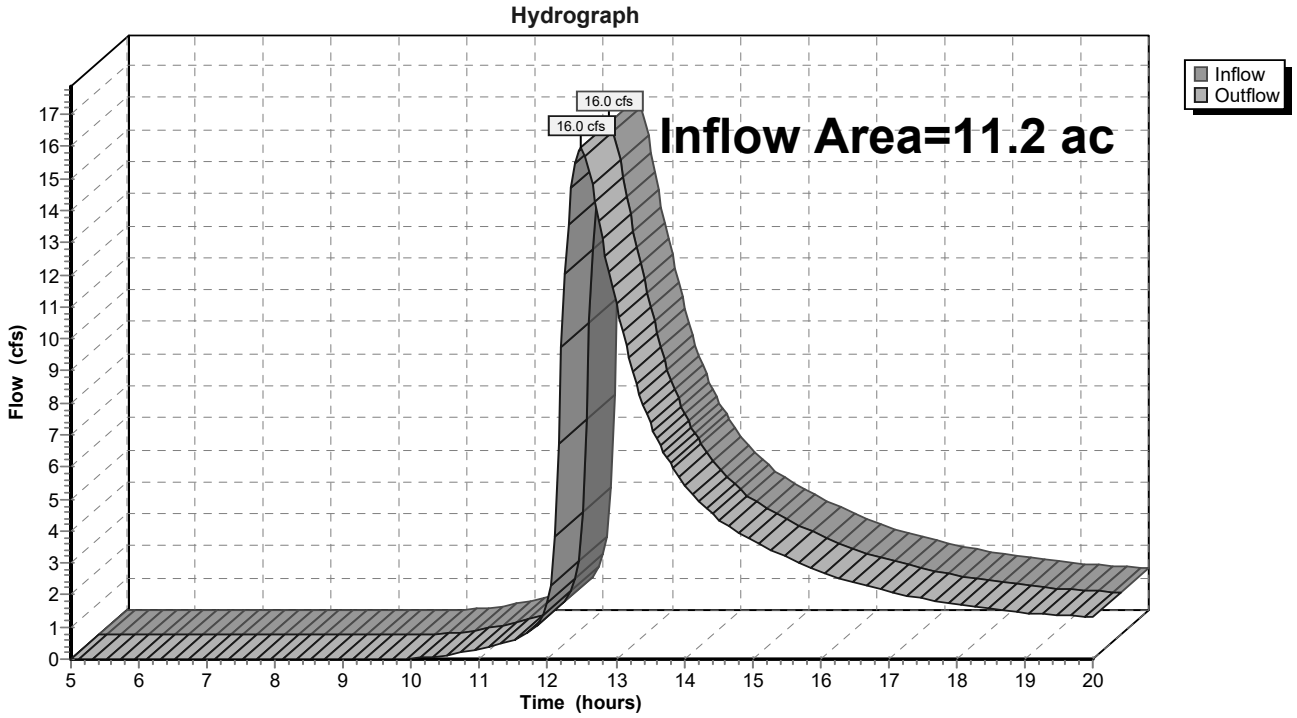
Summary for Reach A: Design Point A: Flow To Wetland

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 11.2 ac, 36.68% Impervious, Inflow Depth > 3.13" for 100-Year event
Inflow = 16.0 cfs @ 12.49 hrs, Volume= 2.933 af
Outflow = 16.0 cfs @ 12.49 hrs, Volume= 2.933 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach A: Design Point A: Flow To Wetland



Summary for Pond P1: Ex. Detention Basin

Inflow Area = 10.3 ac, 39.85% Impervious, Inflow Depth > 4.02" for 100-Year event
Inflow = 45.7 cfs @ 12.14 hrs, Volume= 3.460 af
Outflow = 15.3 cfs @ 12.51 hrs, Volume= 2.792 af, Atten= 67%, Lag= 22.4 min
Primary = 15.3 cfs @ 12.51 hrs, Volume= 2.792 af
Routed to Reach A : Design Point A: Flow To Wetland

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Starting Elev= 30.00' Surf.Area= 10,000 sf Storage= 4,261 cf
Peak Elev= 33.46' @ 12.51 hrs Surf.Area= 24,553 sf Storage= 65,578 cf (61,317 cf above start)

Plug-Flow detention time= 122.3 min calculated for 2.694 af (78% of inflow)
Center-of-Mass det. time= 59.3 min (858.6 - 799.3)

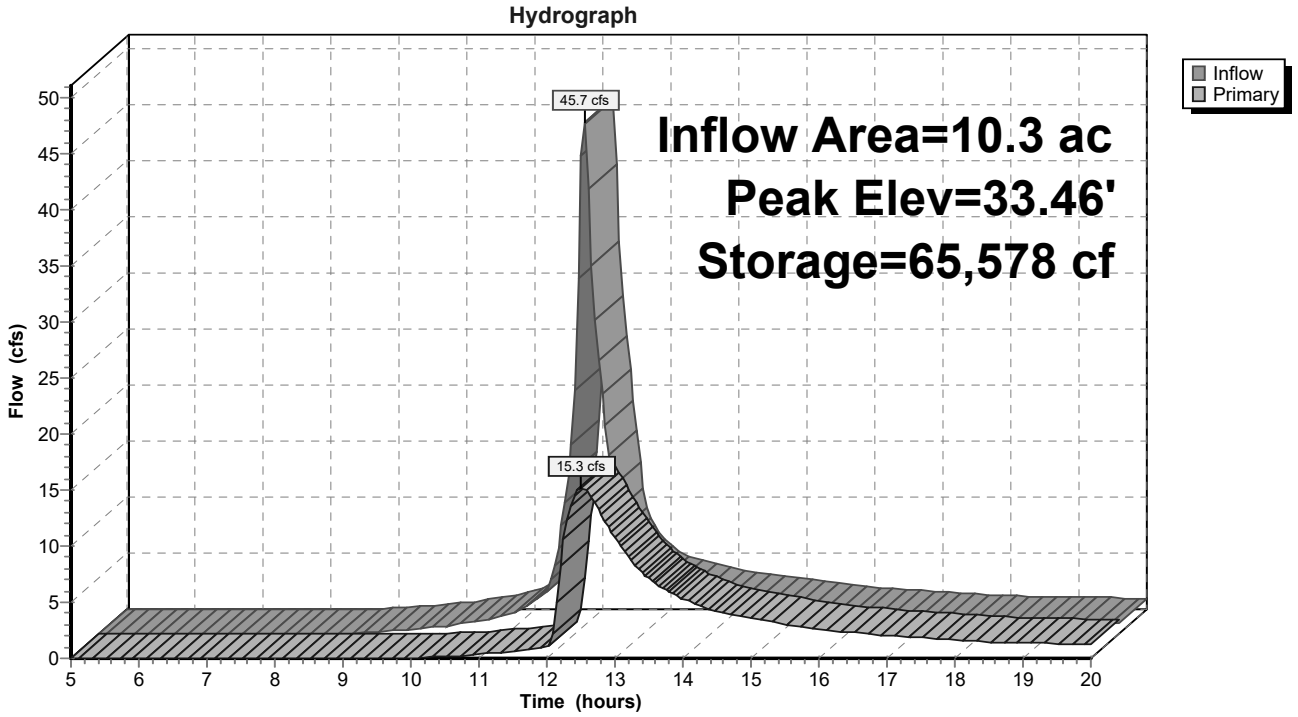
Volume	Invert	Avail.Storage	Storage Description
#1	29.00'	101,165 cf	**Custom Stage Data (Irregular)** Listed below

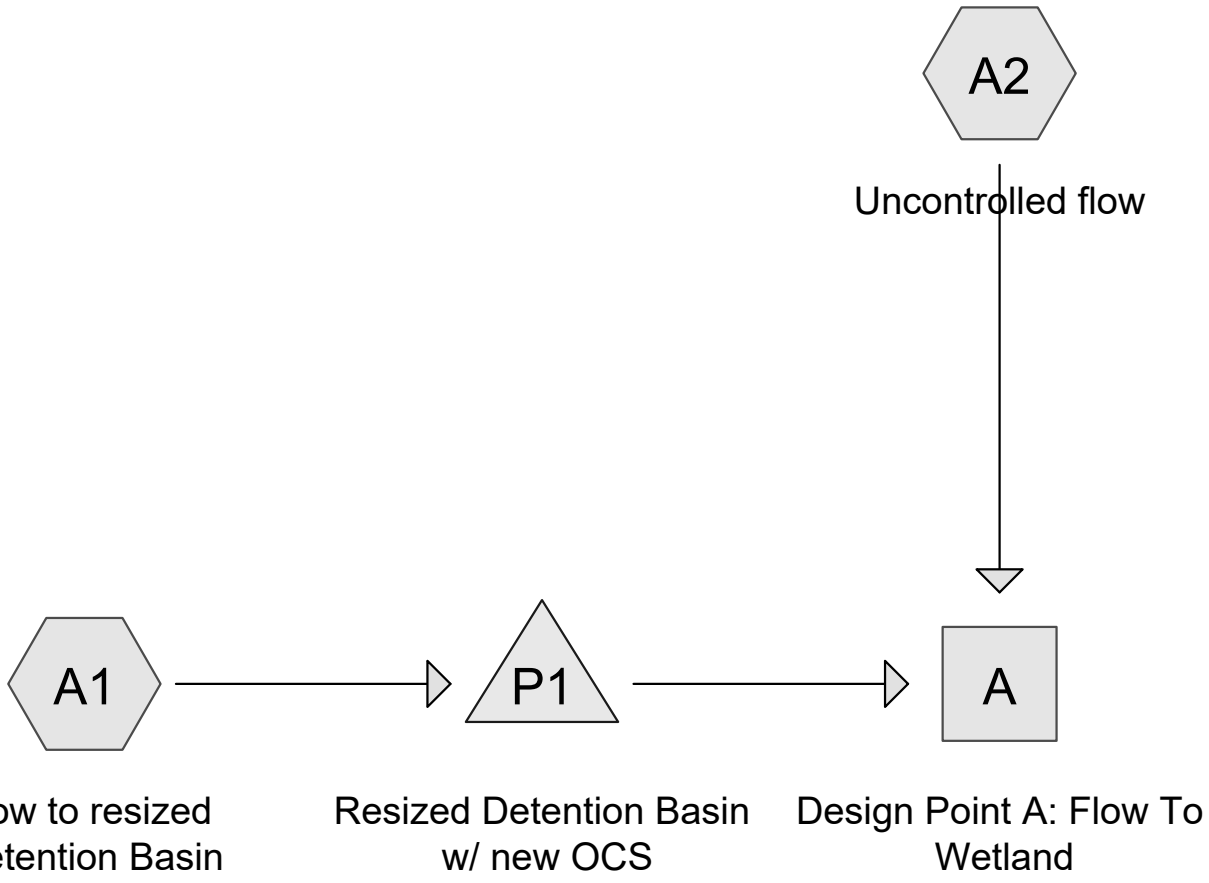
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
29.00	515	100.0	0	0	515
30.00	10,000	845.0	4,261	4,261	56,542
31.00	14,725	1,210.0	12,287	16,548	116,239
32.00	18,875	1,245.0	16,757	33,305	123,186
33.00	22,820	1,255.0	20,816	54,121	125,536
34.00	26,550	1,275.0	24,661	78,783	129,757
34.80	29,430	1,300.0	22,382	101,165	134,982

Device	Routing	Invert	Outlet Devices
#1	Device 3	32.00'	**2.5' long x 2.80' rise Sharp-Crested Rectangular Weir** 2 End Contraction(s) 3.0' Crest Height
#2	Device 3	30.00'	**6.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#3	Primary	28.91'	**24.0" Round Culvert** L= 40.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 28.91' / 28.51' S= 0.0100 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=15.2 cfs @ 12.51 hrs HW=33.46' (Free Discharge)
└**3=Culvert** (Passes 15.2 cfs of 22.5 cfs potential flow)
  ├**1=Sharp-Crested Rectangular Weir** (Weir Controls 13.5 cfs @ 4.19 fps)
  └**2=Orifice/Grate** (Orifice Controls 1.7 cfs @ 8.63 fps)

Pond P1: Ex. Detention Basin





Subcat Reach Pond Link

Routing Diagram for Post Drainage Calcs
Prepared by CEC Inc, Printed 3/8/2024

Project Notes

Rainfall events imported from "Atlas-14-Rain.txt" for 447 MA Plymouth

Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-Year	Type III 24-hr		Default	24.00	1	3.35	2
2	10-Year	Type III 24-hr		Default	24.00	1	4.95	2
3	25-Year	Type III 24-hr		Default	24.00	1	6.19	2
4	100-Year	Type III 24-hr		Default	24.00	1	8.68	2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
1.8	39	>75% Grass cover, Good, HSG A (A1)
0.1	98	Conc. slab areas/Shed roofs, HSG A (A1)
0.1	76	Field Transmission Material Storage area, HSG A (A1)
2.2	76	Gravel areas, HSG A (A1)
4.1	98	Paved parking, HSG A (A1)
0.8	98	Water Surface, HSG A (A1)
2.0	45	Woods, Poor, HSG A (A1, A2)
0.2	98	paved sidewalks, HSG A (A1)
11.2	**75**	**TOTAL AREA**

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
11.2	HSG A	A1, A2
0.0	HSG B	
0.0	HSG C	
0.0	HSG D	
0.0	Other	
11.2		**TOTAL AREA**

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
1.8	0.0	0.0	0.0	0.0	1.8	>75% Grass cover, Good	A1
0.1	0.0	0.0	0.0	0.0	0.1	Conc. slab areas/Shed roofs	A1
0.1	0.0	0.0	0.0	0.0	0.1	Field Transmission Material Storage area	A1
2.2	0.0	0.0	0.0	0.0	2.2	Gravel areas	A1
4.1	0.0	0.0	0.0	0.0	4.1	Paved parking	A1
0.8	0.0	0.0	0.0	0.0	0.8	Water Surface	A1
2.0	0.0	0.0	0.0	0.0	2.0	Woods, Poor	A1, A2
0.2	0.0	0.0	0.0	0.0	0.2	paved sidewalks	A1
11.2	**0.0**	**0.0**	**0.0**	**0.0**	**11.2**	**TOTAL AREA**	

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentA1: Flow to resized Runoff Area=450,410 sf 49.60% Impervious Runoff Depth>1.21"
Flow Length=690' Tc=9.6 min CN=77 Runoff=13.6 cfs 1.044 af

SubcatchmentA2: Uncontrolled flow Runoff Area=38,860 sf 0.00% Impervious Runoff Depth>0.04"
Flow Length=103' Tc=8.7 min CN=45 Runoff=0.0 cfs 0.003 af

Reach A: Design Point A: Flow To Wetland Inflow=0.5 cfs 0.302 af
Outflow=0.5 cfs 0.302 af

Pond P1: Resized Detention Basin w/ new Peak Elev=32.23' Storage=65,562 cf Inflow=13.6 cfs 1.044 af
Outflow=0.5 cfs 0.298 af

Total Runoff Area = 11.2 ac Runoff Volume = 1.048 af Average Runoff Depth = 1.12"
54.34% Pervious = 6.1 ac 45.66% Impervious = 5.1 ac

Summary for Subcatchment A1: Flow to resized Detention Basin

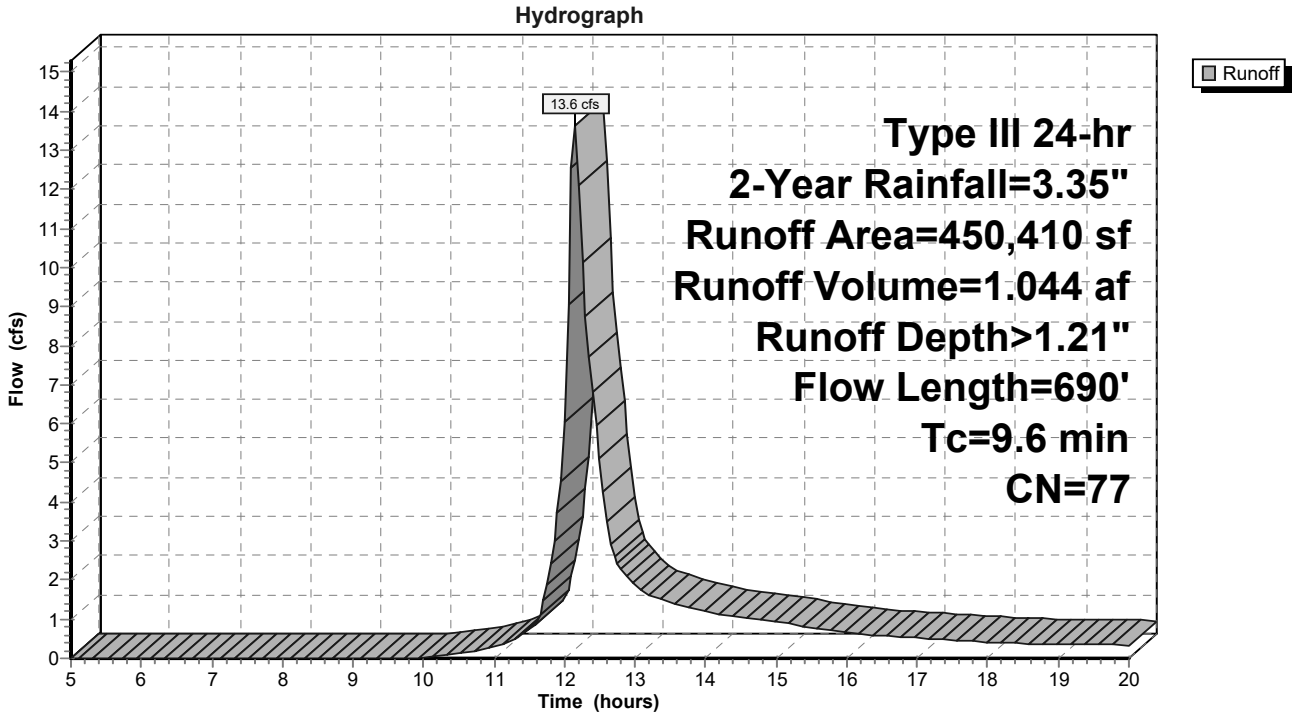
Runoff = 13.6 cfs @ 12.15 hrs, Volume= 1.044 af, Depth> 1.21"
Routed to Pond P1 : Resized Detention Basin w/ new OCS

