Stormwater Management Report

150 Tihonet Road PV+ES Project

150 Tihonet Road (aka 0 & 169 Tihonet Road) Wareham, Massachusetts

Prepared for:



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Borrego Solar

Prepared by:



BEALS + THOMAS

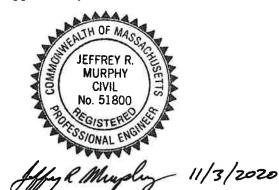
BEALS AND THOMAS, INC. 32 Court Street Plymouth, MA 02360

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1.0 INTRODUCTION

The proposed project includes a stormwater management system designed to mitigate potential impacts the proposed project could have on the existing watershed. Stormwater controls are proposed to control peak runoff rates, provide water quality, promote groundwater recharge and sediment removal. The proposed system has been designed to comply with:

- The 2008 Massachusetts Department of Environmental Protection (MassDEP) Stormwater Management Handbook,
- The Massachusetts Wetland Protection Act (310 CMR 10.00), and
- Town of Wareham Zoning Bylaw
- Wareham Wetland Protective Bylaw

The pre- and post-development hydrologic conditions were modeled using HydroCADTM version 10.00 to demonstrate that post-development stormwater runoff rates will be less than or equal to the pre-development rates. Watershed maps with soil types as well as detailed analysis of the model results are also included. The following table summarizes the peak runoff rates for the pre- and post-development conditions.

Table 1: Pre- & Post-development Peak Runoff Rate Comparison, units are in cubic feet per second (cfs).

Storm Event	2 Year		10 Year		100 Year	
Storm Event	Pre	Post	Pre	Post	Pre	Post
Design Point 1	0.2	0.1	0.8	0.5	2.4	1.6
Design Point 2	2.4	2.3	4.6	4.4	9.7	8.8
Design Point 3	0.0	0.0	0.0	0.0	0.3	0.1
Design Point 4	0.0	0.0	0.1	0.0	2.1	0.1
Design Point 5	0.0	0.0	0.1	0.0	2.1	1.0
Design Point 6	2.6	2.3	5.4	4.6	15.2	12.8



2.0 PRE-DEVELOPMENT CONDITIONS

2.1 Site Conditions

The site is accessed from Tihonet Road. The site is currently undeveloped and is primarily wooded. Runoff from the northern most portion discharges to an existing stream on the northern side of the property. Runoff from the eastern most portion of the site flow to a wetland system on the east side of the property. Runoff from the western most portion of the site flow to a wetland and potential vernal pool system on the west side of the property. Runoff from the southwestern portion of the site drain to a wetland system on the southwest side of the property. Runoff from the northwestern portion of the site drain to a wetland and potential vernal pool system on the northwest side of the property. Runoff from the southwest, west and northwest wetland system ultimately discharge to Tihonet Pond. South of the proposed solar array, there is an existing cleared utility transmission easement.

The site does not contain, nor is it tributary to any Critical Areas.

The site does not discharge to a surface water with a TMDL or draft TMDL.

2.2 Soil Description

The Natural Resources Conservation Service (NRCS) lists the on-site soils groups as hydrologic soil class A, B and D. The soil groups classified by NRCS as hydrologic soil class A include Carver coarse sand, Poquonock sand, and Windsor loamy sand. These soil groups constitute a large portion of the project area. On-site areas mapped as Canton fine sandy loam are classified as hydrologic soil class B. Hydrologic soil class D series found on-site include Birchwood sand, Massasoit-complex and Udipsamments.

2.3 Hydrologic Analysis

Sub-catchment areas were delineated based on existing runoff patterns and topographic information. This information is shown on the *Pre-Development Conditions Hydrologic Areas Map* included in Attachment 2. Summaries of each area with respect to Curve Number and Time of Concentration calculations can be found in the model results also in Attachment 2.



3.0 POST-DEVELOPMENT CONDITIONS

3.1 Design Strategy

During the design phase of the site layout, consideration was given to conserving environmentally sensitive features and minimizing impact on the existing hydrology. To achieve this, the proposed grading endeavored to match the existing drainage patterns where feasible.

The wetland systems in the vicinity of the site were evaluated as individual design points to demonstrate that these systems maintain their existing hydrology.

The proposed solar panels are raised above the ground with the leading edge tilted to the south. Stormwater that lands on the panels will sheet down off the front edge to the pervious sandy ground below, which will be vegetated with an herbaceous seed mix.

There will be several concrete pads associated with the utility equipment that will produce a negligible amount of runoff which will flow to adjacent pervious soils. These have been accounted for in the stormwater design and analysis.

3.2 Hydrologic Analysis Methodology

The established design points used in the pre-development conditions analysis were used in the post-development analysis for direct comparison. The tributary areas and flow paths were modified to reflect post-development conditions. See Attachment 3 for the *Post-Development Conditions Hydrologic Areas Map*. Summaries of each area with respect to Curve Number and Time of Concentration calculations can be found in the model results in Attachment 3.

3.3 Compliance with MassDEP Stormwater Management Standards

The proposed stormwater management system was designed in compliance with the ten (10) MassDEP Stormwater Management Standards. The following summary provides key information related to the design approach and mitigation measures for stormwater.



STANDARD 1:

No new stormwater conveyance (e.g. outfalls) may discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.

There will be no direct discharge of untreated stormwater from the site. Erosion control barriers will be installed as depicted on the plans and will remain in place throughout construction and until the site is stabilized with vegetation.

STANDARD 2:

Stormwater management systems shall be designed so that postdevelopment peak discharge rates do not exceed pre-development peak discharge rates.

The proposed stormwater management system will effectively maintain the post-development peak discharge rates for the 2-, 10-, and 100-year, 24-hour storms. Refer to Section 1.0 Introduction for a summary of the peak runoff rates.

STANDARD 3:

Loss of annual recharge to groundwater shall be eliminated or minimized through the use of environmentally sensitive site design, low impact development techniques, stormwater management practices and good operation and maintenance. At a minimum, the annual recharge from the post-development site shall approximate the annual recharge from pre-development conditions based on soil types. This Standard is met when the stormwater management system is designed to infiltrate the required recharge volume as determined in accordance with the Massachusetts Stormwater Handbook.

The proposed solar panels, while covering a large footprint, will allow water to sheet flow to the ground below where it can be absorbed into the sandy on-site soils. Other minimal areas of impervious (i.e. concrete pads) as well as the proposed changes in vegetative cover have been accounted for in the design. Proposed infiltration basins will provide the required recharge based on the impervious footprint of the various concrete pads. Therefore, recharge of groundwater will be maintained under the post-development condition.



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STANDARD 4: Stormwater management systems shall be designed to remove 80% of the average annual post-construction load of Total Suspended Solids (TSS).

The proposed project does not include any proposed impervious surfaces requiring treatment for water quality. Therefore, the 80% TSS removal requirement does not apply.

STANDARD 5:

For land uses with higher potential pollutant loads (LUHPPLs), source control and pollution prevention shall be implemented in accordance with the Massachusetts Stormwater Handbook to eliminate or reduce the discharge of stormwater runoff from such land uses to the maximum extent practicable.

The proposed project is not associated with stormwater discharges from land uses with higher potential pollutant loads.

STANDARD 6:

Stormwater discharges to critical areas must utilize certain stormwater management BMPs approved for critical areas. Critical areas are Outstanding Resource Waters, shellfish beds, swimming beaches, coldwater fisheries and recharge areas for public water supplies.

There are no stormwater discharges to critical areas associated with this project.

STANDARD 7:

Redevelopment of previously developed sites must meet the Stormwater Management Standards to the maximum extent practicable. However, if it is not practicable to meet all the Standards, new (retrofitted or expanded) stormwater management systems must be designed to improve existing conditions.

The proposed project is a new development, and therefore this standard does not apply.



STANDARD 8:

A plan to control construction-related impacts during erosion, sedimentation and other pollutant sources during construction and land disturbance activities (construction period erosion, sedimentation, and pollution prevention plan) shall be developed and implemented.

Since the project will disturb greater than 1 acre, a DRAFT Stormwater Pollution Prevention Plan (SWPPP) has been developed and is included in Attachment 6. The SWPPP will be finalized prior to construction to comply with Section 3 of the NPDES Construction General Permit for Stormwater Discharges; therefore the requirements of Standard 8 are fulfilled.

STANDARD 9: A Long-Term Operation and Maintenance (O&M) Plan shall be developed and implemented to ensure that stormwater management systems function as designed.

The Site Owner's Manual complies with the Long-Term Pollution Prevention Plan (Standard 4) and the Long-Term Operation and Maintenance Plan (Standard 9) requirements of the 2008 MassDEP Stormwater Management Standards. The Manual outlines source control and pollution prevention measures and maintenance requirements associated with the proposed development. A Site Owner's Manual is included as Attachment 5.

STANDARD 10: All illicit discharges to the stormwater management system are prohibited.

There will be no illicit discharges to the proposed stormwater management system associated with the proposed project. An Illicit Discharge Compliance Statement is provided on the following page.



3.4 Illicit Discharge Compliance Statement

An illicit discharge is any discharge to a stormwater management system that is not comprised entirely of stormwater, discharges from fire-fighting activities, and certain non-designated non-stormwater discharges.

To the best of my knowledge, no detectable illicit discharge exists on site. The site plans included with this report detail the storm sewers that convey stormwater on the site and demonstrate that these systems do not include the entry of an illicit discharge. A Site Owner's Manual is included, which contains the Long Term Pollution Prevention Plan that outlines measures to prevent future illicit discharges. As the Site Owner, I will ultimately be responsible for implementing the Long Term Pollution Prevention Plan.

Signature:

Owner's Name





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Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.





A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals. This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8²
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

¹ The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

² For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



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Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

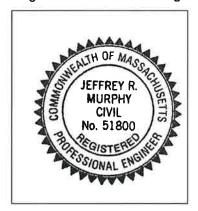
Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



Signature and Date 11/3/2020

Checklist

	eject Type: Is the application for new development, redevelopment, or a mix of new and evelopment?
\boxtimes	New development
	Redevelopment
	Mix of New Development and Redevelopment



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Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

\boxtimes	No disturbance to any Wetland Resource Areas
	Site Design Practices (e.g. clustered development, reduced frontage setbacks)
	Reduced Impervious Area (Redevelopment Only)
	Minimizing disturbance to existing trees and shrubs
	LID Site Design Credit Requested:
	Credit 1
	☐ Credit 2
	☐ Credit 3
\boxtimes	Use of "country drainage" versus curb and gutter conveyance and pipe
	Bioretention Cells (includes Rain Gardens)
	Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
	Treebox Filter
	Water Quality Swale
	Grass Channel
	Green Roof
	Other (describe):
Sta	ndard 1: No New Untreated Discharges
\boxtimes	No new untreated discharges
	Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
\boxtimes	Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



Checklist for Stormwater Report

Cł	necklist (contin	ued)			
Sta	ındard 2: Peak Rat	te Attenuation			
	Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding. Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.				
	Calculations provided to show that post-development peak discharge rates do not exceed pre- development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24- hour storm.				
Sta	ındard 3: Recharge	•			
\boxtimes	Soil Analysis provid	ded.			
\boxtimes	Required Recharge	e Volume calculation provided.			
	Required Recharge	e volume reduced through use of	the LID site Design Credits.		
\boxtimes	Sizing the infiltratio	n, BMPs is based on the followir	g method: Check the method used.		
	Static	⊠ Simple Dynamic	☐ Dynamic Field¹		
\boxtimes	Runoff from all imp	ervious areas at the site dischar	ging to the infiltration BMP.		
	are provided showi		scharging to the infiltration BMP and calculations uting runoff to the infiltration BMPs is sufficient to		
\boxtimes	Recharge BMPs ha	ave been sized to infiltrate the Re	equired Recharge Volume.		
		ave been sized to infiltrate the Refort the following reason:	equired Recharge Volume only to the maximum		
	☐ Site is comprise	ed solely of C and D soils and/or	bedrock at the land surface		
	☐ M.G.L. c. 21E s	sites pursuant to 310 CMR 40.00	00		
	☐ Solid Waste La	andfill pursuant to 310 CMR 19.0	00		
	Project is other practicable.	wise subject to Stormwater Man	agement Standards only to the maximum extent		
\boxtimes	Calculations showing	ng that the infiltration BMPs will o	drain in 72 hours are provided.		
	Property includes a	a M.G.L. c. 21E site or a solid wa	ste landfill and a mounding analysis is included.		

¹ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



Checklist for Stormwater Report

Checkli	st (continued)
Standard 3	3: Recharge (continued)
year 24	iltration BMP is used to attenuate peak flows during storms greater than or equal to the 10- 1-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding s is provided.
	entation is provided showing that infiltration BMPs do not adversely impact nearby wetland ce areas.
Standard 4	1: Water Quality
 Good h 	Ferm Pollution Prevention Plan typically includes the following: nousekeeping practices; ons for storing materials and waste products inside or under cover;
 Vehicle Require Spill pro Provision Require Pet was Provision Provision Snow of the control of the contro	ewashing controls; ements for routine inspections and maintenance of stormwater BMPs; evention and response plans; ons for maintenance of lawns, gardens, and other landscaped areas; ements for storage and use of fertilizers, herbicides, and pesticides; ste management provisions; ons for operation and management of septic systems; ons for solid waste management; disposal and plowing plans relative to Wetland Resource Areas; Road Salt and/or Sand Use and Storage restrictions; sweeping schedules; ons for prevention of illicit discharges to the stormwater management system; tentation that Stormwater BMPs are designed to provide for shutdown and containment in the of a spill or discharges to or near critical areas or from LUHPPL; g for staff or personnel involved with implementing Long-Term Pollution Prevention Plan; Emergency contacts for implementing Long-Term Pollution Prevention Plan.
attachn Treatm	I-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an nent to the Wetlands Notice of Intent. Ient BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule fo ting the water quality volume are included, and discharge:
☐ is v	within the Zone II or Interim Wellhead Protection Area
☐ is r	near or to other critical areas
☐ is v	within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
inve	olves runoff from land uses with higher potential pollutant loads.

☐ The Required Water Quality Volume is reduced through use of the LID site Design Credits.

applicable, the 44% TSS removal pretreatment requirement, are provided.

☐ Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if



Checklist for Stormwater Report

Cr	lecklist (continued)
Sta	ndard 4: Water Quality (continued)
	The BMP is sized (and calculations provided) based on:
	☐ The ½" or 1" Water Quality Volume or
	☐ The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
	The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
	A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.
Sta	ndard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)
	The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report. The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted <i>prior to</i> the discharge of stormwater to the post-construction stormwater BMPs.
	The NPDES Multi-Sector General Permit does <i>not</i> cover the land use.
	LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
	All exposure has been eliminated.
	All exposure has <i>not</i> been eliminated and all BMPs selected are on MassDEP LUHPPL list.
	The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.
Sta	ndard 6: Critical Areas
	The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
	Critical areas and BMPs are identified in the Stormwater Report.



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Checklist for Stormwater Report

Checklist (continued)

<u> </u>	•
Practicable as a:	
 □ Limited Project □ Small Residential Projects: 5-9 single family houses or 5-9 units in a provided there is no discharge that may potentially affect a critical ar □ Small Residential Projects: 2-4 single family houses or 2-4 units in a with a discharge to a critical area □ Marina and/or boatyard provided the hull painting, service and maint from exposure to rain, snow, snow melt and runoff 	ea. multi-family development
□ Bike Path and/or Foot Path□ Redevelopment Project	
☐ Redevelopment portion of mix of new and redevelopment.	
explanation of why these standards are not met is contained in the Storn	nwater Report. at have been taken to edevelopment checklist found be used to document that rds 2, 3 and the pretreatment

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.
- A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



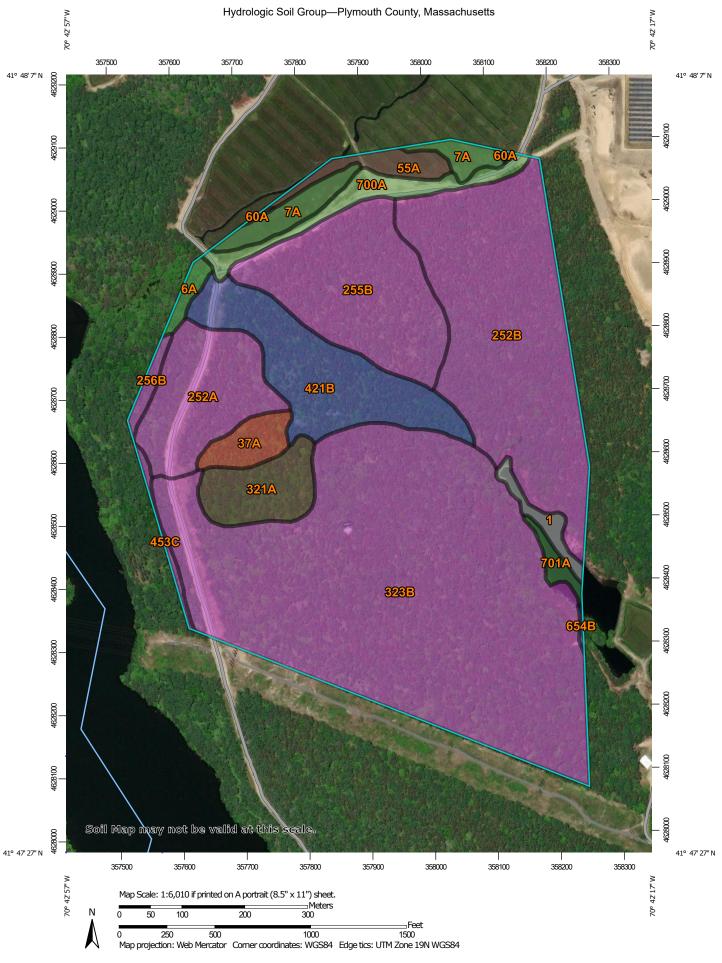
Checklist for Stormwater Report

Checklist (continued)

	Indard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control ntinued)
	The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has <i>not</i> been included in the Stormwater Report but will be submitted <i>before</i> land disturbance begins.
	The project is <i>not</i> covered by a NPDES Construction General Permit.
\boxtimes	The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the
	Stormwater Report. The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.
Sta	ndard 9: Operation and Maintenance Plan
\boxtimes	The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
	Name of the stormwater management system owners;
	□ Party responsible for operation and maintenance;
	Schedule for implementation of routine and non-routine maintenance tasks;
	☑ Plan showing the location of all stormwater BMPs maintenance access areas;
	□ Description and delineation of public safety features;
	The responsible party is <i>not</i> the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
	A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
	A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.
Sta	ndard 10: Prohibition of Illicit Discharges
\boxtimes	The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
\boxtimes	An Illicit Discharge Compliance Statement is attached;
	NO Illicit Discharge Compliance Statement is attached but will be submitted <i>prior to</i> the discharge of any stormwater to post-construction BMPs.