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.35"

	Area (sf)	CN	Description
*	7,841	98	paved sidewalks, HSG A
	47,778	45	Woods, Poor, HSG A
	177,900	98	Paved parking, HSG A
	79,926	39	>75% Grass cover, Good, HSG A
	32,875	98	Water Surface, HSG A
*	94,298	76	Gravel areas, HSG A
*	4,792	98	Conc. slab areas/Shed roofs, HSG A
*	5,000	76	Field Transmission Material Storage area, HSG A
	450,410	77	Weighted Average
	227,002		50.40% Pervious Area
	223,408		49.60% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3	50	0.0400	0.20		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.20"
5.3	640	0.0100	2.03		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
9.6	690	Total			

Subcatchment A1: Flow to resized Detention Basin



Summary for Subcatchment A2: Uncontrolled flow

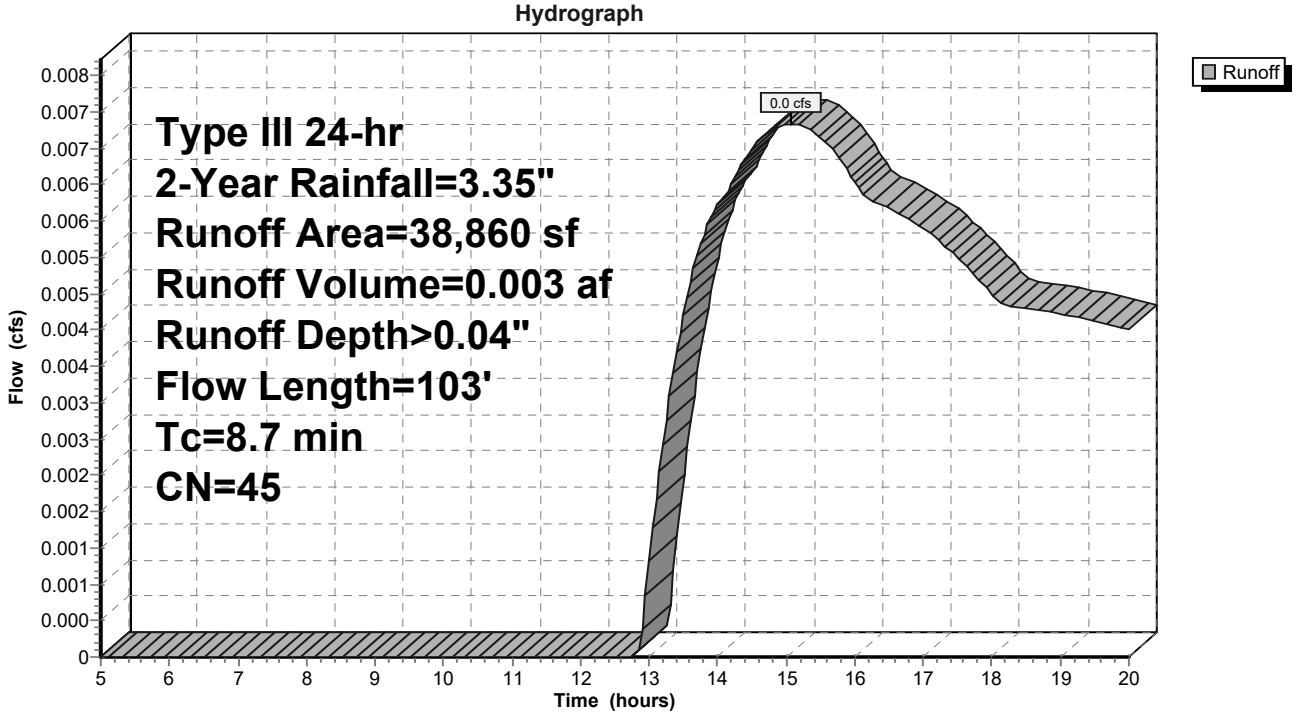
Runoff = 0.0 cfs @ 15.06 hrs, Volume= 0.003 af, Depth> 0.04"
Routed to Reach A : Design Point A: Flow To Wetland

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.35"

Area (sf)	CN	Description
38,860	45	Woods, Poor, HSG A
38,860		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.5	50	0.0500	0.10		**Sheet Flow, A-B** Woods: Light underbrush n= 0.400 P2= 3.20"
0.2	53	0.0900	4.83		**Shallow Concentrated Flow, B-C** Unpaved Kv= 16.1 fps
8.7	103	Total			

Subcatchment A2: Uncontrolled flow



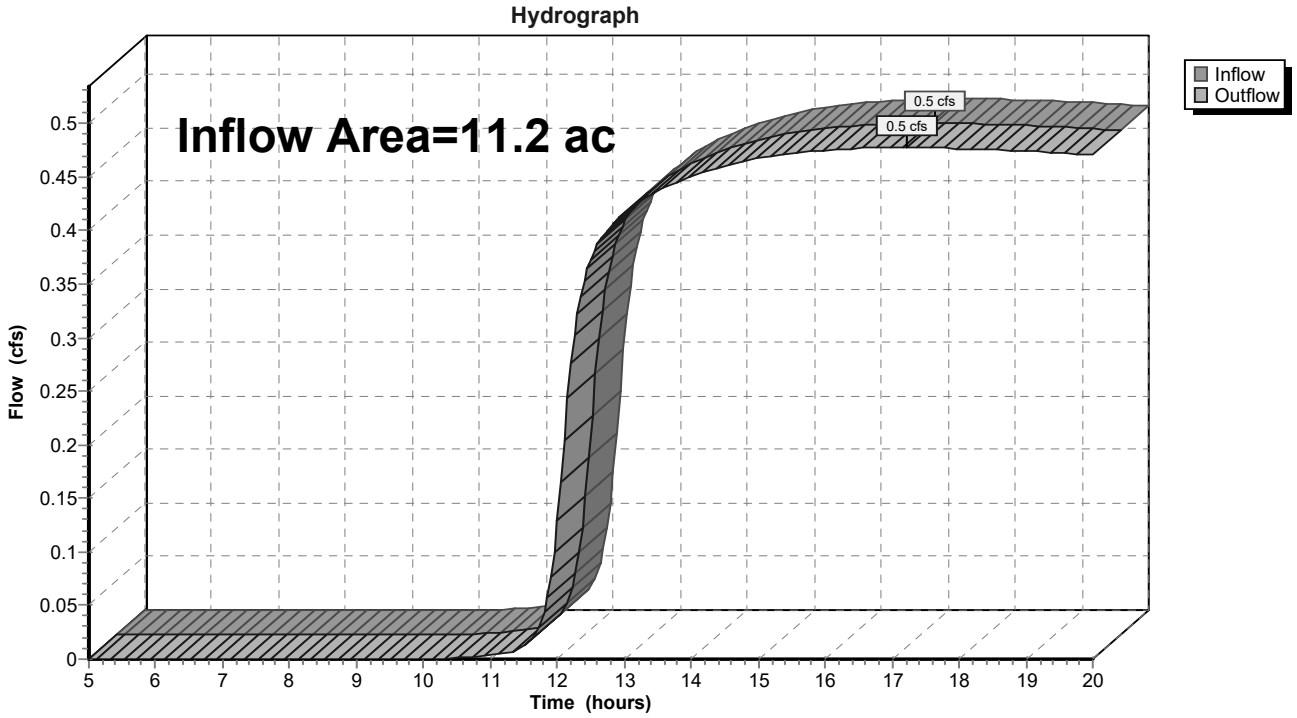
Summary for Reach A: Design Point A: Flow To Wetland

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 11.2 ac, 45.66% Impervious, Inflow Depth > 0.32" for 2-Year event
Inflow = 0.5 cfs @ 17.22 hrs, Volume= 0.302 af
Outflow = 0.5 cfs @ 17.22 hrs, Volume= 0.302 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach A: Design Point A: Flow To Wetland



Summary for Pond P1: Resized Detention Basin w/ new OCS

Inflow Area = 10.3 ac, 49.60% Impervious, Inflow Depth > 1.21" for 2-Year event
Inflow = 13.6 cfs @ 12.15 hrs, Volume= 1.044 af
Outflow = 0.5 cfs @ 17.49 hrs, Volume= 0.298 af, Atten= 97%, Lag= 320.7 min
Primary = 0.5 cfs @ 17.49 hrs, Volume= 0.298 af
Routed to Reach A : Design Point A: Flow To Wetland

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Starting Elev= 30.80' Surf.Area= 20,690 sf Storage= 32,247 cf
Peak Elev= 32.23' @ 17.49 hrs Surf.Area= 25,985 sf Storage= 65,562 cf (33,315 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass det. time= 156.9 min (968.8 - 811.9)

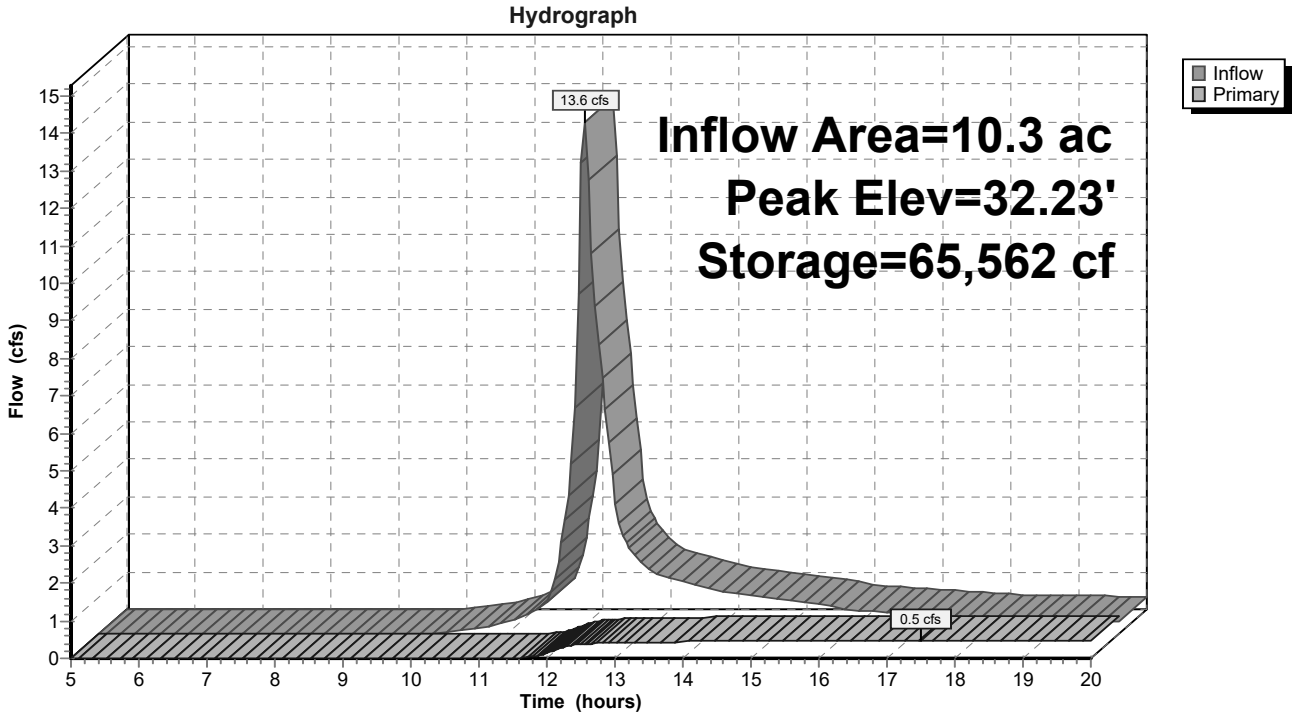
Volume	Invert	Avail.Storage	Storage Description
#1	29.00'	143,189 cf	**Custom Stage Data (Irregular)** Listed below

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
29.00	14,700	950.0	0	0	14,700
30.00	18,250	1,005.0	16,443	16,443	23,312
31.00	21,300	1,220.0	19,755	36,198	61,396
32.00	25,150	1,250.0	23,198	59,397	67,421
33.00	28,800	1,260.0	26,954	86,351	69,780
34.00	32,550	1,285.0	30,656	117,007	75,000
34.80	32,905	1,295.0	26,182	143,189	77,298

Device	Routing	Invert	Outlet Devices
#1	Primary	29.18'	**24.0" Round Culvert** L= 35.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 29.18' / 29.00' S= 0.0051 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf
#2	Device 1	30.80'	**4.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#3	Device 1	32.50'	**4.0" Vert. Orifice/Grate X 2.00** C= 0.600 Limited to weir flow at low heads
#4	Device 1	33.83'	**4.0' long x 0.5' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#5	Device 1	34.50'	**2.0" x 2.0" Horiz. Orifice/Grate X 8.00 columns** X 8 rows C= 0.600 in 24.0" x 24.0" Grate (44% open area) Limited to weir flow at low heads

Primary OutFlow Max=0.5 cfs @ 17.49 hrs HW=32.23' (Free Discharge)
└**1=Culvert** (Passes 0.5 cfs of 17.1 cfs potential flow)
  ├**2=Orifice/Grate** (Orifice Controls 0.5 cfs @ 5.41 fps)
  ├**3=Orifice/Grate** (Controls 0.0 cfs)
  ├**4=Broad-Crested Rectangular Weir** (Controls 0.0 cfs)
  └**5=Orifice/Grate** (Controls 0.0 cfs)

Pond P1: Resized Detention Basin w/ new OCS



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentA1: Flow to resized Runoff Area=450,410 sf 49.60% Impervious Runoff Depth>2.40"
Flow Length=690' Tc=9.6 min CN=77 Runoff=27.3 cfs 2.066 af

SubcatchmentA2: Uncontrolled flow Runoff Area=38,860 sf 0.00% Impervious Runoff Depth>0.36"
Flow Length=103' Tc=8.7 min CN=45 Runoff=0.2 cfs 0.027 af

Reach A: Design Point A: Flow To Wetland Inflow=1.3 cfs 0.817 af
Outflow=1.3 cfs 0.817 af

Pond P1: Resized Detention Basin w/ new Peak Elev=33.26' Storage=94,465 cf Inflow=27.3 cfs 2.066 af
Outflow=1.3 cfs 0.790 af

Total Runoff Area = 11.2 ac Runoff Volume = 2.093 af Average Runoff Depth = 2.24"
54.34% Pervious = 6.1 ac 45.66% Impervious = 5.1 ac

Summary for Subcatchment A1: Flow to resized Detention Basin

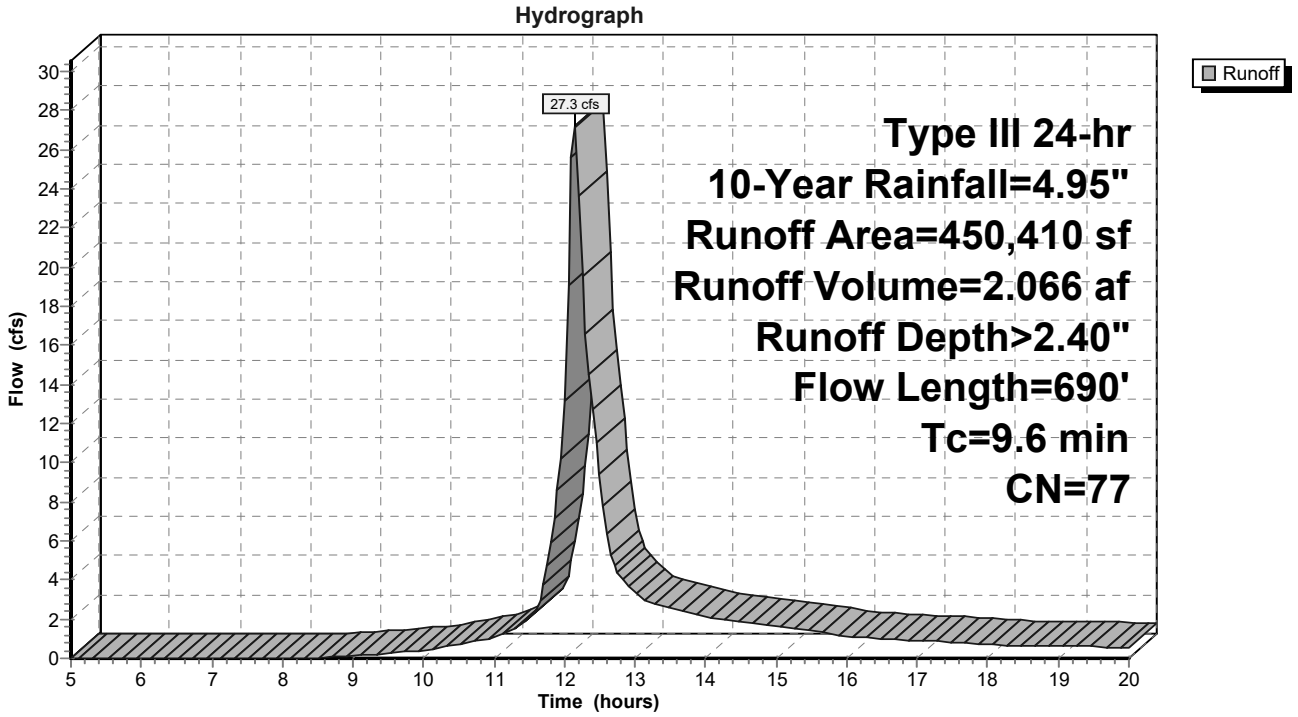
Runoff = 27.3 cfs @ 12.14 hrs, Volume= 2.066 af, Depth> 2.40"
Routed to Pond P1 : Resized Detention Basin w/ new OCS

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=4.95"

	Area (sf)	CN	Description
*	7,841	98	paved sidewalks, HSG A
	47,778	45	Woods, Poor, HSG A
	177,900	98	Paved parking, HSG A
	79,926	39	>75% Grass cover, Good, HSG A
	32,875	98	Water Surface, HSG A
*	94,298	76	Gravel areas, HSG A
*	4,792	98	Conc. slab areas/Shed roofs, HSG A
*	5,000	76	Field Transmission Material Storage area, HSG A
	450,410	77	Weighted Average
	227,002		50.40% Pervious Area
	223,408		49.60% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3	50	0.0400	0.20		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.20"
5.3	640	0.0100	2.03		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
9.6	690	Total			

Subcatchment A1: Flow to resized Detention Basin



Summary for Subcatchment A2: Uncontrolled flow

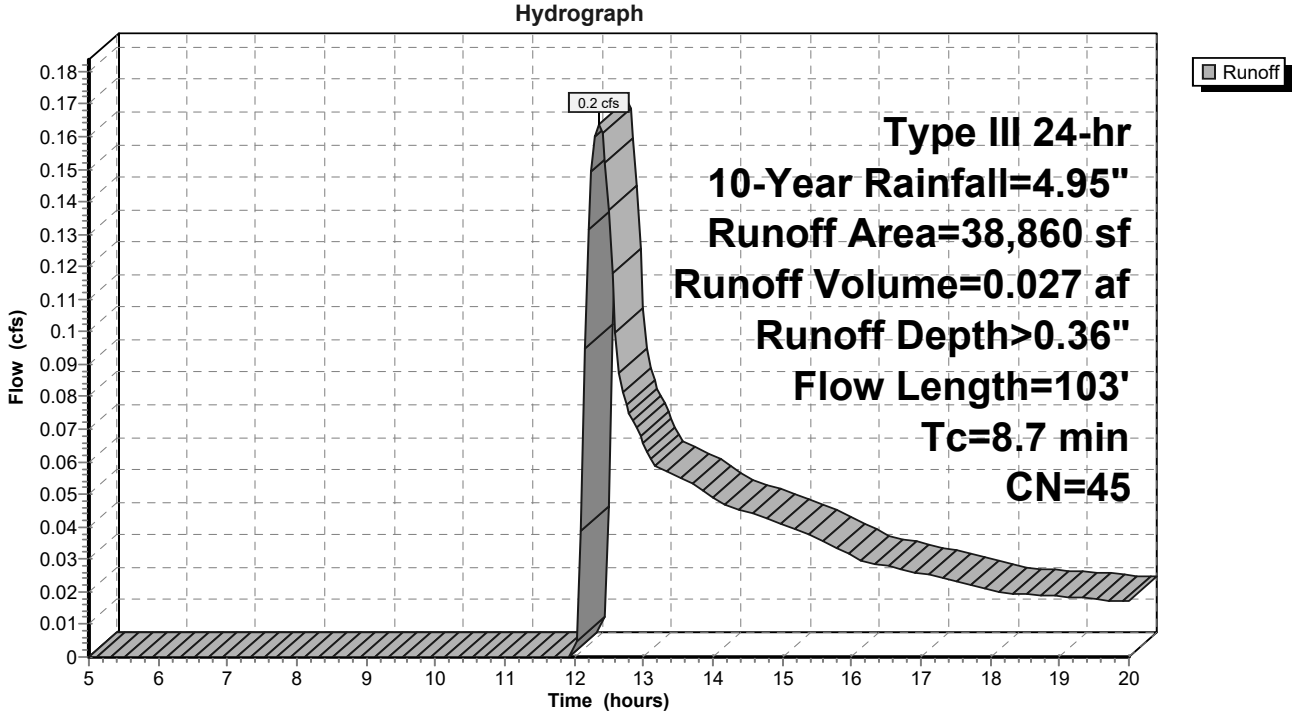
Runoff = 0.2 cfs @ 12.35 hrs, Volume= 0.027 af, Depth> 0.36"
Routed to Reach A : Design Point A: Flow To Wetland

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=4.95"

Area (sf)	CN	Description
38,860	45	Woods, Poor, HSG A
38,860		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.5	50	0.0500	0.10		**Sheet Flow, A-B** Woods: Light underbrush n= 0.400 P2= 3.20"
0.2	53	0.0900	4.83		**Shallow Concentrated Flow, B-C** Unpaved Kv= 16.1 fps
8.7	103	Total			

Subcatchment A2: Uncontrolled flow



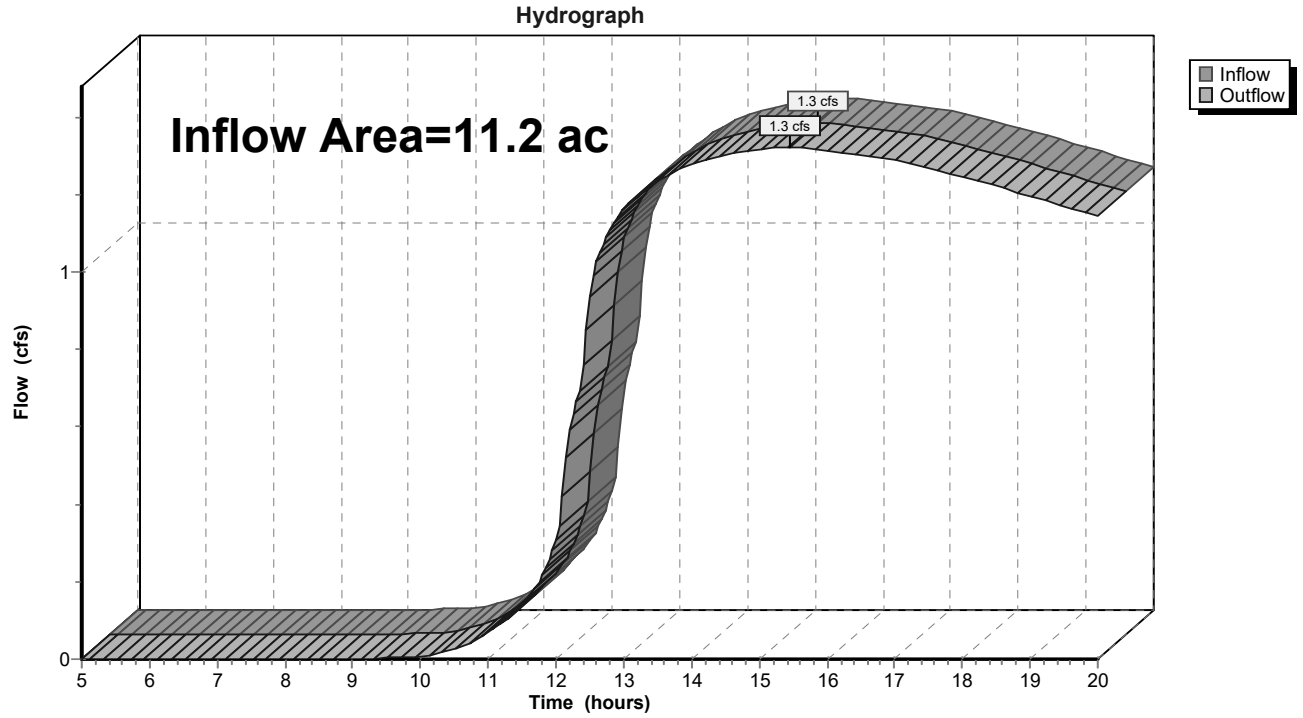
Summary for Reach A: Design Point A: Flow To Wetland

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 11.2 ac, 45.66% Impervious, Inflow Depth > 0.87" for 10-Year event
Inflow = 1.3 cfs @ 15.44 hrs, Volume= 0.817 af
Outflow = 1.3 cfs @ 15.44 hrs, Volume= 0.817 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach A: Design Point A: Flow To Wetland



Summary for Pond P1: Resized Detention Basin w/ new OCS

Inflow Area = 10.3 ac, 49.60% Impervious, Inflow Depth > 2.40" for 10-Year event
Inflow = 27.3 cfs @ 12.14 hrs, Volume= 2.066 af
Outflow = 1.3 cfs @ 15.71 hrs, Volume= 0.790 af, Atten= 95%, Lag= 213.9 min
Primary = 1.3 cfs @ 15.71 hrs, Volume= 0.790 af
Routed to Reach A : Design Point A: Flow To Wetland

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Starting Elev= 30.80' Surf.Area= 20,690 sf Storage= 32,247 cf
Peak Elev= 33.26' @ 15.71 hrs Surf.Area= 29,793 sf Storage= 94,465 cf (62,218 cf above start)

Plug-Flow detention time= 590.7 min calculated for 0.050 af (2% of inflow)
Center-of-Mass det. time= 168.3 min (965.0 - 796.6)

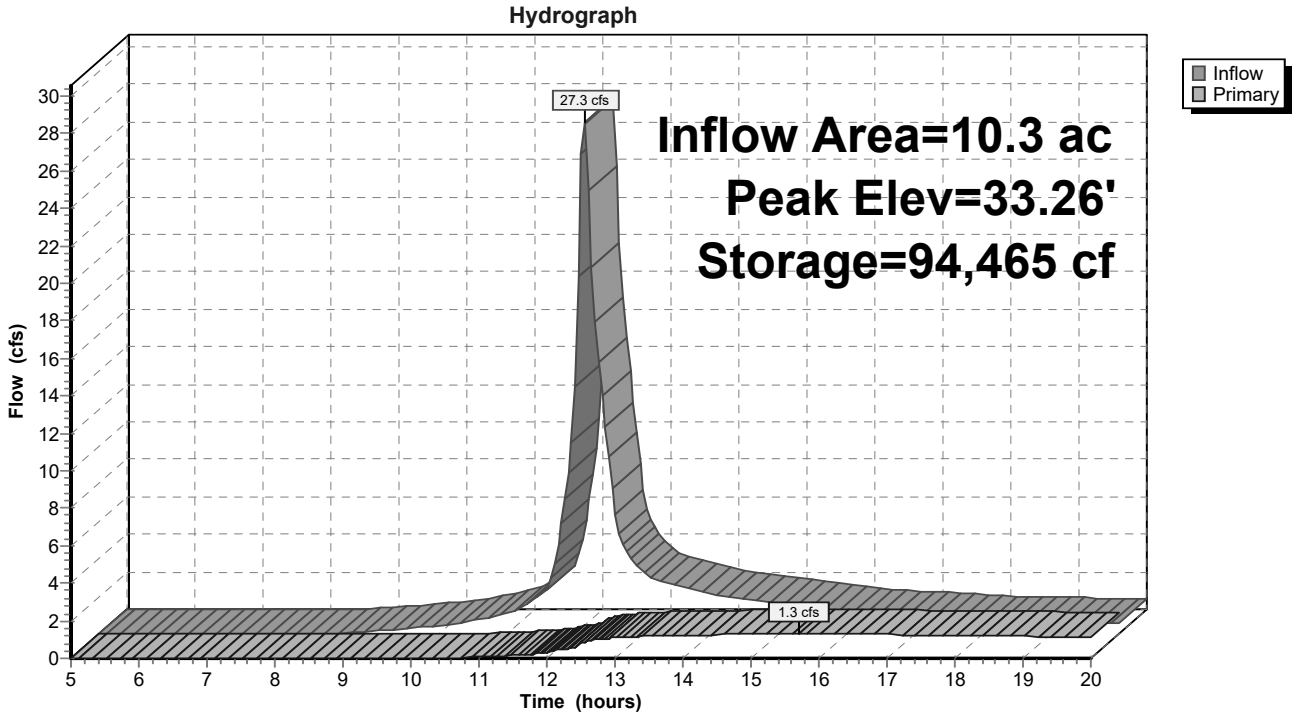
Volume	Invert	Avail.Storage	Storage Description
#1	29.00'	143,189 cf	**Custom Stage Data (Irregular)** Listed below

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
29.00	14,700	950.0	0	0	14,700
30.00	18,250	1,005.0	16,443	16,443	23,312
31.00	21,300	1,220.0	19,755	36,198	61,396
32.00	25,150	1,250.0	23,198	59,397	67,421
33.00	28,800	1,260.0	26,954	86,351	69,780
34.00	32,550	1,285.0	30,656	117,007	75,000
34.80	32,905	1,295.0	26,182	143,189	77,298

Device	Routing	Invert	Outlet Devices
#1	Primary	29.18'	**24.0" Round Culvert** L= 35.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 29.18' / 29.00' S= 0.0051 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf
#2	Device 1	30.80'	**4.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#3	Device 1	32.50'	**4.0" Vert. Orifice/Grate X 2.00** C= 0.600 Limited to weir flow at low heads
#4	Device 1	33.83'	**4.0' long x 0.5' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#5	Device 1	34.50'	**2.0" x 2.0" Horiz. Orifice/Grate X 8.00 columns** X 8 rows C= 0.600 in 24.0" x 24.0" Grate (44% open area) Limited to weir flow at low heads

Primary OutFlow Max=1.3 cfs @ 15.71 hrs HW=33.26' (Free Discharge)
- **1=Culvert** (Passes 1.3 cfs of 21.0 cfs potential flow)
  - **2=Orifice/Grate** (Orifice Controls 0.6 cfs @ 7.30 fps)
  - **3=Orifice/Grate** (Orifice Controls 0.6 cfs @ 3.72 fps)
  - **4=Broad-Crested Rectangular Weir** (Controls 0.0 cfs)
  - **5=Orifice/Grate** (Controls 0.0 cfs)

Pond P1: Resized Detention Basin w/ new OCS



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A1: Flow to resized Runoff Area=450,410 sf 49.60% Impervious Runoff Depth>3.40"
Flow Length=690' Tc=9.6 min CN=77 Runoff=38.5 cfs 2.933 af

Subcatchment A2: Uncontrolled flow Runoff Area=38,860 sf 0.00% Impervious Runoff Depth>0.77"
Flow Length=103' Tc=8.7 min CN=45 Runoff=0.5 cfs 0.057 af

Reach A: Design Point A: Flow To Wetland Inflow=2.5 cfs 1.305 af
Outflow=2.5 cfs 1.305 af

Pond P1: Resized Detention Basin w/ new Peak Elev=34.00' Storage=116,916 cf Inflow=38.5 cfs 2.933 af
Outflow=2.5 cfs 1.247 af

Total Runoff Area = 11.2 ac Runoff Volume = 2.990 af Average Runoff Depth = 3.19"
54.34% Pervious = 6.1 ac 45.66% Impervious = 5.1 ac

Summary for Subcatchment A1: Flow to resized Detention Basin

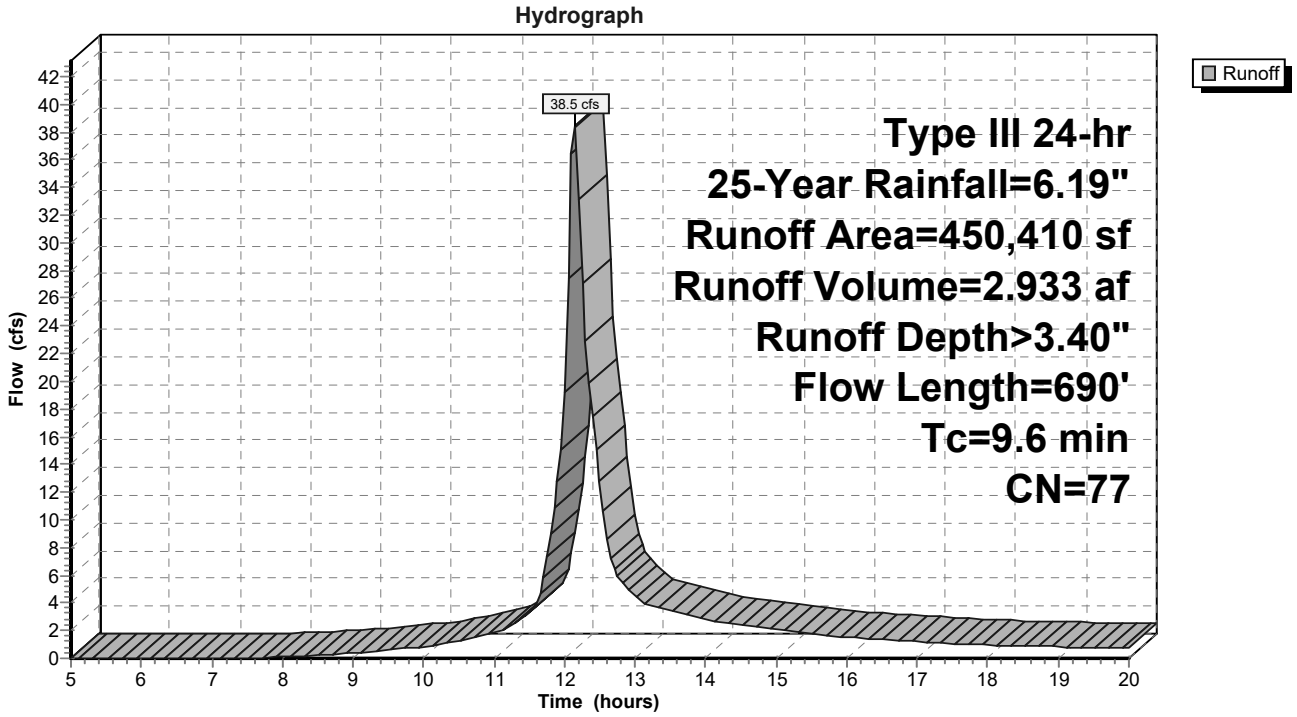
Runoff = 38.5 cfs @ 12.14 hrs, Volume= 2.933 af, Depth> 3.40"
Routed to Pond P1 : Resized Detention Basin w/ new OCS