Attachment 1 Soil Data





MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) С 1:12.000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D Soil Rating Polygons Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil **Water Features** line placement. The maps do not show the small areas of A/D contrasting soils that could have been shown at a more detailed Streams and Canals Transportation B/D Rails ---Please rely on the bar scale on each map sheet for map measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service **US Routes** Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available -Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Soil Rating Lines Background distance and area. A projection that preserves area, such as the Aerial Photography Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. Soil Survey Area: Plymouth County, Massachusetts Survey Area Data: Version 12, Sep 12, 2019 Soil map units are labeled (as space allows) for map scales 1:50.000 or larger. Not rated or not available Date(s) aerial images were photographed: Dec 31, 2009—Jul 3. 2017 **Soil Rating Points** The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. B/D

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
1	Water		1.2	0.9%
6A	Scarboro muck, coastal lowland, 0 to 3 percent slopes	A/D	0.8	0.6%
7A	Rainberry coarse sand, 0 to 3 percent slopes, sanded surface	A/D	3.3	2.4%
37A	Massasoit - Mashpee complex, 0 to 3 percent slopes	D	2.2	1.6%
55A	Freetown coarse sand, 0 to 3 percent slopes, sanded surface	B/D	2.0	1.5%
60A	Swansea coarse sand, 0 to 2 percent slopes	B/D	0.1	0.1%
252A	Carver coarse sand, 0 to 3 percent slopes	А	8.7	6.4%
252B	Carver coarse sand, 3 to 8 percent slopes	А	25.5	18.8%
255B	Windsor loamy sand, 3 to 8 percent slopes	А	14.0	10.3%
256B	Deerfield loamy fine sand, 3 to 8 percent slopes	A	0.8	0.6%
321A	Birchwood sand, 0 to 3 percent slopes, very stony	B/D	4.4	3.3%
323B	Poquonock sand, 3 to 8 percent slopes, very stony	A	56.6	41.8%
421B	Canton fine sandy loam, 0 to 8 percent slopes, very stony	В	11.1	8.2%
453C	Gloucester - Canton complex, 8 to 15 percent slopes, extremely bouldery	A	0.6	0.4%
654B	Udorthents, loamy, 0 to 8 percent slopes	В	0.1	0.1%
700A	Udipsamments, wet substratum, 0 to 3 percent slopes	A/D	3.3	2.4%
701A	Rainberry coarse sand, 0 to 3 percent slope, sanded surface, inactive	A/D	0.8	0.6%

	Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI	
Totals for Area of Interest		135.3	100.0%			

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified

Tie-break Rule: Higher

252B—Carver coarse sand, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2y07x

Elevation: 0 to 240 feet

Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 240 days

Farmland classification: Not prime farmland

Map Unit Composition

Carver, coarse sand, and similar soils: 80 percent

Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Carver, Coarse Sand

Setting

Landform: Outwash plains, moraines

Landform position (two-dimensional): Summit, shoulder,

backslope, footslope, toeslope

Landform position (three-dimensional): Crest, head slope, nose

slope, side slope, tread

Down-slope shape: Linear, convex

Across-slope shape: Linear

Parent material: Sandy glaciofluvial deposits

Typical profile

Oi - 0 to 2 inches: slightly decomposed plant material Oe - 2 to 3 inches: moderately decomposed plant material

A - 3 to 7 inches: coarse sand E - 7 to 10 inches: coarse sand Bw1 - 10 to 15 inches: coarse sand Bw2 - 15 to 28 inches: coarse sand BC - 28 to 32 inches: coarse sand C - 32 to 67 inches: coarse sand

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Excessively drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat):

Moderately high to very high (1.42 to 14.17 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Salinity, maximum in profile: Nonsaline (0.0 to 1.9 mmhos/cm) Available water storage in profile: Low (about 4.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3s

Hydrologic Soil Group: A

Ecological site: Dry Outwash (F149BY005MA)

Hydric soil rating: No

Minor Components

Deerfield

Percent of map unit: 10 percent

Landform: Outwash plains, outwash terraces, outwash deltas,

kame terraces

Landform position (three-dimensional): Tread

Down-slope shape: Linear Across-slope shape: Concave

Hydric soil rating: No

Hinckley

Percent of map unit: 5 percent

Landform: Moraines, kames, outwash terraces, eskers, kame

terraces, outwash plains, outwash deltas

Landform position (two-dimensional): Summit, toeslope, shoulder,

backslope, footslope

Landform position (three-dimensional): Side slope, crest, head

slope, nose slope, riser, tread Down-slope shape: Convex Across-slope shape: Convex

Hydric soil rating: No

Merrimac

Percent of map unit: 3 percent

Landform: Outwash terraces, outwash deltas, kame terraces

Landform position (three-dimensional): Tread, riser

Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Mashpee

Percent of map unit: 2 percent

Landform: Terraces, drainageways, depressions Landform position (three-dimensional): Tread

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Data Source Information

Soil Survey Area: Plymouth County, Massachusetts

Survey Area Data: Version 12, Sep 12, 2019

323B—Poquonock sand, 3 to 8 percent slopes, very stony

Map Unit Setting

National map unit symbol: bcz7

Elevation: 0 to 400 feet

Mean annual precipitation: 41 to 54 inches Mean annual air temperature: 43 to 54 degrees F

Frost-free period: 145 to 240 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Poquonock, very stony, and similar soils: 80 percent

Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Poquonock, Very Stony

Setting

Landform: Drumlins, ground moraines, till plains

Landform position (two-dimensional): Shoulder, summit

Landform position (three-dimensional): Interfluve

Down-slope shape: Convex Across-slope shape: Convex

Parent material: Sandy eolian deposits and/or glaciofluvial deposits

over coarse-loamy lodgment till

Typical profile

Oi - 0 to 1 inches: slightly decomposed plant material
Oe - 1 to 2 inches: moderately decomposed plant material

A - 2 to 4 inches: sand E - 4 to 5 inches: sand

Bs - 5 to 7 inches: loamy sand Bw - 7 to 26 inches: sand

BC - 26 to 35 inches: loamy sand

2Cd1 - 35 to 49 inches: gravelly sandy loam 2Cd2 - 49 to 71 inches: gravelly sandy loam

Properties and qualities

Slope: 3 to 8 percent

Percent of area covered with surface fragments: 1.5 percent Depth to restrictive feature: 20 to 39 inches to densic material

Natural drainage class: Well drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): Very

low to moderately low (0.00 to 0.14 in/hr) Depth to water table: About 22 to 35 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: Very low (about 1.9 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s

Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Birchwood, very stony

Percent of map unit: 8 percent

Landform: Drumlins, ground moraines, till plains

Landform position (two-dimensional): Summit, footslope

Landform position (three-dimensional): Interfluve

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: No

Mattapoisett, extremely stony

Percent of map unit: 7 percent

Landform: Depressions, drainageways

Landform position (two-dimensional): Toeslope, footslope

Landform position (three-dimensional): Base slope

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Montauk, very stony

Percent of map unit: 3 percent

Landform: Drumlins, ground moraines, till plains
Landform position (two-dimensional): Summit, shoulder
Landform position (three-dimensional): Interfluve

Down-slope shape: Convex Across-slope shape: Convex

Hydric soil rating: No

Scituate, very stony

Percent of map unit: 2 percent Landform: Ridges, drumlins

Landform position (two-dimensional): Footslope, shoulder

Landform position (three-dimensional): Interfluve

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: No

Data Source Information

Soil Survey Area: Plymouth County, Massachusetts

Survey Area Data: Version 12, Sep 12, 2019

321A—Birchwood sand, 0 to 3 percent slopes, very stony

Map Unit Setting

National map unit symbol: 9y46

Elevation: 0 to 400 feet

Mean annual precipitation: 41 to 54 inches Mean annual air temperature: 43 to 54 degrees F

Frost-free period: 145 to 240 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Birchwood, very stony, and similar soils: 80 percent

Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Birchwood, Very Stony

Setting

Landform: Drumlins, ground moraines, till plains

Landform position (two-dimensional): Summit, footslope

Landform position (three-dimensional): Interfluve

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Sandy eolian deposits and/or sandy glaciofluvial

deposits over coarse-loamy lodgment till

Typical profile

Oi - 0 to 1 inches: slightly decomposed plant material
Oe - 1 to 3 inches: moderately decomposed plant material
Oa - 3 to 4 inches: highly decomposed plant material

E - 4 to 5 inches: sand

Ap - 5 to 8 inches: loamy sand Bs - 8 to 13 inches: loamy sand Bw1 - 13 to 19 inches: loamy sand Bw2 - 19 to 29 inches: loamy sand

BC - 29 to 40 inches: sand

Cd1 - 40 to 55 inches: gravelly sandy loam Cd2 - 55 to 75 inches: gravelly sandy loam

Properties and qualities

Slope: 0 to 3 percent

Percent of area covered with surface fragments: 1.0 percent Depth to restrictive feature: 35 to 59 inches to densic material

Natural drainage class: Moderately well drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): Very

low to moderately high (0.00 to 0.20 in/hr) Depth to water table: About 12 to 29 inches

Frequency of flooding: None

37A—Massasoit - Mashpee complex, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: bd1q

Elevation: 0 to 400 feet

Mean annual precipitation: 41 to 54 inches Mean annual air temperature: 43 to 54 degrees F

Frost-free period: 145 to 240 days

Farmland classification: Not prime farmland

Map Unit Composition

Massasoit and similar soils: 55 percent Mashpee and similar soils: 35 percent

Minor components: 10 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Massasoit

Setting

Landform: Terraces, depressions, drainageways

Landform position (two-dimensional): Footslope, toeslope

Landform position (three-dimensional): Tread

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Sandy and gravelly glaciofluvial deposits

Typical profile

Oe - 0 to 1 inches: moderately decomposed plant material Oa - 1 to 3 inches: highly decomposed plant material

A - 3 to 5 inches: fine sand Eg1 - 5 to 11 inches: fine sand Eg2 - 11 to 13 inches: fine sand Bhs - 13 to 17 inches: fine sand Bsm - 17 to 23 inches: fine sand Bs - 23 to 26 inches: fine sand BC - 26 to 43 inches: fine sand

Cg - 43 to 80 inches: loamy very fine sand

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: 7 to 20 inches to ortstein