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.19"

	Area (sf)	CN	Description
*	7,841	98	paved sidewalks, HSG A
	47,778	45	Woods, Poor, HSG A
	177,900	98	Paved parking, HSG A
	79,926	39	>75% Grass cover, Good, HSG A
	32,875	98	Water Surface, HSG A
*	94,298	76	Gravel areas, HSG A
*	4,792	98	Conc. slab areas/Shed roofs, HSG A
*	5,000	76	Field Transmission Material Storage area, HSG A
	450,410	77	Weighted Average
	227,002		50.40% Pervious Area
	223,408		49.60% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3	50	0.0400	0.20		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.20"
5.3	640	0.0100	2.03		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
9.6	690	Total			

Subcatchment A1: Flow to resized Detention Basin



Summary for Subcatchment A2: Uncontrolled flow

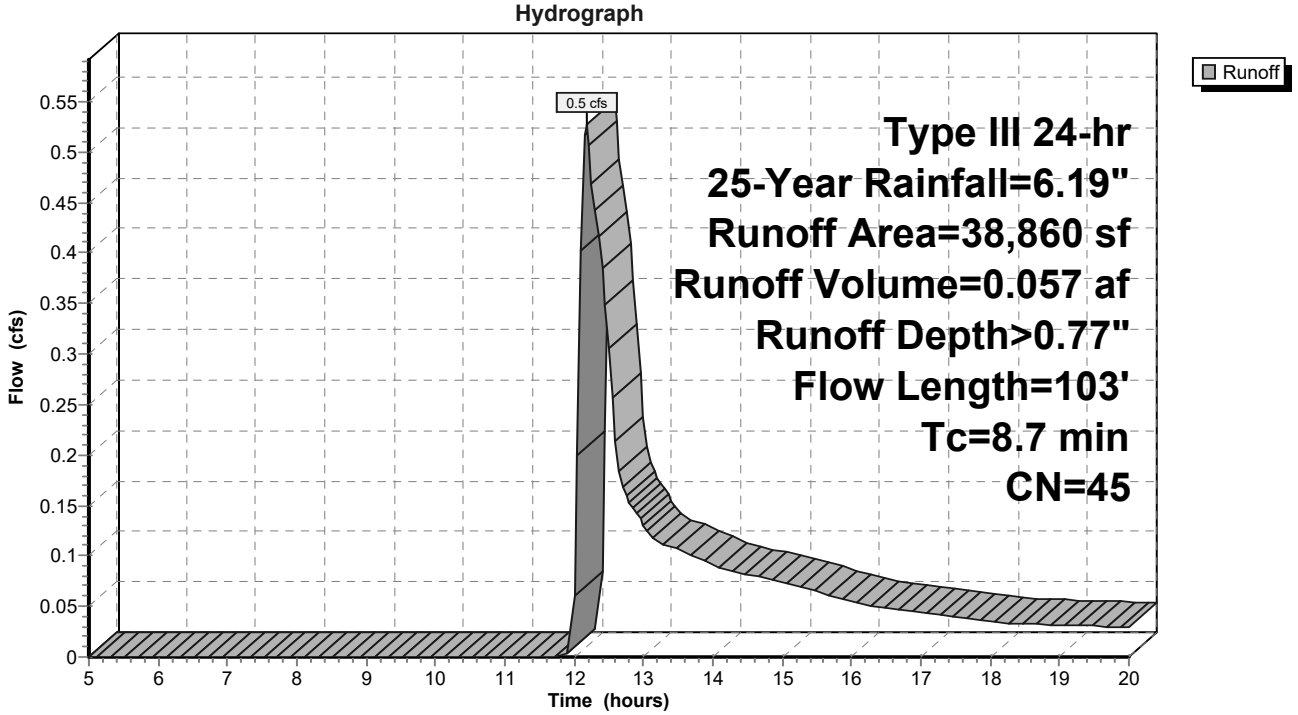
Runoff = 0.5 cfs @ 12.17 hrs, Volume= 0.057 af, Depth> 0.77"
Routed to Reach A : Design Point A: Flow To Wetland

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=6.19"

Area (sf)	CN	Description
38,860	45	Woods, Poor, HSG A
38,860		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.5	50	0.0500	0.10		**Sheet Flow, A-B** Woods: Light underbrush n= 0.400 P2= 3.20"
0.2	53	0.0900	4.83		**Shallow Concentrated Flow, B-C** Unpaved Kv= 16.1 fps
8.7	103	Total			

Subcatchment A2: Uncontrolled flow



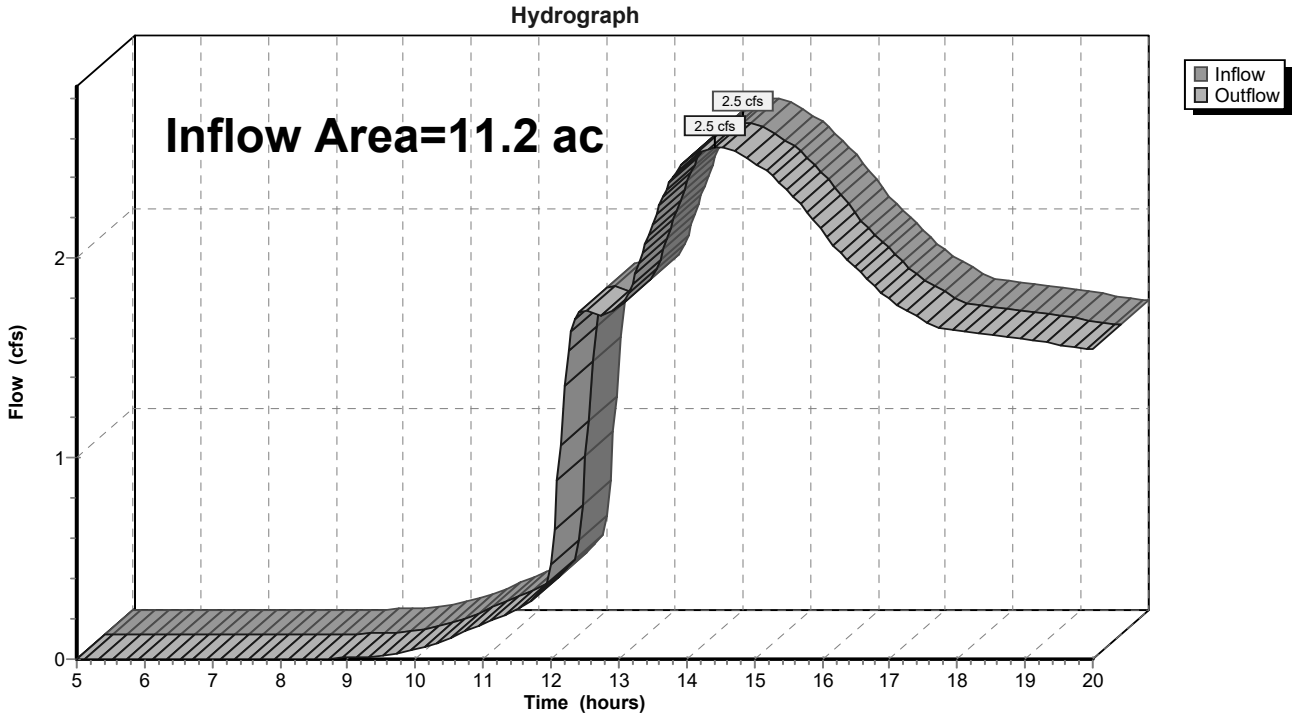
Summary for Reach A: Design Point A: Flow To Wetland

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 11.2 ac, 45.66% Impervious, Inflow Depth > 1.39" for 25-Year event
Inflow = 2.5 cfs @ 14.43 hrs, Volume= 1.305 af
Outflow = 2.5 cfs @ 14.43 hrs, Volume= 1.305 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach A: Design Point A: Flow To Wetland



Summary for Pond P1: Resized Detention Basin w/ new OCS

Inflow Area = 10.3 ac, 49.60% Impervious, Inflow Depth > 3.40" for 25-Year event
Inflow = 38.5 cfs @ 12.14 hrs, Volume= 2.933 af
Outflow = 2.5 cfs @ 14.46 hrs, Volume= 1.247 af, Atten= 94%, Lag= 139.4 min
Primary = 2.5 cfs @ 14.46 hrs, Volume= 1.247 af
Routed to Reach A : Design Point A: Flow To Wetland

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Starting Elev= 30.80' Surf.Area= 20,690 sf Storage= 32,247 cf
Peak Elev= 34.00' @ 14.46 hrs Surf.Area= 32,539 sf Storage= 116,916 cf (84,668 cf above start)

Plug-Flow detention time= 420.6 min calculated for 0.507 af (17% of inflow)
Center-of-Mass det. time= 159.0 min (947.6 - 788.6)

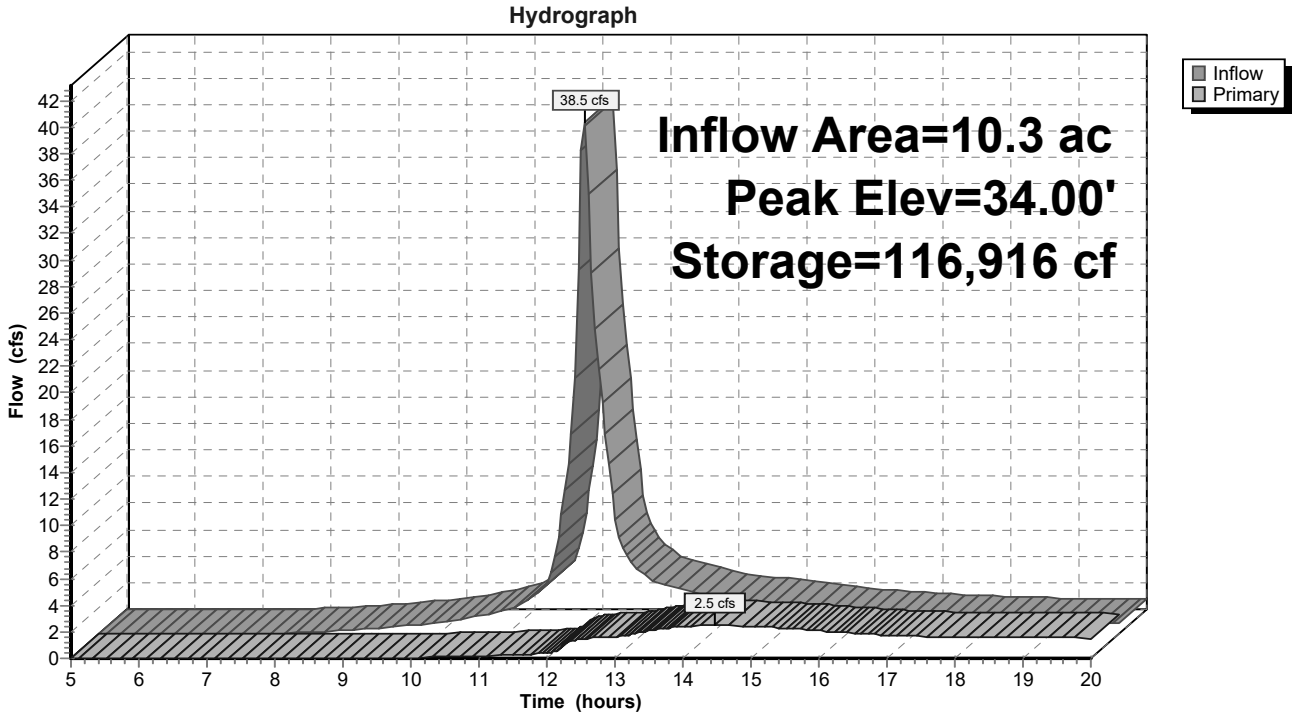
Volume	Invert	Avail.Storage	Storage Description
#1	29.00'	143,189 cf	**Custom Stage Data (Irregular)** Listed below

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
29.00	14,700	950.0	0	0	14,700
30.00	18,250	1,005.0	16,443	16,443	23,312
31.00	21,300	1,220.0	19,755	36,198	61,396
32.00	25,150	1,250.0	23,198	59,397	67,421
33.00	28,800	1,260.0	26,954	86,351	69,780
34.00	32,550	1,285.0	30,656	117,007	75,000
34.80	32,905	1,295.0	26,182	143,189	77,298

Device	Routing	Invert	Outlet Devices
#1	Primary	29.18'	**24.0" Round Culvert** L= 35.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 29.18' / 29.00' S= 0.0051 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf
#2	Device 1	30.80'	**4.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#3	Device 1	32.50'	**4.0" Vert. Orifice/Grate X 2.00** C= 0.600 Limited to weir flow at low heads
#4	Device 1	33.83'	**4.0' long x 0.5' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#5	Device 1	34.50'	**2.0" x 2.0" Horiz. Orifice/Grate X 8.00 columns** X 8 rows C= 0.600 in 24.0" x 24.0" Grate (44% open area) Limited to weir flow at low heads

Primary OutFlow Max=2.5 cfs @ 14.46 hrs HW=34.00' (Free Discharge)
- **1=Culvert** (Passes 2.5 cfs of 23.3 cfs potential flow)
  - **2=Orifice/Grate** (Orifice Controls 0.7 cfs @ 8.38 fps)
  - **3=Orifice/Grate** (Orifice Controls 1.0 cfs @ 5.55 fps)
  - **4=Broad-Crested Rectangular Weir** (Weir Controls 0.8 cfs @ 1.14 fps)
  - **5=Orifice/Grate** (Controls 0.0 cfs)

Pond P1: Resized Detention Basin w/ new OCS



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentA1: Flow to resized Runoff Area=450,410 sf 49.60% Impervious Runoff Depth>5.54"
Flow Length=690' Tc=9.6 min CN=77 Runoff=61.7 cfs 4.777 af

SubcatchmentA2: Uncontrolled flow Runoff Area=38,860 sf 0.00% Impervious Runoff Depth>1.89"
Flow Length=103' Tc=8.7 min CN=45 Runoff=1.7 cfs 0.141 af

Reach A: Design Point A: Flow To Wetland Inflow=18.0 cfs 3.110 af
Outflow=18.0 cfs 3.110 af

Pond P1: Resized Detention Basin w/ new Peak Elev=34.76' Storage=141,808 cf Inflow=61.7 cfs 4.777 af
Outflow=17.3 cfs 2.969 af

Total Runoff Area = 11.2 ac Runoff Volume = 4.917 af Average Runoff Depth = 5.25"
54.34% Pervious = 6.1 ac 45.66% Impervious = 5.1 ac

Summary for Subcatchment A1: Flow to resized Detention Basin

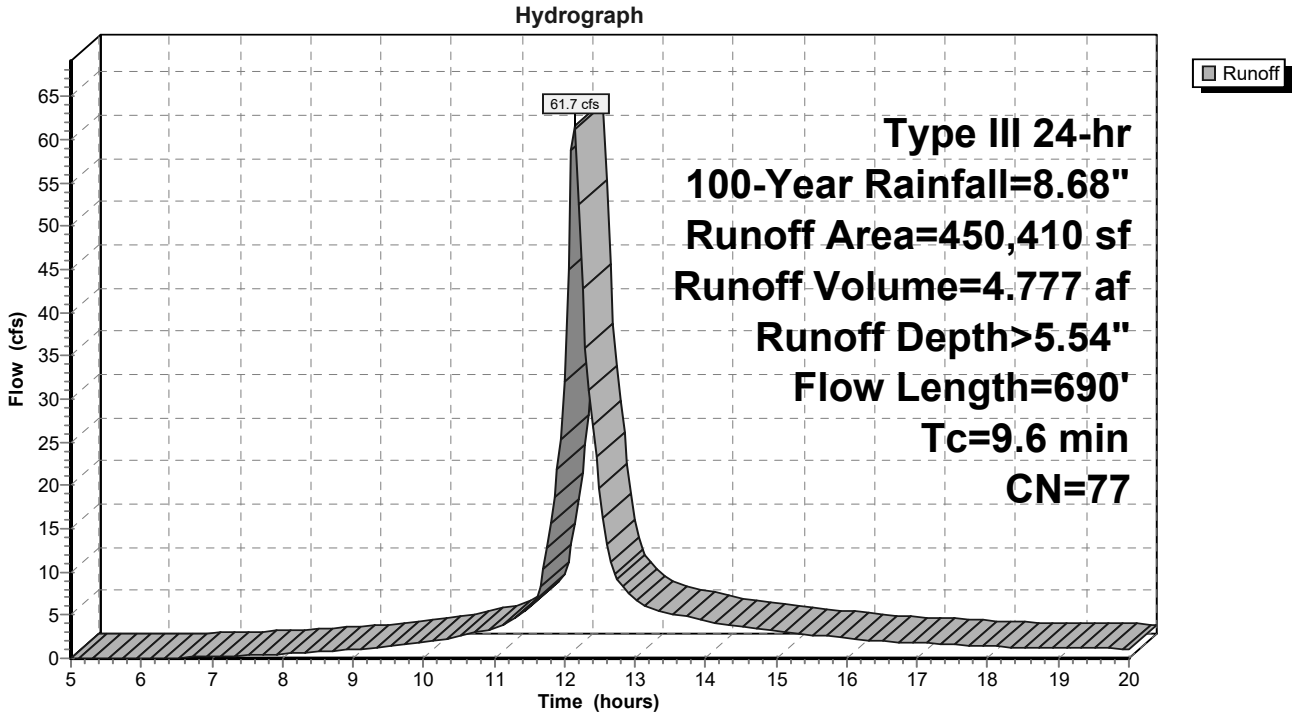
Runoff = 61.7 cfs @ 12.14 hrs, Volume= 4.777 af, Depth> 5.54"
Routed to Pond P1 : Resized Detention Basin w/ new OCS

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=8.68"

	Area (sf)	CN	Description
*	7,841	98	paved sidewalks, HSG A
	47,778	45	Woods, Poor, HSG A
	177,900	98	Paved parking, HSG A
	79,926	39	>75% Grass cover, Good, HSG A
	32,875	98	Water Surface, HSG A
*	94,298	76	Gravel areas, HSG A
*	4,792	98	Conc. slab areas/Shed roofs, HSG A
*	5,000	76	Field Transmission Material Storage area, HSG A
	450,410	77	Weighted Average
	227,002		50.40% Pervious Area
	223,408		49.60% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3	50	0.0400	0.20		**Sheet Flow, A-B** Grass: Short n= 0.150 P2= 3.20"
5.3	640	0.0100	2.03		**Shallow Concentrated Flow, B-C** Paved Kv= 20.3 fps
9.6	690	Total			

Subcatchment A1: Flow to resized Detention Basin



Summary for Subcatchment A2: Uncontrolled flow

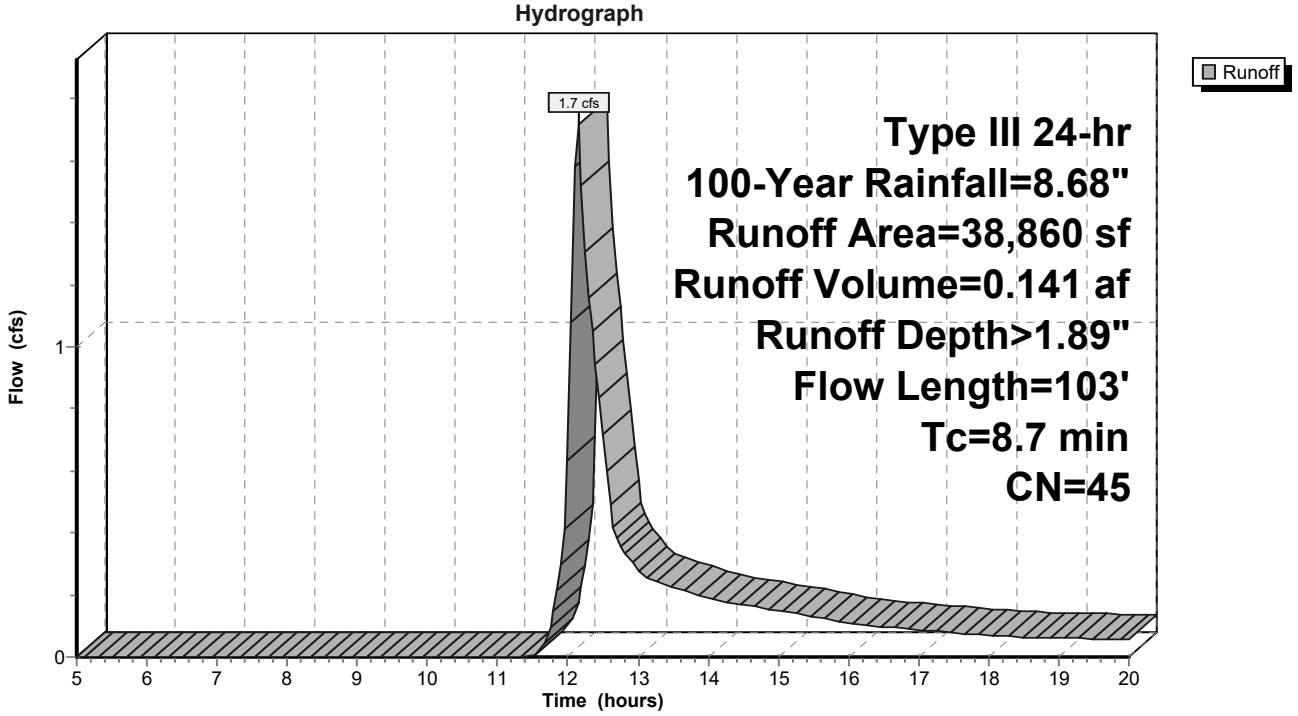
Runoff = 1.7 cfs @ 12.15 hrs, Volume= 0.141 af, Depth> 1.89"
Routed to Reach A : Design Point A: Flow To Wetland

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=8.68"

Area (sf)	CN	Description
38,860	45	Woods, Poor, HSG A
38,860		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.5	50	0.0500	0.10		**Sheet Flow, A-B** Woods: Light underbrush n= 0.400 P2= 3.20"
0.2	53	0.0900	4.83		**Shallow Concentrated Flow, B-C** Unpaved Kv= 16.1 fps
8.7	103	Total			

Subcatchment A2: Uncontrolled flow



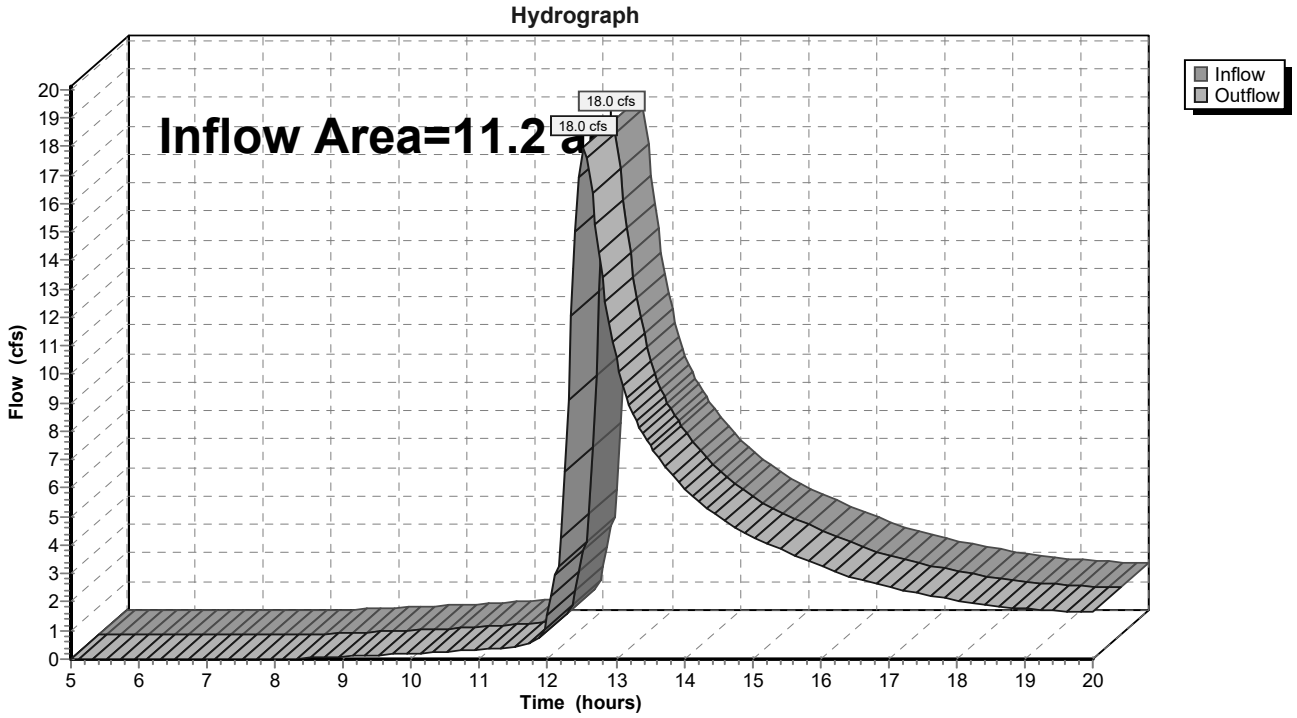
Summary for Reach A: Design Point A: Flow To Wetland

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 11.2 ac, 45.66% Impervious, Inflow Depth > 3.32" for 100-Year event
Inflow = 18.0 cfs @ 12.53 hrs, Volume= 3.110 af
Outflow = 18.0 cfs @ 12.53 hrs, Volume= 3.110 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach A: Design Point A: Flow To Wetland



Summary for Pond P1: Resized Detention Basin w/ new OCS

Inflow Area = 10.3 ac, 49.60% Impervious, Inflow Depth > 5.54" for 100-Year event
Inflow = 61.7 cfs @ 12.14 hrs, Volume= 4.777 af
Outflow = 17.3 cfs @ 12.54 hrs, Volume= 2.969 af, Atten= 72%, Lag= 24.1 min
Primary = 17.3 cfs @ 12.54 hrs, Volume= 2.969 af
Routed to Reach A : Design Point A: Flow To Wetland

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Starting Elev= 30.80' Surf.Area= 20,690 sf Storage= 32,247 cf
Peak Elev= 34.76' @ 12.54 hrs Surf.Area= 32,886 sf Storage= 141,808 cf (109,560 cf above start)

Plug-Flow detention time= 221.3 min calculated for 2.222 af (47% of inflow)
Center-of-Mass det. time= 93.1 min (870.3 - 777.2)

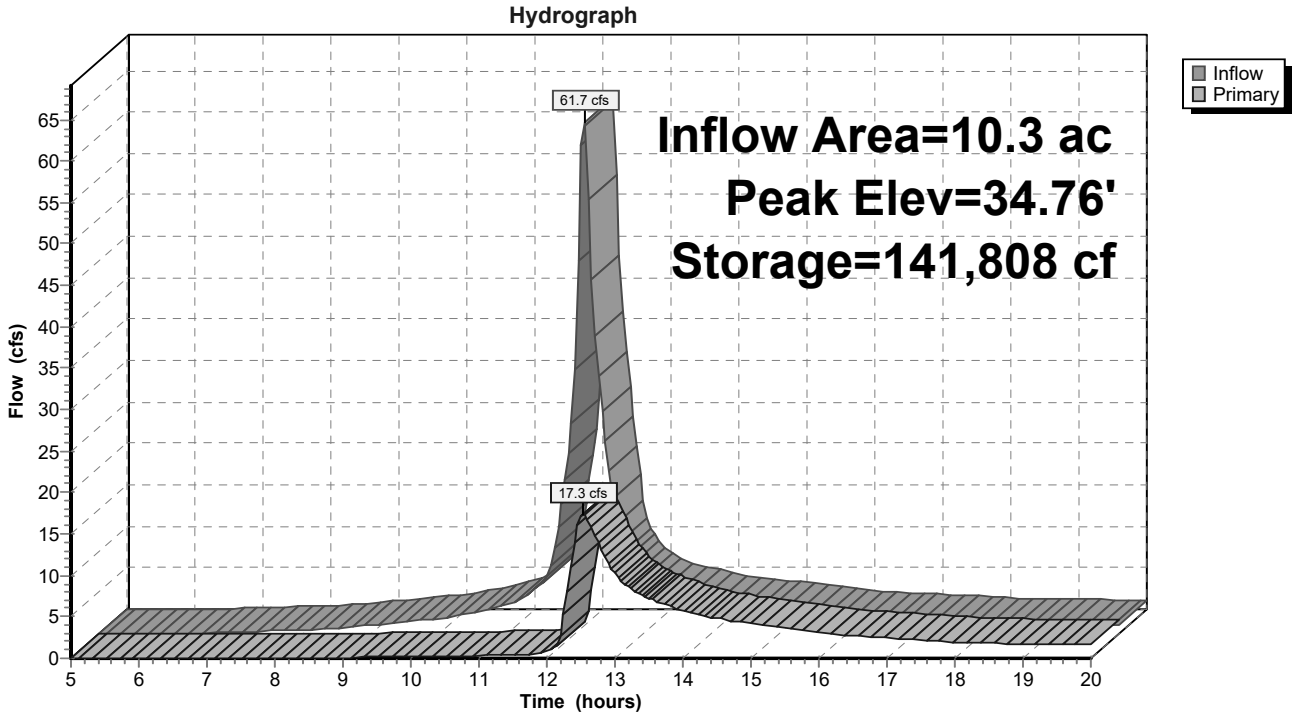
Volume	Invert	Avail.Storage	Storage Description
#1	29.00'	143,189 cf	**Custom Stage Data (Irregular)** Listed below

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
29.00	14,700	950.0	0	0	14,700
30.00	18,250	1,005.0	16,443	16,443	23,312
31.00	21,300	1,220.0	19,755	36,198	61,396
32.00	25,150	1,250.0	23,198	59,397	67,421
33.00	28,800	1,260.0	26,954	86,351	69,780
34.00	32,550	1,285.0	30,656	117,007	75,000
34.80	32,905	1,295.0	26,182	143,189	77,298

Device	Routing	Invert	Outlet Devices
#1	Primary	29.18'	**24.0" Round Culvert** L= 35.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 29.18' / 29.00' S= 0.0051 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf
#2	Device 1	30.80'	**4.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads
#3	Device 1	32.50'	**4.0" Vert. Orifice/Grate X 2.00** C= 0.600 Limited to weir flow at low heads
#4	Device 1	33.83'	**4.0' long x 0.5' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#5	Device 1	34.50'	**2.0" x 2.0" Horiz. Orifice/Grate X 8.00 columns** X 8 rows C= 0.600 in 24.0" x 24.0" Grate (44% open area) Limited to weir flow at low heads

Primary OutFlow Max=17.2 cfs @ 12.54 hrs HW=34.76' (Free Discharge)
└**1=Culvert** (Passes 17.2 cfs of 25.5 cfs potential flow)
  ├**2=Orifice/Grate** (Orifice Controls 0.8 cfs @ 9.37 fps)
  ├**3=Orifice/Grate** (Orifice Controls 1.2 cfs @ 6.96 fps)
  ├**4=Broad-Crested Rectangular Weir** (Weir Controls 11.8 cfs @ 3.19 fps)
  └**5=Orifice/Grate** (Weir Controls 3.4 cfs @ 1.66 fps)

Pond P1: Resized Detention Basin w/ new OCS



TSS Calculations

INSTRUCTIONS:

Version 1, Automated: Mar. 4, 2008

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Location: BMP Treatment Train 1

TSS Removal Calculation Worksheet

B BMP[1]	C TSS Removal Rate[1]	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
Deep Sump and Hooded Catch Basin	**0.25**	**1.00**	**0.25**	**0.75**
Proprietary WQU (STC 900)	**0.60**	**0.75**	**0.45**	**0.30**
Wet Basin (w/Sediment Forebay)	**0.80**	**0.30**	**0.24**	**0.06**
	0.00	**0.06**	**0.00**	**0.06**
	0.00	**0.06**	**0.00**	**0.06**

Total TSS Removal = **94%**

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project: Eversource Training Facility

Prepared By: **CJV**

Date: **3/8/2024**

*Equals remaining load from previous BMP (E) which enters the BMP

INSTRUCTIONS:

Version 1, Automated: Mar. 4, 2008

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Location: BMP Treatment Train 2

TSS Removal Calculation Worksheet

B BMP[1]	C TSS Removal Rate[1]	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
Proprietary WQU (STC 450i)	**0.60**	**1.00**	**0.60**	**0.40**
Wet Basin (w/Sediment Forebay)	**0.80**	**0.40**	**0.32**	**0.08**
	0.00	**0.08**	**0.00**	**0.08**
	0.00	**0.08**	**0.00**	**0.08**
	0.00	**0.08**	**0.00**	**0.08**

Total TSS Removal = **92%**

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project: Eversource Training Facility

Prepared By: **CJV**

Date: **3/8/2024**

*Equals remaining load from previous BMP (E) which enters the BMP

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MASTEP Technology Review

Technology Name: Stormceptor 450i.