Natural drainage class: Poorly drained

Runoff class: Very high

Capacity of the most limiting layer to transmit water (Ksat): Very

low to moderately low (0.00 to 0.01 in/hr) Depth to water table: About 0 to 12 inches

Frequency of flooding: None Frequency of ponding: Occasional

Available water storage in profile: Very low (about 1.3 inches)

421B—Canton fine sandy loam, 0 to 8 percent slopes, very stony

Map Unit Setting

National map unit symbol: 2w81l

Elevation: 0 to 1,180 feet

Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 240 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Canton, very stony, and similar soils: 80 percent

Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Canton, Very Stony

Setting

Landform: Hills, ridges, moraines

Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Side slope, crest, nose

slope

Down-slope shape: Linear, convex Across-slope shape: Convex

Parent material: Coarse-loamy over sandy melt-out till derived from

gneiss, granite, and/or schist

Typical profile

Oi - 0 to 2 inches: slightly decomposed plant material

A - 2 to 5 inches: fine sandy loam
Bw1 - 5 to 16 inches: fine sandy loam

Bw2 - 16 to 22 inches: gravelly fine sandy loam 2C - 22 to 67 inches: gravelly loamy sand

Properties and qualities

Slope: 0 to 8 percent

Percent of area covered with surface fragments: 1.6 percent Depth to restrictive feature: 19 to 39 inches to strongly contrasting

textural stratification

Natural drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat):

Moderately low to high (0.14 to 14.17 in/hr) Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Salinity, maximum in profile: Nonsaline (0.0 to 1.9 mmhos/cm) Available water storage in profile: Low (about 3.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s
Hydrologic Soil Group: B

Hydric soil rating: No

Minor Components

Scituate, very stony

Percent of map unit: 9 percent

Landform: Drumlins, hills, ground moraines

Landform position (two-dimensional): Footslope, backslope,

summit

Landform position (three-dimensional): Side slope, crest

Down-slope shape: Linear, convex Across-slope shape: Convex Hydric soil rating: No

Montauk, very stony

Percent of map unit: 5 percent

Landform: Recessionial moraines, hills, drumlins, ground moraines Landform position (two-dimensional): Backslope, shoulder, summit

Landform position (three-dimensional): Side slope, crest

Down-slope shape: Linear, convex Across-slope shape: Convex Hydric soil rating: No

Gloucester, very stony

Percent of map unit: 4 percent Landform: Hills, ridges, moraines

Landform position (two-dimensional): Backslope, shoulder, summit

Landform position (three-dimensional): Side slope, crest

Down-slope shape: Linear, convex Across-slope shape: Convex Hydric soil rating: No

Swansea

Percent of map unit: 2 percent

Landform: Kettles, bogs, depressions, swamps, marshes

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Data Source Information

Soil Survey Area: Plymouth County, Massachusetts Survey Area Data: Version 12, Sep 12, 2019

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 4w

Hydrologic Soil Group: D Hydric soil rating: Yes

Description of Mashpee

Setting

Landform: Depressions, drainageways, terraces

Landform position (two-dimensional): Footslope, toeslope

Landform position (three-dimensional): Tread

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Sandy and gravelly glaciofluvial deposits

Typical profile

Oe1 - 0 to 2 inches: moderately decomposed plant material Oe2 - 2 to 4 inches: moderately decomposed plant material

Oa - 4 to 5 inches: highly decomposed plant material

AE - 5 to 7 inches: loamy fine sand Eg - 7 to 11 inches: fine sand Bh1 - 11 to 13 inches: fine sand Bh2 - 13 to 17 inches: fine sand Bs - 17 to 24 inches: loamy fine sand

C1 - 24 to 39 inches: fine sand C2 - 39 to 65 inches: fine sand

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Poorly drained

Runoff class: Very high

Capacity of the most limiting layer to transmit water (Ksat):

Moderately high to high (1.42 to 5.95 in/hr) Depth to water table: About 0 to 12 inches

Frequency of flooding: None Frequency of ponding: Occasional

Available water storage in profile: Low (about 4.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4w

Hydrologic Soil Group: A/D Hydric soil rating: Yes

Minor Components

Deerfield

Percent of map unit: 5 percent

Landform: Outwash plains, terraces, deltas

Landform position (two-dimensional): Footslope, summit

Landform position (three-dimensional): Tread

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: No

Rainberry

Percent of map unit: 3 percent Landform: Depressions, kettles

Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: Yes

Squamscott

Percent of map unit: 2 percent Landform: Lake terraces, lake plains

Landform position (two-dimensional): Footslope, toeslope

Landform position (three-dimensional): Talf

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Data Source Information

Soil Survey Area: Plymouth County, Massachusetts

Survey Area Data: Version 12, Sep 12, 2019

Frequency of ponding: None

Available water storage in profile: Low (about 3.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 5s

Hydrologic Soil Group: B/D Hydric soil rating: No

Minor Components

Poquonock, very stony

Percent of map unit: 6 percent

Landform: Ground moraines, till plains, drumlins Landform position (two-dimensional): Summit, shoulder Landform position (three-dimensional): Interfluve

Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

Mattapoisett, extremely stony

Percent of map unit: 6 percent

Landform: Depressions, drainageways

Landform position (two-dimensional): Toeslope, footslope

Landform position (three-dimensional): Base slope

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Scituate, very stony

Percent of map unit: 5 percent Landform: Ridges, drumlins

Landform position (two-dimensional): Summit, footslope

Landform position (three-dimensional): Interfluve

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: No

Newfields, extremely stony

Percent of map unit: 3 percent Landform: Till plains, hills, moraines

Landform position (two-dimensional): Footslope, summit

Landform position (three-dimensional): Interfluve

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: No

Data Source Information

Soil Survey Area: Plymouth County, Massachusetts

Survey Area Data: Version 12, Sep 12, 2019

Attachment 2
Pre-Development Hydrologic Analysis





CALCULATION SUMMARY

T 508.366.0560 F 508.366.4391 www.bealsandthomas.com Regional Office: Plymouth, MA

$I \cap R$	$M\Omega$	$/I \cap CA$	TION:
JUD	WO.	/LUUA	110N.

1833.112 Wareham, MA

CLIENT/PROJECT:

Borrego Solar Systems, Inc. 150 Tihonet Road PV+ES Project

SUBJECT/TITLE:

Pre-Development Hydrologic Calculations

OBJECTIVE OF CALCULATION:

• To determine the pre-development peak rates of runoff from the site for the 2, 10, & 100-year storm events at design points DP-1 through DP-6.

CALCULATION METHOD(S):

- Runoff curve numbers (CN), time-of-concentration (Tc), and runoff rates were calculated based on TR-55 methodology.
- Autodesk Civil 3D 2019 computer program was utilized for digitizing ground cover areas.
- Peak runoff rates were computed using HydroCAD version 10.00.
- Peak runoff rates were rounded to the nearest tenth.

ASSUMPTIONS:

- The ground cover types were determined using MassGIS aerial imagery and hydrologic soil groups based on United States Department of Agriculture, NRCS Soil Survey map information.
- Watershed boundaries have been estimated based upon contour information depicted on the Topographic Plan as well as MassGIS contours for offsite areas outside limits of topographic plan.
- Wetland systems were included in the hydrologic analysis and modeled as Woods Good.

SOURCES OF DATA/EQUATIONS:

- Pre-Development Conditions Hydrologic Areas Map prepared by Beals and Thomas, Inc. File No. 1833112P594A-001.
- Existing topography from Limited Alta/ NSPS Land Title Survey of Land in Wareham, MA (1 Sheet), prepared by Northeast Survey Consultants.
- NRCS Soil Survey for Plymouth County, hydrologic soil group report, downloaded from Web Soil Survey on 3/12/2020.
- TR-55 urban Hydrology for Small Watersheds, SCS, 1986.
- Massachusetts DEP Stormwater Management Handbook, February 2008.

REV	CALC. BY	DATE	CHECKED BY	DATE	APPROVED BY	DATE
0	EAE	5/21/2020	J. Murphy	06/02/2020	J. Murphy	06/02/2020
			1 2		1 2	

EAE/1833112CS004





CALCULATION SUMMARY

T 508.366.0560 F 508.366.4391 www.bealsandthomas.com Regional Office: Plymouth, MA

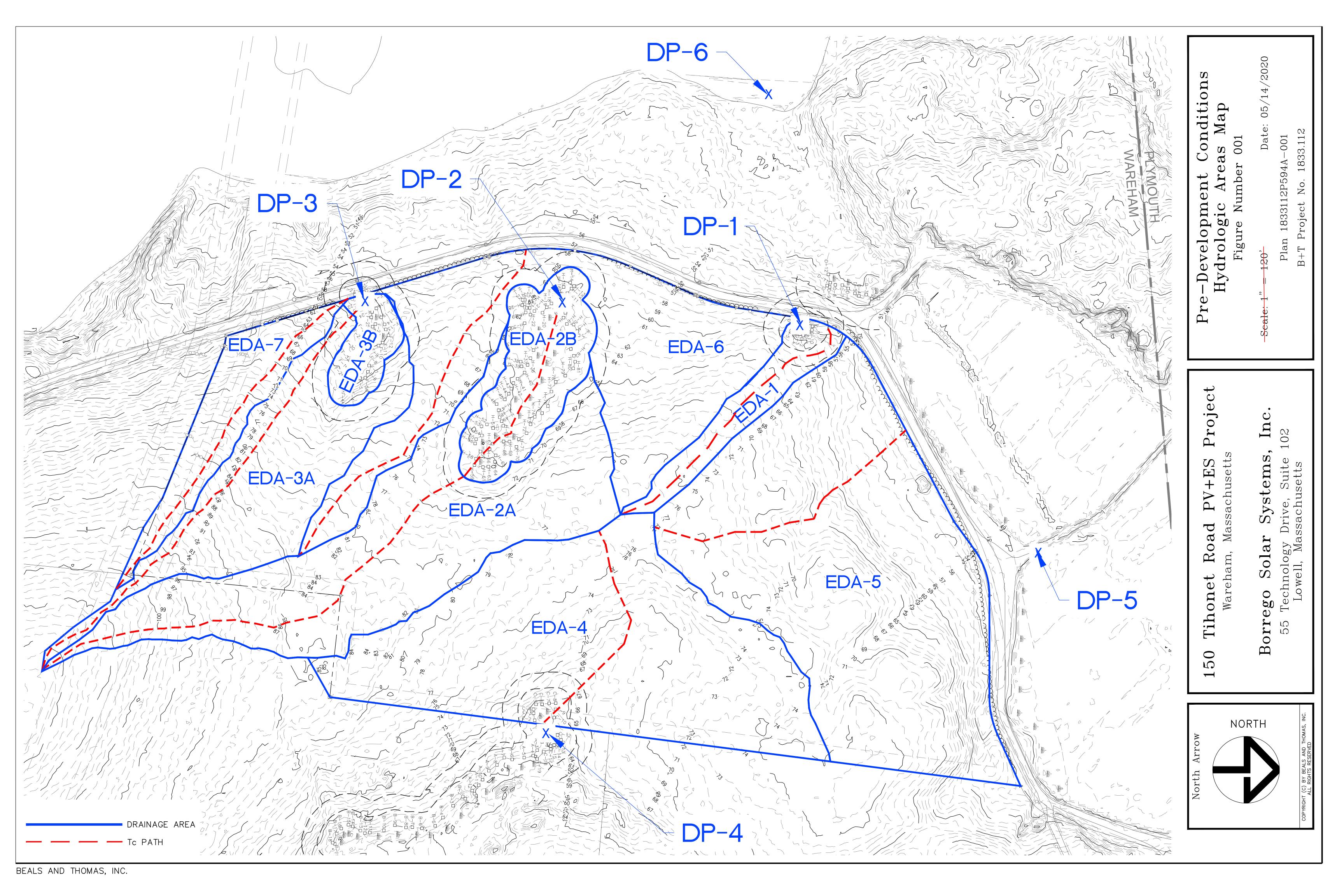
CONCLUSIONS:

Storm Event	DP-1 (CFS)	DP-2 (CFS)	DP-3 (CFS)	DP-4 (CFS)	DP-5 (CFS)	DP-6 (CFS)
2-Year	0.2	2.4	0.0	0.0	0.0	2.6
10-Year	0.8	4.6	0.0	0.1	0.1	5.4
100-Year	2.4	9.7	0.3	2.1	2.1	15.2

REV	CALC. BY	DATE	CHECKED BY	DATE	APPROVED BY	DATE
0	EAE	5/21/2020	J. Murphy	06/02/2020	J. Murphy	06/02/2020

EAE/1833112CS004





150 Tihonet Road Solar Pre-Development Conditions Hydrology EDA-3B EDA-2B EDA\3B EDA-2A DP-2 EDA-3A DP-3 EDA-2A DP-3 DP-2 EDA-3A EDA-6 DP-1 EDA-1 DP-6 EDA-6 DP-1 EDA-1 DP-6 EDA-7 EDA-7 EDA-4 DP-4 EDA-4 DP-4 DP-5 EDA-5 EDA-5 DP-5 Link Routing Diagram for 1833112HC003 Subcat Reach Pond` Prepared by Beals and Thomas, Inc, Printed 6/4/2020 HydroCAD® 10.10-3a s/n 04493 © 2020 HydroCAD Software Solutions LLC

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Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
3.077	39	>75% Grass cover, Good, HSG A (EDA-2A, EDA-3A, EDA-7)
59.166	30	Woods, Good, HSG A (EDA-2A, EDA-2B, EDA-3A, EDA-3B, EDA-4, EDA-5, EDA-6,
		EDA-7)
10.323	55	Woods, Good, HSG B (EDA-1, EDA-2A, EDA-4, EDA-5, EDA-6)
6.642	77	Woods, Good, HSG D (EDA-2A, EDA-2B, EDA-6)
79.208	38	TOTAL AREA

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-1: EDA-1	Runoff Area=2.052 ac 0.00% Impervious Runoff Depth>2.10" Flow Length=945' Tc=36.8 min CN=55 Runoff=2.44 cfs 0.360 af
Subcatchment EDA-2A: EDA-2A	Runoff Area=12.932 ac 0.00% Impervious Runoff Depth>0.61" Flow Length=1,746' Tc=54.8 min CN=37 Runoff=2.04 cfs 0.657 af
Subcatchment EDA-2B: EDA-2B	Runoff Area=3.787 ac 0.00% Impervious Runoff Depth>3.91" Flow Length=575' Tc=36.9 min CN=73 Runoff=8.95 cfs 1.233 af
Subcatchment EDA-3A: EDA-3A	Runoff Area=7.077 ac 0.00% Impervious Runoff Depth>0.25" Flow Length=1,591' Tc=41.2 min CN=31 Runoff=0.26 cfs 0.150 af
Subcatchment EDA-3B: EDA-3B	Runoff Area=1.263 ac 0.00% Impervious Runoff Depth>0.21" Tc=6.0 min CN=30 Runoff=0.04 cfs 0.022 af
Subcatchment EDA-4: EDA-4	Runoff Area=16.492 ac 0.00% Impervious Runoff Depth>0.49" Flow Length=747' Tc=32.6 min CN=35 Runoff=2.14 cfs 0.668 af
Subcatchment EDA-5: EDA-5	Runoff Area=20.756 ac 0.00% Impervious Runoff Depth>0.42" Flow Length=948' Tc=30.7 min CN=34 Runoff=2.11 cfs 0.734 af
Subcatchment EDA-6: EDA-6	Runoff Area=11.651 ac 0.00% Impervious Runoff Depth>0.83" Flow Length=1,264' Tc=45.3 min CN=40 Runoff=3.34 cfs 0.802 af
Subcatchment EDA-7: EDA-7	Runoff Area=3.198 ac 0.00% Impervious Runoff Depth>0.49" Flow Length=1,177' Tc=32.3 min CN=35 Runoff=0.42 cfs 0.129 af
Reach DP-1: DP-1	Inflow=2.44 cfs 0.360 af Outflow=2.44 cfs 0.360 af
Reach DP-2: DP-2	Inflow=9.69 cfs 1.890 af Outflow=9.69 cfs 1.890 af
Reach DP-3: DP-3	Inflow=0.30 cfs 0.172 af Outflow=0.30 cfs 0.172 af
Reach DP-4: DP-4	Inflow=2.14 cfs 0.668 af Outflow=2.14 cfs 0.668 af
Reach DP-5: DP-5	Inflow=2.11 cfs 0.734 af Outflow=2.11 cfs 0.734 af
Reach DP-6: DP-6	Inflow=15.20 cfs 3.354 af Outflow=15.20 cfs 3.354 af

Total Runoff Area = 79.208 ac Runoff Volume = 4.755 af Average Runoff Depth = 0.72" 100.00% Pervious = 79.208 ac 0.00% Impervious = 0.000 ac

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Summary for Subcatchment EDA-1: EDA-1

Runoff = 2.44 cfs @ 12.56 hrs, Volume= 0.360 af, Depth> 2.10"

Area	Area (ac) CN Description							
2.	052 5	55 Woo	ds, Good,	HSG B				
2.	052	100.	00% Pervi	ous Area				
Tc	Length	Slope (ft/ft)	Velocity (ft/sec)	Capacity	Description			
(min)	(feet)			(cfs)	Chast Flour			
15.8	50	0.0100	0.05		Sheet Flow,			
8.3	248	0.0100	0.50		Woods: Light underbrush n= 0.400 P2= 3.40" Shallow Concentrated Flow,			
0.5	240	0.0100	0.50		Woodland Kv= 5.0 fps			
4.2	178	0.0200	0.71		Shallow Concentrated Flow,			
	170	0.0200	0.7 1		Woodland Kv= 5.0 fps			
0.3	20	0.0500	1.12		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.7	34	0.0300	0.87		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.4	27	0.0400	1.00		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
1.3	55	0.0200	0.71		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
1.3	68	0.0300	0.87		Shallow Concentrated Flow,			
0.5	00	0.0400	4.00		Woodland Kv= 5.0 fps			
0.5	28	0.0400	1.00		Shallow Concentrated Flow,			
0.2	18	0.0600	1.22		Woodland Kv= 5.0 fps Shallow Concentrated Flow,			
0.2	10	0.0000	1.22		Woodland Kv= 5.0 fps			
0.6	31	0.0300	0.87		Shallow Concentrated Flow,			
0.0	01	0.0000	0.07		Woodland Kv= 5.0 fps			
0.3	22	0.0500	1.12		Shallow Concentrated Flow,			
0.0		0.0000			Woodland Kv= 5.0 fps			
0.6	31	0.0300	0.87		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.8	51	0.0400	1.00		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.3	22	0.0500	1.12		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.5	28	0.0400	1.00		Shallow Concentrated Flow,			
2.2		0.0700	4.00		Woodland Kv= 5.0 fps			
0.2	14	0.0700	1.32		Shallow Concentrated Flow,			
0.0	20	0.0500	4.40		Woodland Kv= 5.0 fps			
0.3	20	0.0500	1.12		Shallow Concentrated Flow,			
26.0	045	Total			Woodland Kv= 5.0 fps			
36.8	945	Total						

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Summary for Subcatchment EDA-2A: EDA-2A

Runoff = 2.04 cfs @ 13.06 hrs, Volume= 0.657 af, Depth> 0.61"