Studies Reviewed: Multi-Phase Physical Model Testing of a Stormceptor STC450i

Date: March 14, 2009

Reviewers: Jerry Schoen

Rating: 2

Brief rationale for rating:

This laboratory study is generally well conducted and documented. No documentation of a quality assurance project, plan but quality control data was reported. Sediment analysis was done by the SSC method, but not the TSS method. Although SSC is considered by many scientists to be the preferred method, it is at odds with Massachusetts stormwater regulations, which are based on TSS treatment. Comparing SSC and TSS results is considered an inexact science.

TARP Requirements Not Met*:

- No documentation of a Quality Assurance Project Plan
- TSS analysis was not performed.

Other Comments

- SSC removal efficiency, calculated according to the NJDEP weighted formula, was 59.5 – 63.6%.
- SSC removal evaluated using event mean concentration and modified mass balance method, the latter considered to be a particularly accurate method of evaluating sediment removal in a laboratory setting.
- Particle Size Distribution (with d50 of 67 microns) closely matched the 55% sand, 40% silt, 5% clay mix recommended by NJDEP.
- A full range of flows (2% - 125%) was tested.
- Scour test was performed at 500% of design flow. This is more rigorous than the 125% recommended for scour tests. Effluent concentrations for the scour tests ranged from 5.9 – 6.1mg/l, not considered a significant level of scour.

* Laboratory testing was based on the NJDEP TARP laboratory testing guidelines.

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MASTEP Technology Review

Technology Name: Stormceptor

Studies Reviewed: Final NJCAT Technology Verification Stormceptor STC900 September 2004; Coventry University Study, 1996; Technology Assessment, University of Massachusetts, 1997; SeaTac Stormceptor Performance report 2001; SWAMP report Ontario 2004; Phoenix Group Edmonton report 1995; Stormceptor 1200 Field Evaluation report 2004; Applied Hydrology Associates Denver report 2003; Rinker Materials Como Park St. Paul MN report 2002; VA DOT / UVA "Testing of Ultra-Urban Stormwater Best Management Practices" report 2001.
Hydrodynamic Separator Sediment Retention Testing, Mohseni, 2010.

Date: September 17, 2013

Reviewer: Jerry Schoen

Rating: 2

Brief rationale for rating: This rating is primarily based on the 2005 NJCAT Technology Verification study. In general, this was a well-conducted test, which in large part followed NJDEP test guidelines for laboratory studies, which MASTEP considers as the laboratory equivalent of TARP field protocols. Issues of concern: the study measured suspended sediment concentration (SSC) rather than total suspended solids (TSS). Although SSC is considered by many scientists to be the preferred method, it is at odds with Massachusetts stormwater regulations, which are based on TSS treatment. Comparing SSC and TSS results is considered an inexact science. The test was conducted with higher influent sediment concentrations than is preferred, but results were fairly consistent across all ranges studied. The particle size distribution also appears to be slightly higher than the target test range. There are additional field studies that in general support the results obtained in this laboratory studies. These studies do not satisfy TARP protocols, but they do not contradict results obtained in the NJCAT study.

TARP Requirements Not Met*:

- Measurements in TSS.
- Influent sediment concentration is 100 – 300 mg/l: actual was 153-460.
- No documentation of a Quality Assurance Project Plan
- Third party studies are preferred. This was conducted by Stormceptor personnel, with sample analyses conducted by an external laboratory.

Other Comments:
* The 2010 Mohseni study evaluates the susceptibility of the Stormceptor to scouring, or washout of collected sediments. Report concluded that the unit does not scour at high flows as long as sediment depth does not exceed maintenance level.

* Criteria also based on NJDEP laboratory testing guidelines.

Phosphorus Removal Calculations

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu (values obtained from Massachusetts Stormwater Handbook)
3. After BMP is selected, Phosphorus Removal and other Columns are automatically completed.

Location: 37 Doty Street Wareham MA

Phosphorus Removal Calculation Worksheet

B BMP[1]	C Phosphorus Removal Rate[1]	D Starting Phosphorus Load*	E Amount Phosphorus Removed (C*D)	F Remaining Phosphorus Load (D-E)
Non-Use	**0.33**	**1.00**	**0.33**	**0.67**
Wet Basin (w/Sediment Forebay)	**0.60**	**0.67**	**0.40**	**0.27**
	0.00	**0.27**	**0.00**	**0.27**
	0.00	**0.27**	**0.00**	**0.27**
	0.00	**0.27**	**0.00**	**0.27**

Total Phosphorus Removal = **73%**

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project: Eversource Training Facilty

Prepared By: **CJV**

Date: **3/7/2024**

*Equals remaining load from previous BMP (E) which enters the BMP

Water Quality Volume Calculations

Water Quality Volume Flow Rate Calculations

Project Name:	Eversource	Date:	3/25/2024
Project Location:	37 Doty Street Wareham MA	Calculated By:	CJV
Project Number:	323-322	Checked By:	BEP

Structure Name: P1
Subcatchment: A1

Description: Wet Basin

Total Drainage Area:	450,410	sq ft
	10.34	ac
Total Impervious Area:	223,408	sq ft
	5.13	ac

** Roof Areas are considered clean and are not subject to WQV calculation*

Runoff Depth to be Treated: 1.0 inches

Required Water Quality Volume:	18,617	cf
	0.43	ac ft

Provided Water Quality Volume

Bottom of basin:	14700	sq ft
Low Flow Outlet Elevation:	30.8	ft
**Water Quality Volume Provided:	32,247	cu ft

Provided Water Quality Volume:	32,247	cf
	0.74	ac ft

(**See attached documentation.)

Water Quality Volume Flow Rate Calculations

Project Name:	Eversource Training Facility	Date:	3/20/2024
Project Location:	Wareham MA	Calculated By:	CJV
Project Number:	323-322	Checked By:	BEP

Stormwater BMP: STC 450i Description: Flow to Stormceptor STC 450i Unit (WQU-5)

Total Drainage Area:	25,000	sq ft
	0.57	ac
Total Impervious Area:	20,000	sq ft
	0.46	ac

** Roof Areas are considered clean and are not subject to WQV calculation*

Runoff Depth to be Treated: 1.0 inches

Required Water Quality Volume:	1,667	cf
	0.038	ac ft

FLOW RATE CONVERSION

$Q = (qu)(A)(WQV)$

Where:

Q = flow rate associated with the depth of runoff, in cfs
qu = the unit peak discharge, in csm/in.
A = impervious surface drainage area, in square miles
WQV = water quality volume in watershed inches

Given:

1-acre =	0.00156	mi^2
5 minute =	0.083	hours
qu (**1/2**-inch) =	**773**	csm/in

Calculation:

qu=	**773**	
A=	0.46	ac
WQV=	1.0	in

Required Water Quality Flow Rate:	**0.55**	**cfs**

The Stormceptor STC 450i will provide 89% TSS Removal Efficiency for flows up to 0.55 cfs

(Based on Manufacturer's sizing. See attached documentation in Appendix D.)

* Flow rate conversion based on the Massachusetts Department of Environmental Protection Wetlands Program - Standard Method to Convert Required Water Quality Volume to a Discharge Rate for Sizing Flow Based Manufactured Proprietary Stormwater Treatment Practices

Stage-Area-Storage for Pond P1: Resized Detention Basin w/ new OCS

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
29.00	14,700	0
29.10	15,055	1,644
29.20	15,410	3,289
29.30	15,765	4,933
29.40	16,120	6,577
29.50	16,475	8,222
29.60	16,830	9,866
29.70	17,185	11,510
29.80	17,540	13,154
29.90	17,895	14,799
30.00	18,250	16,443
30.10	18,555	18,419
30.20	18,860	20,394
30.30	19,165	22,370
30.40	19,470	24,345
30.50	19,775	26,321
30.60	20,080	28,296
30.70	20,385	30,272
30.80	20,690	32,247
30.90	20,995	34,223
31.00	21,300	36,198
31.10	21,685	38,518
31.20	22,070	40,838
31.30	22,455	43,158
31.40	22,840	45,478
31.50	23,225	47,798
31.60	23,610	50,117
31.70	23,995	52,437
31.80	24,380	54,757
31.90	24,765	57,077
32.00	25,150	59,397
32.10	25,515	62,092
32.20	25,880	64,788
32.30	26,245	67,483
32.40	26,610	70,179
32.50	26,975	72,874
32.60	27,340	75,569
32.70	27,705	78,265
32.80	28,070	80,960
32.90	28,435	83,656
33.00	28,800	86,351
33.10	29,175	89,417
33.20	29,550	92,482
33.30	29,925	95,548
33.40	30,300	98,614
33.50	30,675	101,679
33.60	31,050	104,745
33.70	31,425	107,810
33.80	31,800	110,876
33.90	32,175	113,941
34.00	32,550	117,007
34.10	32,594	120,280
34.20	32,639	123,553
34.30	32,683	126,825
34.40	32,728	130,098
34.50	32,772	133,371
34.60	32,816	136,643
34.70	32,861	139,916
34.80	**32,905**	**143,189**

LOW FLOW ORIFICE ELEVATION

Water Quality Volume Flow Rate Calculations

Project Name:	Eversource Training Facility	Date:	3/20/2024
Project Location:	Wareham MA	Calculated By:	CJV
Project Number:	323-322	Checked By:	BEP

Stormwater BMP: STC 900 Description: Flow to Stormceptor STC 900 Units (WQU-1, WQU-2, WQU-3, WQU-4)

Total Drainage Area:	141,800	sq ft
	3.26	ac
Total Impervious Area:	56,845	sq ft
	1.30	ac

** Roof Areas are considered clean and are not subject to WQV calculation*

Runoff Depth to be Treated: 1.0 inches

Required Water Quality Volume:	4,737	cf
	0.109	ac ft

FLOW RATE CONVERSION

$$Q = (qu)(A)(WQV)$$

Where:

Q = flow rate associated with the depth of runoff, in cfs
qu = the unit peak discharge, in csm/in.

A = impervious surface drainage area, in square miles

WQV = water quality volume in watershed inches

Given:

1-acre =	0.00156	mi^2
5 minute =	0.083	hours
qu (**1/2**-inch) =	**773**	csm/in

Calculation:

qu=	**773**	
A=	1.30	ac
WQV=	1.0	in

Required Water Quality Flow Rate:	**1.58**	**cfs**

The Stormceptor STC 900 will provide 87% TSS Removal Efficiency for flows up to 1.58 cfs

(Based on Manufacturer's sizing. See attached documentation in Appendix D.)

* Flow rate conversion based on the Massachusetts Department of Environmental Protection Wetlands Program - Standard Method to Convert Required Water Quality Volume to a Discharge Rate for Sizing Flow Based Manufactured Proprietary Stormwater Treatment Practices

Drawdown Calculations

DRAWDOWN CALCULATIONS

Project Name:	Eversource Training Facility	Date:	3/20/2024
Project Location:	Wareham	Calculated By:	CJV
Project Number:	323-322	Checked By:	BEP
			1 of 1

Stormwater BMP: Wet Basin

Description: Wet Basin

Provided Permanent Pool Volume

Bottom of Basin:	29.00	ft	
Overflow Outlet Elevation:	30.80	ft	
*** Volume Provided:	0.740	ac ft	
	32247	cu ft	*** (See attached HydroCAD output)

Wet Basin

Total Provided Permanent Pool Volume:	**32,247**	**cu ft**

40-hour Drawdown Calculation (Per Massachusetts Stormwater Standards for Wet Basins)

Provided Permanent Pool Volume:	32,247	cu ft	
Saturated Hydraulic Conductivity:	1.02	in / hr*	*(A Rawls Rate for Sandy Loam (HSG C) was assumed
Bottom Area:	14,700	sq ft	
Drawdown Time:	**25.8**	**hours**	

Rip-Rap Outlet Sizing Calculations

Civil & Environmental Consultants, Inc.

Calculations For Outlet Protection

Project:	Eversource training Facility	**Date:**	3/20/2024
Prepared by:	CJV	**Job #:**	323-322
Revised by:		**Checked by:**	BEP

Flow into Sediment Forebay (Rip-Rap #1)

Note: To be conservative the flow rate into the basin (38.5 cfs) was divided by 3 since there are essetuially 3 separate discharge points in to the basin

Total Peak Flow (Q)	12.83	cfs
Total Diameter of Structure (Do)	1	ft
Invert Out	30.86	ft
Tailwater Elevation (at Peak Flow) in Basin	34	ft

Step 1: Calculate unit discharge (q):

q = Q/Do

q= 12.8

Step 2: Calculate Tw: Tailwater Elevation - Invert Out Elevation

3.14 ft

Step 3: Calculate La:

Determine which La formula to use

If TW < 1/2Do Then , La = 1.8(q/(Do^1.5)) + 7

If TW > 1/2Do Then , La = 3*Do(q/(Do^1.5))

Since TW > 1/2 Do:

La = 38 ft

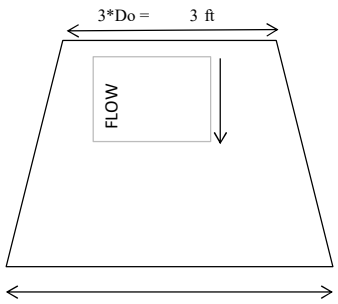
Step 3: Calculate W:

TW < 1/2Do W = 3*Do + La

TW > 1/2Do W = 3*Do + 0.4La

Since TW > 1/2 Do:

W = 18 ft



Note: due to the physical site constraints, the rip-rap oultet provided has be sized to maximum extent practicable

Step 4: Size Rip Rap:

d50 = (0.02/Tw) * q^1.33

d50 = 0.19 ft

2.28 in

d50 = 3 inches (min) *** Use 3" D50 min.**

Note: due to the physical site constraints from where the flared ends come into the sediment forebays, the size of the rip-rap outlets provided have be sized to maximum extent practicable given the site constraints (See Site plans)

Thickness (6-inch minimum) = d50 x 2

Thickness = 6 inches (min)

Notes:

1.) Equations and tables taken from "*Standards for Soil Erosion and Sediment Control in New Jersey*", Standard for Conduit Outlet Protection, dated May 2012, based upon design standards from U.S. Department of Transportation Federal Highway Administration.

2.) Flow and Tailwater taken from HydroCAD model for 25-Year, 24-Hour design storm.

Civil & Environmental Consultants, Inc.

Calculations For Outlet Protection

Project:	Eversource training Facility	**Date:**	3/20/2024
Prepared by:	CJV	**Job #:**	323-322
Revised by:		**Checked by:**	BEP

Flow into Sediment Forebay (Rip-Rap #2)

Note: To be conservative the flow rate into the basin (38.5 cfs) was divided by 3 since there are essetuially 3 separate discharge points in to the basin

Total Peak Flow (Q)	12.83	cfs
Total Diameter of Structure (Do)	1	ft
Invert Out	30.8	ft
Tailwater Elevation (at Peak Flow) in Basin	34	ft

Step 1: Calculate unit discharge (q):

q = Q/Do

q= 12.8

Step 2: Calculate Tw: Tailwater Elevation - Invert Out Elevation

3.20 ft

Step 3: Calculate La:

Determine which La formula to use

If TW < 1/2Do Then , La = 1.8(q/(Do^1.5)) + 7

If TW > 1/2Do Then , La = 3*Do(q/(Do^1.5))

Since TW > 1/2 Do:

La = 38 ft

Note: due to the physical site constraints from where the flared ends come into the sediment forebays, the size of the rip-rap outlets provided have be sized to maximum extent practicable given the site constraints (See Site plans)

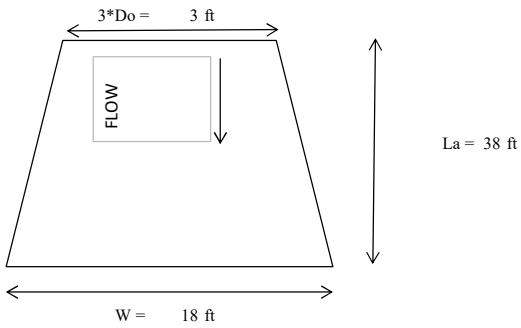
Step 3: Calculate W:

TW < 1/2Do W = 3*Do + La

TW > 1/2Do W = 3*Do + 0.4La

Since TW > 1/2 Do:

W = 18 ft



Step 4: Size Rip Rap:

d50 = (0.02/Tw) * q^1.33

d50 = 0.19 ft

2.23 in

d50 = 3 inches (min) *** Use 3" D50 min.**

Thickness (6-inch minimum) = d50 x 2

Thickness = 6 inches (min)

Notes:

1.) Equations and tables taken from "*Standards for Soil Erosion and Sediment Control in New Jersey"* , Standard for Conduit Outlet Protection, dated May 2012, based upon design standards from U.S. Department of Transportation Federal Highway Administration.

2.) Flow and Tailwater taken from HydroCAD model for 25-Year, 24-Hour design storm.

Civil & Environmental Consultants, Inc.