Area (ac)	CN	Description
10.340	30	Woods, Good, HSG A
1.499	77	Woods, Good, HSG D
0.425	55	Woods, Good, HSG B
0.668	39	>75% Grass cover, Good, HSG A
12.932	37	Weighted Average
12.932		100.00% Pervious Area

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	15.8	50	0.0100	0.05		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.40"
	7.1	213	0.0100	0.50		Shallow Concentrated Flow,
	4 7	004	0.0000	0.74		Woodland Kv= 5.0 fps
	4.7	201	0.0200	0.71		Shallow Concentrated Flow,
	8.0	338	0.0200	0.71		Woodland Kv= 5.0 fps Shallow Concentrated Flow,
	0.0	330	0.0200	0.71		Woodland Kv= 5.0 fps
	3.4	175	0.0300	0.87		Shallow Concentrated Flow,
	0.1	170	0.0000	0.07		Woodland Kv= 5.0 fps
	1.7	105	0.0400	1.00		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	4.4	188	0.0200	0.71		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.5	36	0.0600	1.22		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	1.2	60	0.0300	0.87		Shallow Concentrated Flow,
	0.0	40	0.0000	4 44		Woodland Kv= 5.0 fps
	0.2	13	0.0800	1.41		Shallow Concentrated Flow,
	1.6	96	0.0400	1.00		Woodland Kv= 5.0 fps
	1.0	90	0.0400	1.00		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
	1.3	85	0.0500	1.12		Shallow Concentrated Flow,
	1.0	00	0.0000	1.12		Woodland Kv= 5.0 fps
	0.2	15	0.0700	1.32		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	1.9	56	0.0100	0.50		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.4	13	0.0100	0.50		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	1.1	45	0.0200	0.71		Shallow Concentrated Flow,
	0.0	40	0.0000	4 44		Woodland Kv= 5.0 fps
	0.2	13	0.0800	1.41		Shallow Concentrated Flow,
	1.0	44	0.0200	0.71		Woodland Kv= 5.0 fps Shallow Concentrated Flow,
	1.0	44	0.0200	0.7 1		Woodland Kv= 5.0 fps
-	5/1.8	1 7/6	Total			Troodiana 137 0.0 ipo

54.8 1,746 Total

Summary for Subcatchment EDA-2B: EDA-2B

Runoff = 8.95 cfs @ 12.51 hrs, Volume= 1.233 af, Depth> 3.91"

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Area	(ac) C	N Desc	cription			
0.	.354 3	0 Woo	ds, Good,	HSG A		
3.433 77 Woods, Good, HSG D						
3.	.787 7	'3 Weig	hted Aver	age		
3.	.787	100.	00% Pervi	ous Area		
T .	1	01	M. I	0	December the co	
Tc (min)	Length	Slope	Velocity	Capacity	Description	
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)		
15.8	50	0.0100	0.05		Sheet Flow, Tc-1	
					Woods: Light underbrush n= 0.400 P2= 3.40"	
3.7	111	0.0100	0.50		Shallow Concentrated Flow, Tc-2	
					Woodland Kv= 5.0 fps	
2.1	107	0.0300	0.87		Shallow Concentrated Flow, Tc-3	
					Woodland Kv= 5.0 fps	
0.4	25	0.0400	1.00		Shallow Concentrated Flow, Tc-4	
					Woodland Kv= 5.0 fps	
14.9	282	0.0040	0.32		Shallow Concentrated Flow,	
					Woodland Kv= 5.0 fps	
36.9	575	Total				

Summary for Subcatchment EDA-3A: EDA-3A

Runoff = 0.26 cfs @ 14.06 hrs, Volume= 0.150 af, Depth> 0.25"

 Area (ac)	CN	Description
6.509	30	Woods, Good, HSG A
 0.568	39	>75% Grass cover, Good, HSG A
7.077	31	Weighted Average
7.077		100.00% Pervious Area

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Slope Velocity Capacity Description Тс Length (feet) (ft/ft) (ft/sec) (cfs) (min) 0.0200 10.6 43 0.07 Sheet Flow. Woods: Light underbrush n= 0.400 P2= 3.40" 2.1 7 0.0300 0.06 Sheet Flow. Woods: Light underbrush n= 0.400 P2= 3.40" 0.5 **Shallow Concentrated Flow.** 28 0.0300 0.87 Woodland Kv= 5.0 fps 1.3 54 0.0200 0.71 **Shallow Concentrated Flow,** Woodland Kv= 5.0 fps 0.6 33 0.0300 0.87 **Shallow Concentrated Flow,** Woodland Kv= 5.0 fps 2.1 **Shallow Concentrated Flow.** 88 0.0200 0.71 Woodland Kv= 5.0 fps 0.4 26 0.0400 1.00 **Shallow Concentrated Flow.** Woodland Kv= 5.0 fps 1.3 69 0.0300 0.87 **Shallow Concentrated Flow,** Woodland Kv= 5.0 fps 0.3 22 0.0500 1.12 **Shallow Concentrated Flow.** Woodland Kv= 5.0 fps 0.4 25 0.0400 1.00 **Shallow Concentrated Flow.** Woodland Kv= 5.0 fps 0.6 **Shallow Concentrated Flow,** 30 0.0300 0.87 Woodland Kv= 5.0 fps 0.9 0.0400 1.00 **Shallow Concentrated Flow.** 57 Woodland Kv= 5.0 fps 0.3 20 0.0500 1.12 **Shallow Concentrated Flow.** Woodland Kv= 5.0 fps **Shallow Concentrated Flow,** 1.0 43 0.0200 0.71 Woodland Kv= 5.0 fps **Shallow Concentrated Flow,** 0.4 26 0.0400 1.00 Woodland Kv= 5.0 fps 0.6 0.0300 0.87 **Shallow Concentrated Flow.** 31 Woodland Kv= 5.0 fps 0.4 26 0.0400 **Shallow Concentrated Flow,** 1.00 Woodland Kv= 5.0 fps 1.4 72 0.0300 0.87 **Shallow Concentrated Flow.** Woodland Kv= 5.0 fps 0.5 29 0.0400 1.00 **Shallow Concentrated Flow.** Woodland Kv= 5.0 fps **Shallow Concentrated Flow,** 0.3 20 0.0500 1.12 Woodland Kv= 5.0 fps 0.7 35 0.0300 0.87 **Shallow Concentrated Flow.** Woodland Kv= 5.0 fps **Shallow Concentrated Flow.** 0.9 52 0.0400 1.00 Woodland Kv= 5.0 fps 0.6 **Shallow Concentrated Flow,** 30 0.0300 0.87 Woodland Kv= 5.0 fps 0.4 23 0.0400 1.00 **Shallow Concentrated Flow.** Woodland Kv= 5.0 fps 2.0 **Shallow Concentrated Flow.** 103 0.0300 0.87 Woodland Kv= 5.0 fps 0.9 54 0.0400 1.00 **Shallow Concentrated Flow,**

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Woodland	Kv= 5	.0 fps		
0.3	19	0.0500	1.12	Shallow Concentrated Flow,
0.4	24	0.0400	1.00	Woodland Kv= 5.0 fps
0.4	24	0.0400	1.00	Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.2	52	0.0200	0.71	Shallow Concentrated Flow,
				Woodland Kv= 5.0 fps
1.3	84	0.0500	1.12	Shallow Concentrated Flow,
0.6	33	0.0300	0.87	Woodland Kv= 5.0 fps
0.6	33	0.0300	0.67	Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.8	50	0.0400	1.00	Shallow Concentrated Flow,
				Woodland Kv= 5.0 fps
0.6	31	0.0300	0.87	Shallow Concentrated Flow,
0.0	00	0.0500	4.40	Woodland Kv= 5.0 fps
0.3	23	0.0500	1.12	Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.4	71	0.0300	0.87	Shallow Concentrated Flow,
		0.0000	0.0.	Woodland Kv= 5.0 fps
0.3	23	0.0500	1.12	Shallow Concentrated Flow,
2.2			0.07	Woodland Kv= 5.0 fps
0.6	30	0.0300	0.87	Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.2	18	0.0600	1.22	Shallow Concentrated Flow,
0.2	10	0.0000	1.22	Woodland Kv= 5.0 fps
0.3	21	0.0500	1.12	Shallow Concentrated Flow,
				Woodland Kv= 5.0 fps
1.3	66	0.0300	0.87	Shallow Concentrated Flow,
44.0	1 504	Total		Woodland Kv= 5.0 fps
41.2	1,591	Total		

Summary for Subcatchment EDA-3B: EDA-3B

Runoff = 0.04 cfs @ 13.77 hrs, Volume= 0.022 af, Depth> 0.21"

Area	(ac)	CN	Desc	cription		
1.	1.263 30 Woods, Good, HSG A					
1.	1.263 100.00% Pervious Area					
_			01			
Tc	Leng	th S	Slope	Velocity	Capacity	Description
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
6.0						Direct Entry,

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Summary for Subcatchment EDA-4: EDA-4

Runoff = 2.14 cfs @ 12.76 hrs, Volume= 0.668 af, Depth> 0.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=7.00"

	Area	(ac) C	N Desc	cription		
13.411 30 Woods, Good, HS			ds, Good,	HSG A		
_	3.	081 5	55 Woo	ds, Good,	HSG B	
	16.	492 3	35 Weig	ghted Aver	age	
	16.	492	100.	00% Pervi	ous Area	
	То	Longth	Clone	Volocity	Consoity	Description
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	15.8	50	0.0100	0.05		Sheet Flow, Tc-1
						Woods: Light underbrush n= 0.400 P2= 3.40"
	5.2	258	0.0270	0.82		Shallow Concentrated Flow, Tc-2
						Woodland Kv= 5.0 fps
	11.6	439	0.0160	0.63		Shallow Concentrated Flow, Tc-3
_						Woodland Kv= 5.0 fps
	32.6	747	Total			

Summary for Subcatchment EDA-5: EDA-5

Runoff = 2.11 cfs @ 12.76 hrs, Volume= 0.734 af, Depth> 0.42"

_	Area	(ac) C	N Desc	cription		
				ds, Good,		
_	3.193 55 Woods, Good, HSG B 20.756 34 Weighted Average 20.756 100.00% Pervious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	12.0	50	0.0200	0.07	, ,	Sheet Flow, Tc-1
	7.2	306	0.0200	0.71		Woods: Light underbrush n= 0.400 P2= 3.40" Shallow Concentrated Flow, Tc-2 Woodland Kv= 5.0 fps
	6.1	342	0.0350	0.94		Shallow Concentrated Flow, Tc-3
	5.4	250	0.0240	0.77		Woodland Kv= 5.0 fps Shallow Concentrated Flow, Tc-4 Woodland Kv= 5.0 fps
	30.7	948	Total			

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Summary for Subcatchment EDA-6: EDA-6

Runoff = 3.34 cfs @ 12.82 hrs, Volume= 0.802 af, Depth> 0.83"

Area	Area (ac) CN Description							
1.	572 5	55 Woo	ds, Good,	HSG B				
8.	369 3	30 Woo	ds, Good,	HSG A				
1.	710 7	77 Woo	ds, Good,	HSG D				
11.	651 4	0 Weig	hted Aver	age				
11.	651	100.	00% Pervi	ous Area				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
15.8	50	0.0100	0.05		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.40"			
2.0	60	0.0100	0.50		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
8.7	371	0.0200	0.71		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
8.0	39	0.0300	0.87		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
1.4	60	0.0200	0.71		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.8	39	0.0300	0.87		Shallow Concentrated Flow,			
0.4	07	0.0400	4.00		Woodland Kv= 5.0 fps			
0.4	27	0.0400	1.00		Shallow Concentrated Flow,			
2.4	102	0.0200	0.71		Woodland Kv= 5.0 fps			
2.4	102	0.0200	0.71		Shallow Concentrated Flow, Woodland Kv= 5.0 fps			
0.7	35	0.0300	0.87		Shallow Concentrated Flow,			
0.7	33	0.0300	0.07		Woodland Kv= 5.0 fps			
3.3	139	0.0200	0.71		Shallow Concentrated Flow,			
0.0	100	0.0200	0.7 1		Woodland Kv= 5.0 fps			
0.7	36	0.0300	0.87		Shallow Concentrated Flow,			
• • • • • • • • • • • • • • • • • • • •		0.000	0.0.		Woodland Kv= 5.0 fps			
1.5	63	0.0200	0.71		Shallow Concentrated Flow,			
_					Woodland Kv= 5.0 fps			
0.7	36	0.0300	0.87		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
1.3	54	0.0200	0.71		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
3.5	104	0.0100	0.50		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
1.2	49	0.0200	0.71		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
45.3	1,264	Total						

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Summary for Subcatchment EDA-7: EDA-7

Runoff = 0.42 cfs @ 12.75 hrs, Volume= 0.129 af, Depth> 0.49"

 Area (ac)	CN	Description
1.357	30	Woods, Good, HSG A
 1.841	39	>75% Grass cover, Good, HSG A
3.198	35	Weighted Average
3.198		100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.2	38	0.0300	0.08		Sheet Flow,
0.0	40		0.05		Woods: Light underbrush n= 0.400 P2= 3.40"
3.8	12	0.0200	0.05		Sheet Flow,
0.2	7	0.0200	0.71		Woods: Light underbrush n= 0.400 P2= 3.40" Shallow Concentrated Flow,
0.2	,	0.0200	0.7 1		Woodland Kv= 5.0 fps
0.4	23	0.0400	1.00		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
1.0	42	0.0200	0.71		Shallow Concentrated Flow,
0.0	47	0.0000	4.00		Woodland Kv= 5.0 fps
0.2	17	0.0600	1.22		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.8	110	0.0400	1.00		Shallow Concentrated Flow,
1.0	110	0.0100	1.00		Woodland Kv= 5.0 fps
1.2	49	0.0200	0.71		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.7	37	0.0300	0.87		Shallow Concentrated Flow,
0.9	55	0.0400	1.00		Woodland Kv= 5.0 fps Shallow Concentrated Flow,
0.9	55	0.0400	1.00		Woodland Kv= 5.0 fps
0.6	33	0.0300	0.87		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
1.8	108	0.0400	1.00		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.7	37	0.0300	0.87		Shallow Concentrated Flow,
0.3	21	0.0500	1.12		Woodland Kv= 5.0 fps Shallow Concentrated Flow,
0.5	۷ ۱	0.0300	1.12		Woodland Kv= 5.0 fps
0.8	46	0.0400	1.00		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.6	33	0.0300	0.87		Shallow Concentrated Flow,
0.0	00	0.0500	4.40		Woodland Kv= 5.0 fps
0.3	22	0.0500	1.12		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.4	25	0.0400	1.00		Shallow Concentrated Flow,
• • •		0.0.00			Woodland Kv= 5.0 fps
0.2	17	0.0600	1.22		Shallow Concentrated Flow,
	•				Woodland Kv= 5.0 fps
0.3	21	0.0500	1.12		Shallow Concentrated Flow,
0.4	24	0.0400	1.00		Woodland Kv= 5.0 fps Shallow Concentrated Flow,
0.4	27	0.0400	1.00		Woodland Kv= 5.0 fps
0.3	19	0.0500	1.12		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
1.0	42	0.0200	0.71		Shallow Concentrated Flow,
0.0	F.C.	0.0400	4.00		Woodland Kv= 5.0 fps
0.9	56	0.0400	1.00		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.8	40	0.0300	0.87		Shallow Concentrated Flow,
3.0	.0	5.0000	3.01		Woodland Kv= 5.0 fps
0.6	37	0.0500	1.12		Shallow Concentrated Flow,

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Woodland	Kv= 5	.0 fps		
0.7	37	0.0300	0.87	Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.4	23	0.0400	1.00	Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.2	15	0.0700	1.32	Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.4	26	0.0400	1.00	Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.2	62	0.0300	0.87	Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.0	43	0.0200	0.71	Shallow Concentrated Flow, Woodland Kv= 5.0 fps
32.3	1 177	Total		

Summary for Reach DP-1: DP-1

Inflow Are	a =	2.052 ac,	0.00% Impervious, Inflo	w Depth > 2.10"	for 100-Year event
Inflow	=	2.44 cfs @	12.56 hrs, Volume=	0.360 af	
Outflow	=	2.44 cfs @	12.56 hrs, Volume=	0.360 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-2: DP-2

Inflow Area =	16.719 ac,	0.00% Impervious, Infl	ow Depth > 1.36"	for 100-Year event
Inflow =	9.69 cfs @	12.56 hrs, Volume=	1.890 af	
Outflow =	9.69 cfs @	12.56 hrs. Volume=	1.890 af. Atte	en= 0%. Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-3: DP-3

Inflow Are	a =	8.340 ac,	0.00% Impervious,	Inflow Depth > 0.	25" for 100-Year event
Inflow	=	0.30 cfs @	14.05 hrs, Volume	= 0.172 af	
Outflow	=	0.30 cfs @	14.05 hrs, Volume	= 0.172 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-4: DP-4

Inflow Are	ea =	16.492 ac,	0.00% Impervious,	Inflow Depth > 0.4	49" for 100-Year event
Inflow	=	2.14 cfs @	12.76 hrs, Volume	= 0.668 af	
Outflow	=	2.14 cfs @	12.76 hrs, Volume	= 0.668 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Summary for Reach DP-5: DP-5

Inflow Area = 20.756 ac, 0.00% Impervious, Inflow Depth > 0.42" for 100-Year event

Inflow = 2.11 cfs @ 12.76 hrs, Volume= 0.734 af

Outflow = 2.11 cfs @ 12.76 hrs, Volume= 0.734 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-6: DP-6

Inflow Area = 41.960 ac, 0.00% Impervious, Inflow Depth > 0.96" for 100-Year event

Inflow = 15.20 cfs @ 12.63 hrs, Volume= 3.354 af

Outflow = 15.20 cfs @ 12.63 hrs, Volume= 3.354 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-1: EDA-1	Runoff Area=2.052 ac 0.00% Impervious Runoff Depth>0.31" Flow Length=945' Tc=36.8 min CN=55 Runoff=0.20 cfs 0.053 af
Subcatchment EDA-2A: EDA-2A	Runoff Area=12.932 ac 0.00% Impervious Runoff Depth=0.00" Flow Length=1,746' Tc=54.8 min CN=37 Runoff=0.00 cfs 0.000 af
Subcatchment EDA-2B: EDA-2B	Runoff Area=3.787 ac 0.00% Impervious Runoff Depth>1.10" Flow Length=575' Tc=36.9 min CN=73 Runoff=2.39 cfs 0.348 af
Subcatchment EDA-3A: EDA-3A	Runoff Area=7.077 ac 0.00% Impervious Runoff Depth=0.00" Flow Length=1,591' Tc=41.2 min CN=31 Runoff=0.00 cfs 0.000 af
Subcatchment EDA-3B: EDA-3B	Runoff Area=1.263 ac 0.00% Impervious Runoff Depth=0.00" Tc=6.0 min CN=30 Runoff=0.00 cfs 0.000 af
Subcatchment EDA-4: EDA-4	Runoff Area=16.492 ac 0.00% Impervious Runoff Depth=0.00" Flow Length=747' Tc=32.6 min CN=35 Runoff=0.00 cfs 0.000 af
Subcatchment EDA-5: EDA-5	Runoff Area=20.756 ac 0.00% Impervious Runoff Depth=0.00" Flow Length=948' Tc=30.7 min CN=34 Runoff=0.00 cfs 0.000 af
Subcatchment EDA-6: EDA-6	Runoff Area=11.651 ac 0.00% Impervious Runoff Depth>0.01" Flow Length=1,264' Tc=45.3 min CN=40 Runoff=0.02 cfs 0.009 af
Subcatchment EDA-7: EDA-7	Runoff Area=3.198 ac 0.00% Impervious Runoff Depth=0.00" Flow Length=1,177' Tc=32.3 min CN=35 Runoff=0.00 cfs 0.000 af
Reach DP-1: DP-1	Inflow=0.20 cfs 0.053 af Outflow=0.20 cfs 0.053 af
Reach DP-2: DP-2	Inflow=2.39 cfs 0.348 af Outflow=2.39 cfs 0.348 af
Reach DP-3: DP-3	Inflow=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
Reach DP-4: DP-4	Inflow=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
Reach DP-5: DP-5	Inflow=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
Reach DP-6: DP-6	Inflow=2.56 cfs 0.409 af Outflow=2.56 cfs 0.409 af