Calculations For Outlet Protection

Project:	Eversource Training Facility	**Date:**	3/20/2024
Prepared by:	CJV	**Job #:**	323-322
Revised by:		**Checked by:**	BEP

Flow into Sediment Forebay (Rip-Rap #3)

Note: To be conservative the flow rate into the basin (38.5 cfs) was divided by 3 since there are essetuially 3 separate discharge points in to the basin

Total Peak Flow (Q)	12.83	cfs
Total Diameter of Structure (Do)	1	ft
Invert Out	30.36	ft
Tailwater Elevation (at Peak Flow) in Basin	34	ft

Step 1: Calculate unit discharge (q):

q = Q/Do

q= 12.8

Step 2: Calculate Tw: Tailwater Elevation - Invert Out Elevation

3.64 ft

Step 3: Calculate La:

Determine which La formula to use

If TW < 1/2Do Then , La = 1.8(q/(Do^1.5)) + 7

If TW > 1/2Do Then , La = 3*Do(q/(Do^1.5))

Since TW > 1/2 Do:

La = 38 ft

Step 3: Calculate W:

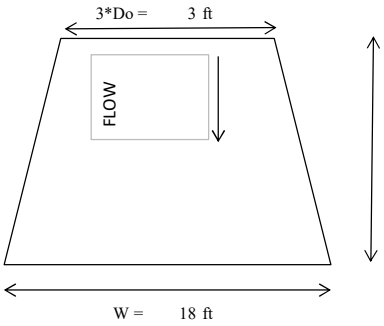
TW < 1/2Do W = 3*Do + La

TW > 1/2Do W = 3*Do + 0.4La

Since TW > 1/2 Do:

W = 18 ft

Note: due to the physical site constraints from where the flared ends come into the sediment forebays, the size of the rip-rap outlets provided have be sized to maximum extent practicable given the site constraints (See Site plans)



Step 4: Size Rip Rap:

d50 = (0.02/Tw) * q^1.33

d50 = 0.16 ft

1.96 in

d50 = 2 inches (min) *** Use 3" D50 min.**

Thickness (6-inch minimum) = d50 x 2

Thickness = 6 inches (min)

Notes:

1.) Equations and tables taken from "*Standards for Soil Erosion and Sediment Control in New Jersey*", Standard for Conduit Outlet Protection, dated May 2012, based upon design standards from U.S. Department of Transportation Federal Highway Administration.

2.) Flow and Tailwater taken from HydroCAD model for 25-Year, 24-Hour design storm.

Stormceptor Sizing Calculations

Detailed Stormceptor Sizing Report – Flow to WQU-1,WQU-2, WQU-3, WQU-4

Project Information & Location			
Project Name	Eversource training facility	**Project Number**	50259
City	Wareham	**State/ Province**	Massachusetts
Country	United States of America	**Date**	3/7/2024
Designer Information		**EOR Information (optional)**	
Name	chris Vandenberghe	**Name**	
Company	CEC	**Company**	
Phone #	617-416-1964	**Phone #**	
Email	cvandenberghe@cecinc.com	**Email**	

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	Flow to WQU-1,WQU-2, WQU-3, WQU-4
Recommended Stormceptor Model	STC 900
Target TSS Removal (%)	80.0
TSS Removal (%) Provided	87
PSD	OK-110
Rainfall Station	HYANNIS

The recommended Stormceptor model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary	
Stormceptor Model	**% TSS Removal Provided**
STC 450i	77
STC 900	87
STC 1200	87
STC 1800	87
STC 2400	90
STC 3600	91
STC 4800	93
STC 6000	93
STC 7200	95
STC 11000	97
STC 13000	97
STC 16000	97

Stormceptor
The Stormceptor oil and sediment separator is sized to treat stormwater runoff by removing pollutants through gravity separation and flotation. Stormceptor's patented design generates positive TSS removal for each rainfall event, including large storms. Significant levels of pollutants such as heavy metals, free oils and nutrients are prevented from entering natural water resources and the re-suspension of previously captured sediment (scour) does not occur.
Stormceptor provides a high level of TSS removal for small frequent storm events that represent the majority of annual rainfall volume and pollutant load. Positive treatment continues for large infrequent events, however, such events have little impact on the average annual TSS removal as they represent a small percentage of the total runoff volume and pollutant load.

Design Methodology
Stormceptor is sized using PCSWMM for Stormceptor, a continuous simulation model based on US EPA SWMM. The program calculates hydrology using local historical rainfall data and specified site parameters. With US EPA SWMM's precision, every Stormceptor unit is designed to achieve a defined water quality objective. The TSS removal data presented follows US EPA guidelines to reduce the average annual TSS load. The Stormceptor's unit process for TSS removal is settling. The settling model calculates TSS removal by analyzing:
• Site parameters
• Continuous historical rainfall data, including duration, distribution, peaks & inter-event dry periods
• Particle size distribution, and associated settling velocities (Stokes Law, corrected for drag)
• TSS load
• Detention time of the system

Hydrology Analysis
PCSWMM for Stormceptor calculates annual hydrology with the US EPA SWMM and local continuous historical rainfall data. Performance calculations of Stormceptor are based on the average annual removal of TSS for the selected site parameters. The Stormceptor is engineered to capture sediment particles by treating the required average annual runoff volume, ensuring positive removal efficiency is maintained during each rainfall event, and preventing negative removal efficiency (scour). Smaller recurring storms account for the majority of rainfall events and average annual runoff volume, as observed in the historical rainfall data analyses presented in this section.

Rainfall Station			
State/Province	Massachusetts	**Total Number of Rainfall Events**	1268
Rainfall Station Name	HYANNIS	**Total Rainfall (in)**	531.6
Station ID #	3821	**Average Annual Rainfall (in)**	33.2
Coordinates	41°24'0"N, 70°10'47"W	**Total Evaporation (in)**	12.4
Elevation (ft)	50	**Total Infiltration (in)**	316.3
Years of Rainfall Data	14	**Total Rainfall that is Runoff (in)**	202.9

Notes
• Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules. • Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed. • For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

Drainage Area	
Total Area (acres)	3.26
Imperviousness %	40.0

Up Stream Storage	
Storage (ac-ft)	Discharge (cfs)
0.000	0.000

Water Quality Objective	
TSS Removal (%)	80.0
Runoff Volume Capture (%)	
Oil Spill Capture Volume (Gal)	
Peak Conveyed Flow Rate (CFS)	
Water Quality Flow Rate (CFS)	1.58

Up Stream Flow Diversion	
Max. Flow to Stormceptor (cfs)	

Design Details	
Stormceptor Inlet Invert Elev (ft)	
Stormceptor Outlet Invert Elev (ft)	
Stormceptor Rim Elev (ft)	
Normal Water Level Elevation (ft)	
Pipe Diameter (in)	
Pipe Material	
Multiple Inlets (Y/N)	No
Grate Inlet (Y/N)	No

Particle Size Distribution (PSD)

Removing the smallest fraction of particulates from runoff ensures the majority of pollutants, such as metals, hydrocarbons and nutrients are captured. The table below identifies the Particle Size Distribution (PSD) that was selected to define TSS removal for the Stormceptor design.

OK-110		
Particle Diameter (microns)	Distribution %	Specific Gravity
1.0	0.0	2.65
53.0	3.0	2.65
75.0	15.0	2.65
88.0	25.0	2.65
106.0	41.0	2.65
125.0	15.0	2.65
150.0	1.0	2.65
212.0	0.0	2.65

Site Name	Flow to WQU-1,WQU-2, WQU-3, WQU-4

Site Details

Drainage Area

Total Area (acres)	3.26
Imperviousness %	40.0

Surface Characteristics

Width (ft)	754.00
Slope %	2
Impervious Depression Storage (in)	0.02
Pervious Depression Storage (in)	0.2
Impervious Manning's n	0.015
Pervious Manning's n	0.25

Maintenance Frequency

Maintenance Frequency (months) >	12

Infiltration Parameters

Horton's equation is used to estimate infiltration

Max. Infiltration Rate (in/hr)	2.44
Min. Infiltration Rate (in/hr)	0.4
Decay Rate (1/sec)	0.00055
Regeneration Rate (1/sec)	0.01

Evaporation

Daily Evaporation Rate (in/day)	0.1

Dry Weather Flow

Dry Weather Flow (cfs)	0

Winter Months

Winter Infiltration	0

TSS Loading Parameters

TSS Loading Function	

Buildup/Wash-off Parameters

Target Event Mean Conc. (EMC) mg/L	
Exponential Buildup Power	
Exponential Washoff Exponent	

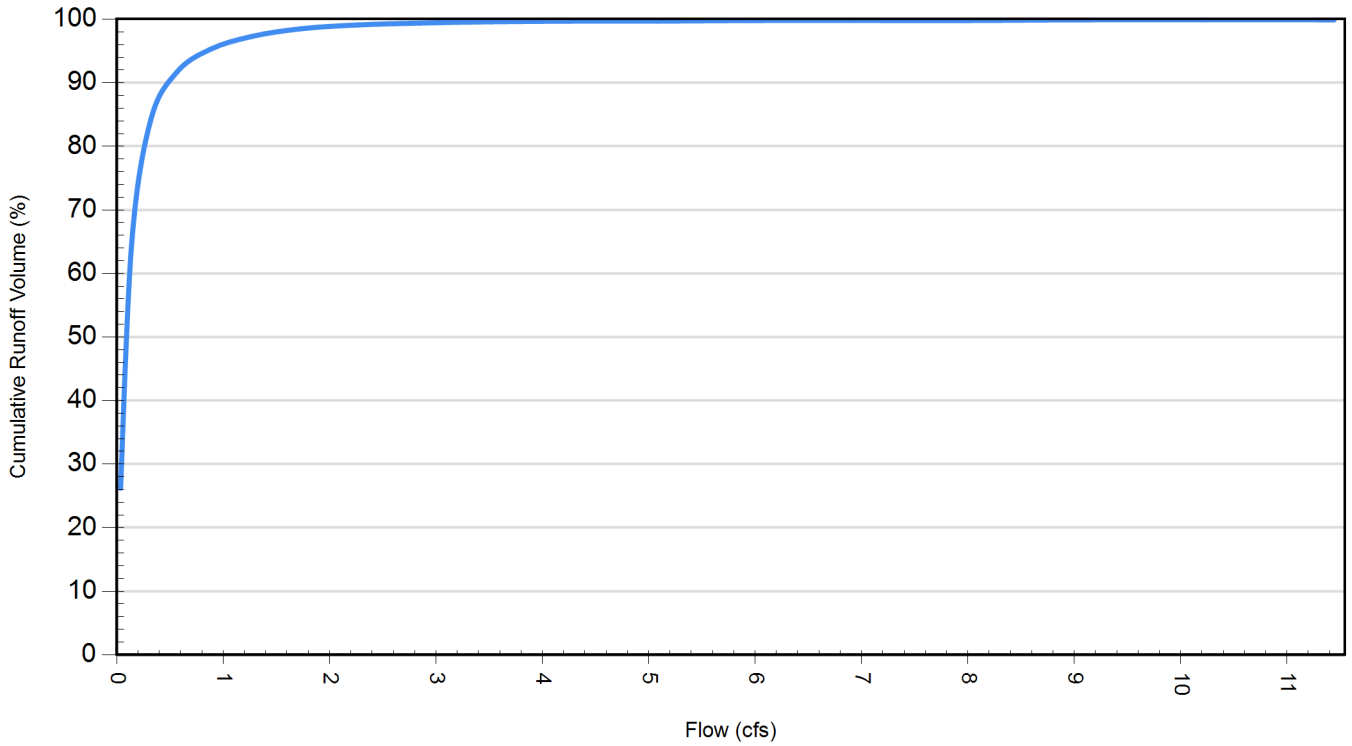
TSS Availability Parameters

Availability Constant A	
Availability Factor B	
Availability Exponent C	
Min. Particle Size Affected by Availability (micron)	

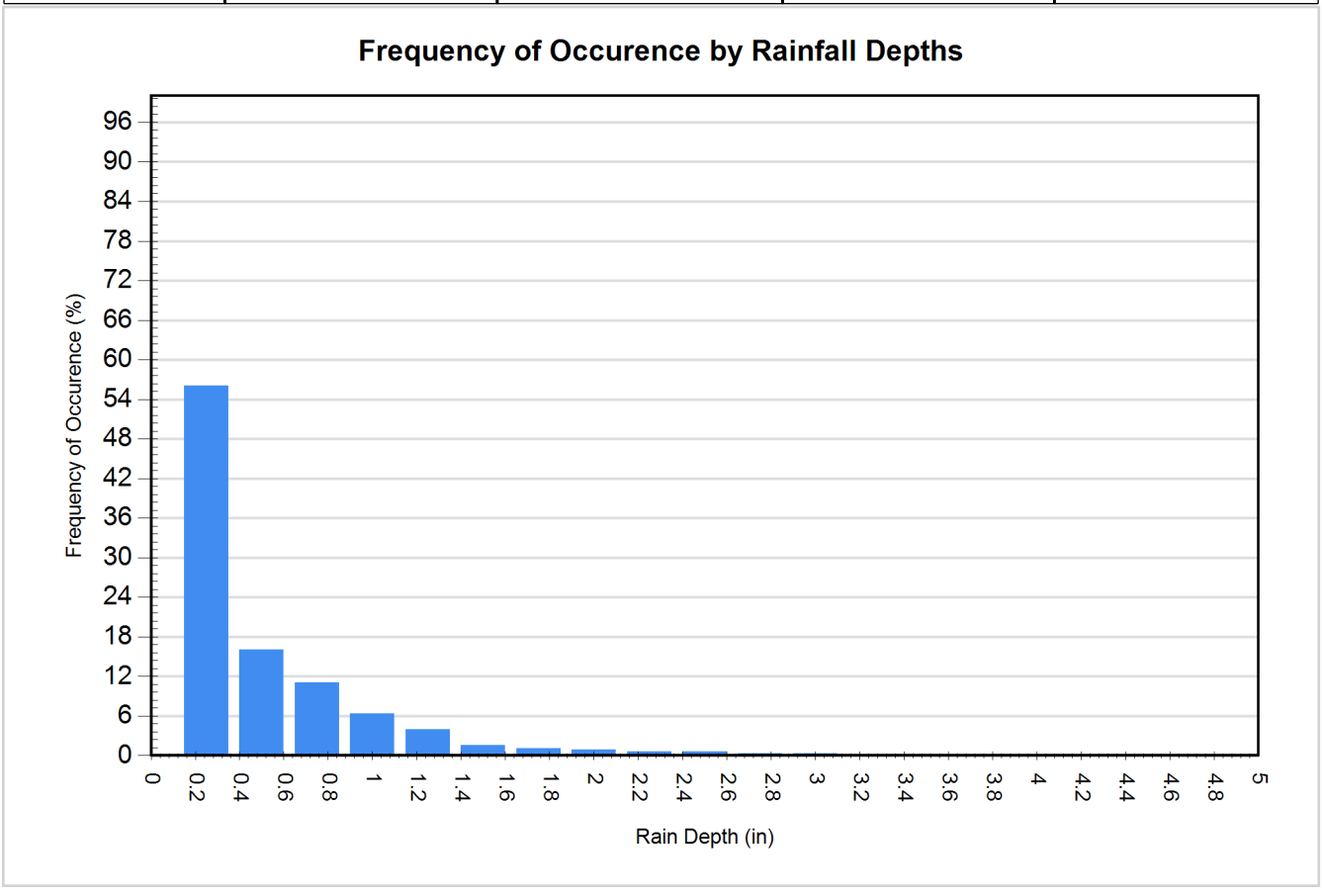
Cumulative Runoff Volume by Runoff Rate			
Runoff Rate (cfs)	**Runoff Volume (ft³)**	**Volume Over (ft³)**	**Cumulative Runoff Volume (%)**
0.035	632012	1782573	26.2
0.141	1570563	844059	65.0
0.318	2029661	384908	84.1
0.565	2214396	200159	91.7
0.883	2301363	113188	95.3
1.271	2349733	64816	97.3
1.730	2377857	36693	98.5
2.260	2393139	21413	99.1
2.860	2400315	14241	99.4
3.531	2404488	10069	99.6
4.273	2406483	8074	99.7
5.085	2407701	6856	99.7
5.968	2408783	5774	99.8
6.922	2409791	4767	99.8
7.946	2410714	3846	99.8
9.041	2411699	2860	99.9
10.206	2412477	2082	99.9
11.442	2413219	1340	99.9

Cumulative Runoff Volume by Runoff Rate

For area: 3.26(ac), imperviousness: 40.0%, rainfall station: HYANNIS



Rainfall Event Analysis				
Rainfall Depth (in)	No. of Events	Percentage of Total Events (%)	Total Volume (in)	Percentage of Annual Volume (%)
0.25	711	56.1	71	13.4
0.50	204	16.1	74	14.0
0.75	141	11.1	88	16.5
1.00	81	6.4	72	13.5
1.25	51	4.0	57	10.7
1.50	20	1.6	28	5.2
1.75	14	1.1	23	4.3
2.00	12	0.9	22	4.2
2.25	7	0.6	15	2.8
2.50	7	0.6	17	3.2
2.75	4	0.3	11	2.0
3.00	4	0.3	12	2.2
3.25	3	0.2	9	1.8
3.50	2	0.2	7	1.3
3.75	2	0.2	7	1.3
4.00	3	0.2	12	2.2
4.25	2	0.2	8	1.6
4.50	0	0.0	0	0.0
4.75	0	0.0	0	0.0



Detailed Stormceptor Sizing Report – Flow to WQU-5

Project Information & Location			
Project Name	Eversource training facility	**Project Number**	50259
City	Wareham	**State/ Province**	Massachusetts
Country	United States of America	**Date**	3/7/2024
Designer Information		**EOR Information (optional)**	
Name	chris Vandenberghe	**Name**	
Company	CEC	**Company**	
Phone #	617-416-1964	**Phone #**	
Email	cvandenberghe@cecinc.com	**Email**	

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	Flow to WQU-5
Recommended Stormceptor Model	STC 450i
Target TSS Removal (%)	80.0
TSS Removal (%) Provided	89
PSD	OK-110
Rainfall Station	HYANNIS

The recommended Stormceptor model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary	
Stormceptor Model	**% TSS Removal Provided**
STC 450i	89
STC 900	94
STC 1200	94
STC 1800	94
STC 2400	96
STC 3600	96
STC 4800	97
STC 6000	97
STC 7200	98
STC 11000	99
STC 13000	99
STC 16000	99

Stormceptor
The Stormceptor oil and sediment separator is sized to treat stormwater runoff by removing pollutants through gravity separation and flotation. Stormceptor's patented design generates positive TSS removal for each rainfall event, including large storms. Significant levels of pollutants such as heavy metals, free oils and nutrients are prevented from entering natural water resources and the re-suspension of previously captured sediment (scour) does not occur.
Stormceptor provides a high level of TSS removal for small frequent storm events that represent the majority of annual rainfall volume and pollutant load. Positive treatment continues for large infrequent events, however, such events have little impact on the average annual TSS removal as they represent a small percentage of the total runoff volume and pollutant load.

Design Methodology
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• Site parameters
• Continuous historical rainfall data, including duration, distribution, peaks & inter-event dry periods
• Particle size distribution, and associated settling velocities (Stokes Law, corrected for drag)
• TSS load
• Detention time of the system

Hydrology Analysis
PCSWMM for Stormceptor calculates annual hydrology with the US EPA SWMM and local continuous historical rainfall data. Performance calculations of Stormceptor are based on the average annual removal of TSS for the selected site parameters. The Stormceptor is engineered to capture sediment particles by treating the required average annual runoff volume, ensuring positive removal efficiency is maintained during each rainfall event, and preventing negative removal efficiency (scour). Smaller recurring storms account for the majority of rainfall events and average annual runoff volume, as observed in the historical rainfall data analyses presented in this section.

Rainfall Station			
State/Province	Massachusetts	**Total Number of Rainfall Events**	1268
Rainfall Station Name	HYANNIS	**Total Rainfall (in)**	531.6
Station ID #	3821	**Average Annual Rainfall (in)**	33.2
Coordinates	41°24'0"N, 70°10'47"W	**Total Evaporation (in)**	24.2
Elevation (ft)	50	**Total Infiltration (in)**	110.4
Years of Rainfall Data	14	**Total Rainfall that is Runoff (in)**	397.0

Notes
• Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules. • Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed. • For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

Drainage Area	
Total Area (acres)	0.57
Imperviousness %	79.0

Up Stream Storage	
Storage (ac-ft)	Discharge (cfs)
0.000	0.000

Water Quality Objective	
TSS Removal (%)	80.0
Runoff Volume Capture (%)	
Oil Spill Capture Volume (Gal)	
Peak Conveyed Flow Rate (CFS)	
Water Quality Flow Rate (CFS)	0.55

Up Stream Flow Diversion	
Max. Flow to Stormceptor (cfs)	

Design Details	
Stormceptor Inlet Invert Elev (ft)	
Stormceptor Outlet Invert Elev (ft)	
Stormceptor Rim Elev (ft)	
Normal Water Level Elevation (ft)	
Pipe Diameter (in)	
Pipe Material	
Multiple Inlets (Y/N)	No
Grate Inlet (Y/N)	No

Particle Size Distribution (PSD)

Removing the smallest fraction of particulates from runoff ensures the majority of pollutants, such as metals, hydrocarbons and nutrients are captured. The table below identifies the Particle Size Distribution (PSD) that was selected to define TSS removal for the Stormceptor design.

OK-110		
Particle Diameter (microns)	Distribution %	Specific Gravity
1.0	0.0	2.65
53.0	3.0	2.65
75.0	15.0	2.65
88.0	25.0	2.65
106.0	41.0	2.65
125.0	15.0	2.65
150.0	1.0	2.65
212.0	0.0	2.65

Site Name	Flow to WQU-5

Site Details

Drainage Area

Total Area (acres)	0.57
Imperviousness %	79.0

Surface Characteristics

Width (ft)	315.00
Slope %	2
Impervious Depression Storage (in)	0.02
Pervious Depression Storage (in)	0.2
Impervious Manning's n	0.015
Pervious Manning's n	0.25

Maintenance Frequency

Maintenance Frequency (months) >	12

Infiltration Parameters

Horton's equation is used to estimate infiltration

Max. Infiltration Rate (in/hr)	2.44
Min. Infiltration Rate (in/hr)	0.4
Decay Rate (1/sec)	0.00055
Regeneration Rate (1/sec)	0.01

Evaporation

Daily Evaporation Rate (in/day)	0.1

Dry Weather Flow

Dry Weather Flow (cfs)	0

Winter Months

Winter Infiltration	0

TSS Loading Parameters

TSS Loading Function	

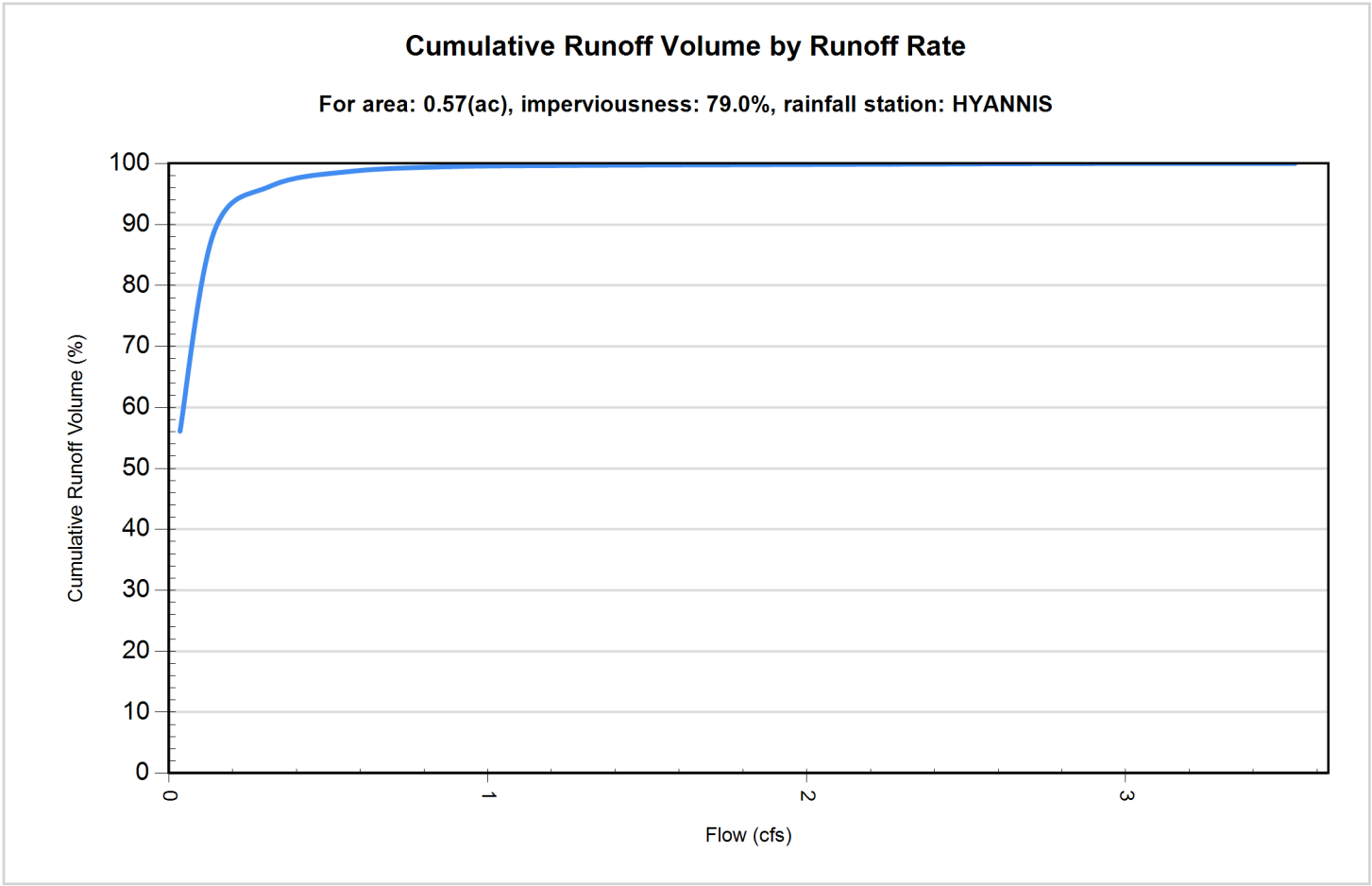
Buildup/Wash-off Parameters

Target Event Mean Conc. (EMC) mg/L	
Exponential Buildup Power	
Exponential Washoff Exponent	

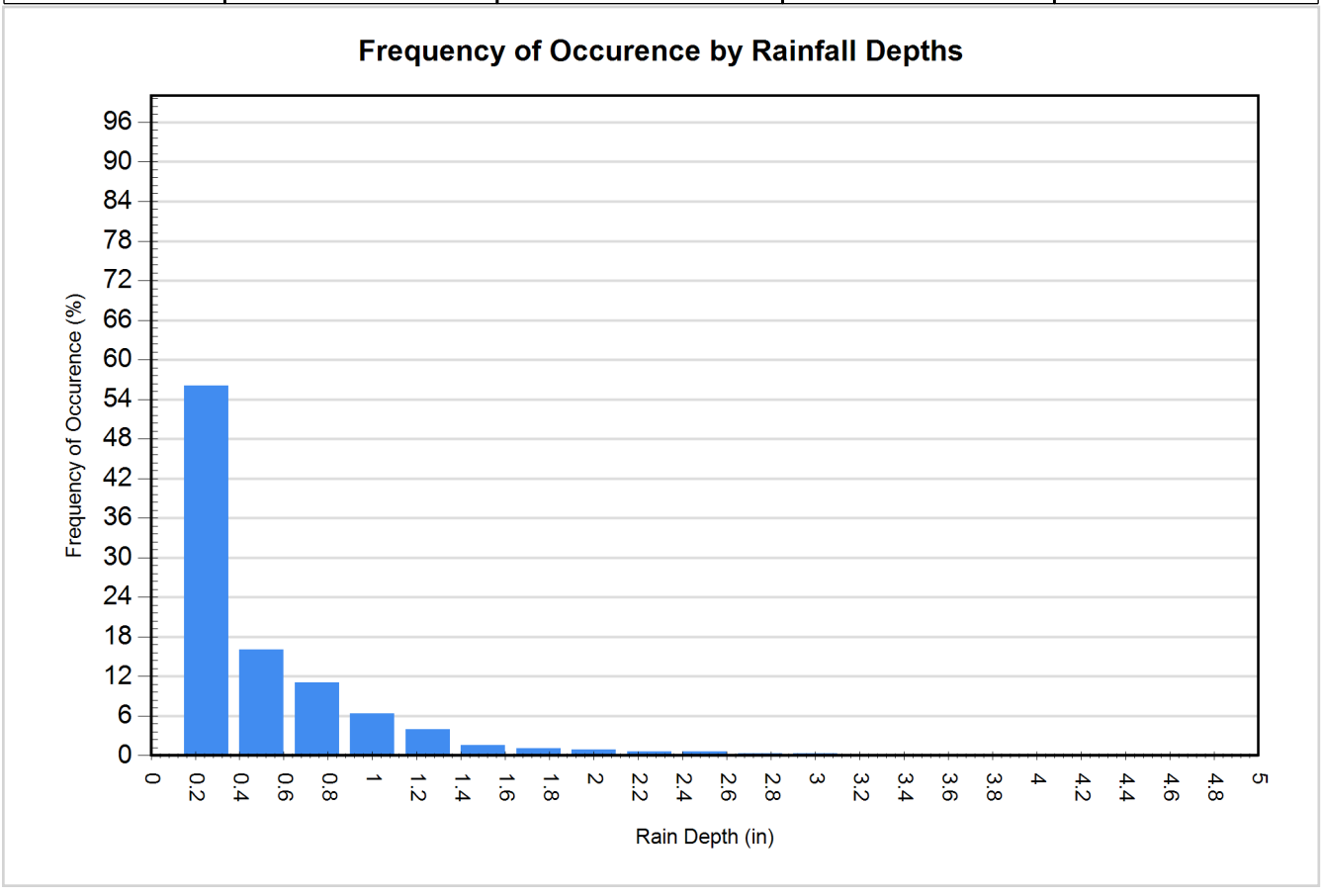
TSS Availability Parameters

Availability Constant A	
Availability Factor B	
Availability Exponent C	
Min. Particle Size Affected by Availability (micron)	

Cumulative Runoff Volume by Runoff Rate			
Runoff Rate (cfs)	Runoff Volume (ft³)	Volume Over (ft³)	Cumulative Runoff Volume (%)
0.035	463604	363075	56.1
0.141	733102	93546	88.7
0.318	795914	30726	96.3
0.565	816265	10373	98.7
0.883	822252	4385	99.5
1.271	824522	2116	99.7
1.730	825267	1371	99.8
2.260	825755	883	99.9
2.860	826296	342	100.0
3.531	826638	0	100.0



Rainfall Event Analysis				
Rainfall Depth (in)	No. of Events	Percentage of Total Events (%)	Total Volume (in)	Percentage of Annual Volume (%)
0.25	711	56.1	71	13.4
0.50	204	16.1	74	14.0
0.75	141	11.1	88	16.5
1.00	81	6.4	72	13.5
1.25	51	4.0	57	10.7
1.50	20	1.6	28	5.2
1.75	14	1.1	23	4.3
2.00	12	0.9	22	4.2
2.25	7	0.6	15	2.8
2.50	7	0.6	17	3.2
2.75	4	0.3	11	2.0
3.00	4	0.3	12	2.2
3.25	3	0.2	9	1.8
3.50	2	0.2	7	1.3
3.75	2	0.2	7	1.3
4.00	3	0.2	12	2.2
4.25	2	0.2	8	1.6
4.50	0	0.0	0	0.0
4.75	0	0.0	0	0.0



Sediment Forebay Sizing Calculations

Sediment Forebay Calculation

Project Name: Eversource Training Facility
Project Location: 37 Doty Street
Project Number: 323-322

Date: 3/7/2024
Calculated By: CJV
Checked By: BEP

SEDIMENT FOREBAY SIZING CALCULATION FOR WET BASIN

TOTAL CONTRIBUTING IMPERVIOUS AREA TO FOREBAY AT NORTHWEST CORNER

= 127,000 s.f.

REQUIRED VOLUME OF SEDIMENT FOREBAY = VOLUME PRODUCED BY 0.1" RUNOFF/IMPERVIOUS ACRE

REQ'D SED. FOREBAY VOLUME = 0.1" INCHES X $\frac{1\ FT}{12\ IN}$ X 127,000 S.F.

= 1,058 C.F.

PROVIDED VOLUME OF SEDIMENT FOREBAY

BOTTOM FOREBAY EL. = 31.00 AREA = 1,300 S.F.
FOREBAY BERM EL. = 32.00 AREA = 2,000 S.F.

VOLUME PROVIDED = 1,650 C.F.

TOTAL CONTRIBUTING IMPERVIOUS AREA TO FOREBAY AT NORTHEAST CORNER

= 63,500 s.f.

REQUIRED VOLUME OF SEDIMENT FOREBAY = VOLUME PRODUCED BY 0.1" RUNOFF/IMPERVIOUS ACRE

REQ'D SED. FOREBAY VOLUME = 0.1" INCHES X $\frac{1\ FT}{12\ IN}$ X 63,500 S.F.

= 529 C.F.

PROVIDED VOLUME OF SEDIMENT FOREBAY

BOTTOM FOREBAY EL. = 30.00 AREA = 380 S.F.
FOREBAY BERM EL. = 31.00 AREA = 690 S.F.

VOLUME PROVIDED = 535 C.F.

APPENDIX D

SUPPORTING INFORMATION

Illicit Discharge Statement

Illicit Discharge Statement

ILLICIT DISCHARGE COMPLIANCE STATEMENT

I VERIFY THAT NO ILLICIT DISCHARGES EXIST FROM THE EVERSOURCE TRAINING FACILITY DEVELOPMENT. THROUGH THE IMPLEMENTATION OF THE *CONSTRUCTION PERIOD POLLUTION PREVENTION AND SEDIMENTATION AND EROSION CONTROL PLAN* AS WELL AS THE *OPERATION AND MAINTENANCE PLAN*, MEASURES ARE SET FORTH TO PREVENT ILLICIT DISCHARGES FROM ENTERING THE STORMWATER MANAGEMENT DRAINAGE SYSTEM.

Jason St. Martin	Jason St Martin	3-8-24
SIGNATURE	**PRINT NAME**	**DATE**

Manager Facilities Operations	Eversource Energy
TITLE	**COMPANY**

SIGNATURE	**PRINT NAME**	**DATE**

TITLE	**COMPANY**

NOTE: THIS CERTIFICATION MUST BE SIGNED BEFORE STORMWATER IS CONVEYED TO THE PROPOSED STORMWATER DRAINAGE SYSTEM IN ACCORDANCE WITH STANDARD 10 OF THE MASSACHUSETTS STORMWATER MANAGEMENT STANDARDS.