Total Runoff Area = 79.208 ac Runoff Volume = 0.409 af Average Runoff Depth = 0.06" 100.00% Pervious = 79.208 ac 0.00% Impervious = 0.000 ac

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-1: EDA-1	Runoff Area=2.052 ac 0.00% Impervious Runoff Depth>0.82" Flow Length=945' Tc=36.8 min CN=55 Runoff=0.80 cfs 0.141 af
Subcatchment EDA-2A: EDA-2A	Runoff Area=12.932 ac 0.00% Impervious Runoff Depth>0.09" Flow Length=1,746' Tc=54.8 min CN=37 Runoff=0.15 cfs 0.094 af
Subcatchment EDA-2B: EDA-2B	Runoff Area=3.787 ac 0.00% Impervious Runoff Depth>2.03" Flow Length=575' Tc=36.9 min CN=73 Runoff=4.59 cfs 0.641 af
Subcatchment EDA-3A: EDA-3A	Runoff Area=7.077 ac 0.00% Impervious Runoff Depth>0.00" Flow Length=1,591' Tc=41.2 min CN=31 Runoff=0.01 cfs 0.001 af
Subcatchment EDA-3B: EDA-3B	Runoff Area=1.263 ac 0.00% Impervious Runoff Depth>0.00" Tc=6.0 min CN=30 Runoff=0.00 cfs 0.000 af
Subcatchment EDA-4: EDA-4	Runoff Area=16.492 ac 0.00% Impervious Runoff Depth>0.05" Flow Length=747' Tc=32.6 min CN=35 Runoff=0.10 cfs 0.066 af
Subcatchment EDA-5: EDA-5	Runoff Area=20.756 ac 0.00% Impervious Runoff Depth>0.03" Flow Length=948' Tc=30.7 min CN=34 Runoff=0.08 cfs 0.055 af
Subcatchment EDA-6: EDA-6	Runoff Area=11.651 ac 0.00% Impervious Runoff Depth>0.17" Flow Length=1,264' Tc=45.3 min CN=40 Runoff=0.28 cfs 0.163 af
Subcatchment EDA-7: EDA-7	Runoff Area=3.198 ac 0.00% Impervious Runoff Depth>0.05" Flow Length=1,177' Tc=32.3 min CN=35 Runoff=0.02 cfs 0.013 af
Reach DP-1: DP-1	Inflow=0.80 cfs 0.141 af Outflow=0.80 cfs 0.141 af
Reach DP-2: DP-2	Inflow=4.59 cfs 0.734 af Outflow=4.59 cfs 0.734 af
Reach DP-3: DP-3	Inflow=0.01 cfs 0.001 af Outflow=0.01 cfs 0.001 af
Reach DP-4: DP-4	Inflow=0.10 cfs 0.066 af Outflow=0.10 cfs 0.066 af
Reach DP-5: DP-5	Inflow=0.08 cfs 0.055 af Outflow=0.08 cfs 0.055 af
Reach DP-6: DP-6	Inflow=5.38 cfs 1.052 af Outflow=5.38 cfs 1.052 af

Total Runoff Area = 79.208 ac Runoff Volume = 1.172 af Average Runoff Depth = 0.18" 100.00% Pervious = 79.208 ac 0.00% Impervious = 0.000 ac

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-1: EDA-1	Runoff Area=2.052 ac 0.00% Impervious Runoff Depth>2.10" Flow Length=945' Tc=36.8 min CN=55 Runoff=2.44 cfs 0.360 af
Subcatchment EDA-2A: EDA-2A	Runoff Area=12.932 ac 0.00% Impervious Runoff Depth>0.61" Flow Length=1,746' Tc=54.8 min CN=37 Runoff=2.04 cfs 0.657 af
Subcatchment EDA-2B: EDA-2B	Runoff Area=3.787 ac 0.00% Impervious Runoff Depth>3.91" Flow Length=575' Tc=36.9 min CN=73 Runoff=8.95 cfs 1.233 af
Subcatchment EDA-3A: EDA-3A	Runoff Area=7.077 ac 0.00% Impervious Runoff Depth>0.25" Flow Length=1,591' Tc=41.2 min CN=31 Runoff=0.26 cfs 0.150 af
Subcatchment EDA-3B: EDA-3B	Runoff Area=1.263 ac 0.00% Impervious Runoff Depth>0.21" Tc=6.0 min CN=30 Runoff=0.04 cfs 0.022 af
Subcatchment EDA-4: EDA-4	Runoff Area=16.492 ac 0.00% Impervious Runoff Depth>0.49" Flow Length=747' Tc=32.6 min CN=35 Runoff=2.14 cfs 0.668 af
Subcatchment EDA-5: EDA-5	Runoff Area=20.756 ac 0.00% Impervious Runoff Depth>0.42" Flow Length=948' Tc=30.7 min CN=34 Runoff=2.11 cfs 0.734 af
Subcatchment EDA-6: EDA-6	Runoff Area=11.651 ac 0.00% Impervious Runoff Depth>0.83" Flow Length=1,264' Tc=45.3 min CN=40 Runoff=3.34 cfs 0.802 af
Subcatchment EDA-7: EDA-7	Runoff Area=3.198 ac 0.00% Impervious Runoff Depth>0.49" Flow Length=1,177' Tc=32.3 min CN=35 Runoff=0.42 cfs 0.129 af
Reach DP-1: DP-1	Inflow=2.44 cfs 0.360 af Outflow=2.44 cfs 0.360 af
Reach DP-2: DP-2	Inflow=9.69 cfs 1.890 af Outflow=9.69 cfs 1.890 af
Reach DP-3: DP-3	Inflow=0.30 cfs 0.172 af Outflow=0.30 cfs 0.172 af
Reach DP-4: DP-4	Inflow=2.14 cfs 0.668 af Outflow=2.14 cfs 0.668 af
Reach DP-5: DP-5	Inflow=2.11 cfs 0.734 af Outflow=2.11 cfs 0.734 af
Reach DP-6: DP-6	Inflow=15.20 cfs 3.354 af Outflow=15.20 cfs 3.354 af

Total Runoff Area = 79.208 ac Runoff Volume = 4.755 af Average Runoff Depth = 0.72" 100.00% Pervious = 79.208 ac 0.00% Impervious = 0.000 ac

Attachment 3
Post-Development Hydrologic Analysis



BEALS + THOMAS BEALS AND THOMAS, INC. Reservoir Corporate Center 144 Turnpike Road

Southborough, MA 01772-2104

CALCULATION SUMMARY

T 508.366.0560 F 508.366.4391 www.bealsandthomas.com Regional Office: Plymouth, MA

JOB NO./LOCATION:

1833.112 Wareham, MA

CLIENT/PROJECT:

Borrego Solar Systems, Inc. 150 Tihonet Road PV+ES Project

SUBJECT/TITLE:

Post-Development Hydrologic Calculations

OBJECTIVE OF CALCULATION:

• To determine the post-development peak rates of runoff from the site for the 2, 10, & 100-year storm events at design points DP-1 through DP-6.

CALCULATION METHOD(S):

- Runoff curve numbers (CN), time-of-concentration (Tc), and runoff rates were calculated based on TR-55 methodology.
- Autodesk Civil 3D 2019 computer program was utilized for digitizing ground cover areas.
- Peak runoff rates were computed using HydroCAD version 10.00.
- Peak runoff rates were rounded to the nearest tenth.

ASSUMPTIONS:

- The ground cover types were determined using MassGIS aerial imagery and hydrologic soil groups based on United States Department of Agriculture, NRCS Soil Survey map information.
- Watershed boundaries have been estimated based upon contour information depicted on the Topographic Plan as well as MassGIS contours for offsite areas outside limits of topographic plan.
- Wetland systems were included in the hydrologic analysis and modeled as Woods Good.

SOURCES OF DATA/EQUATIONS:

- Post-Development Conditions Hydrologic Areas Map prepared by Beals and Thomas, Inc. File No. 1833112P594B-002.
- Existing topography from Limited Alta/ NSPS Land Title Survey of Land in Wareham, MA (1 Sheet), prepared by Northeast Survey Consultants.
- NRCS Soil Survey for Plymouth County, hydrologic soil group report, downloaded from Web Soil Survey on 3/12/2020.
- TR-55 urban Hydrology for Small Watersheds, SCS, 1986.
- Massachusetts DEP Stormwater Management Handbook, February 2008.

REV	CALC. BY	DATE	CHECKED BY	DATE	APPROVED BY	DATE
0	EAE	5/29/2020	J. Murphy	06/02/2020	J. Murphy	06/02/2020
1	N. Bautz	10/30/2020	J. Murphy	11/03/2020	J. Murphy	11/03/2020

NBB/1833112CS005B



BEALS + THOMAS BEALS AND THOMAS, INC. Reservoir Corporate Center 144 Turnpike Road

Southborough, MA 01772-2104

CALCULATION SUMMARY

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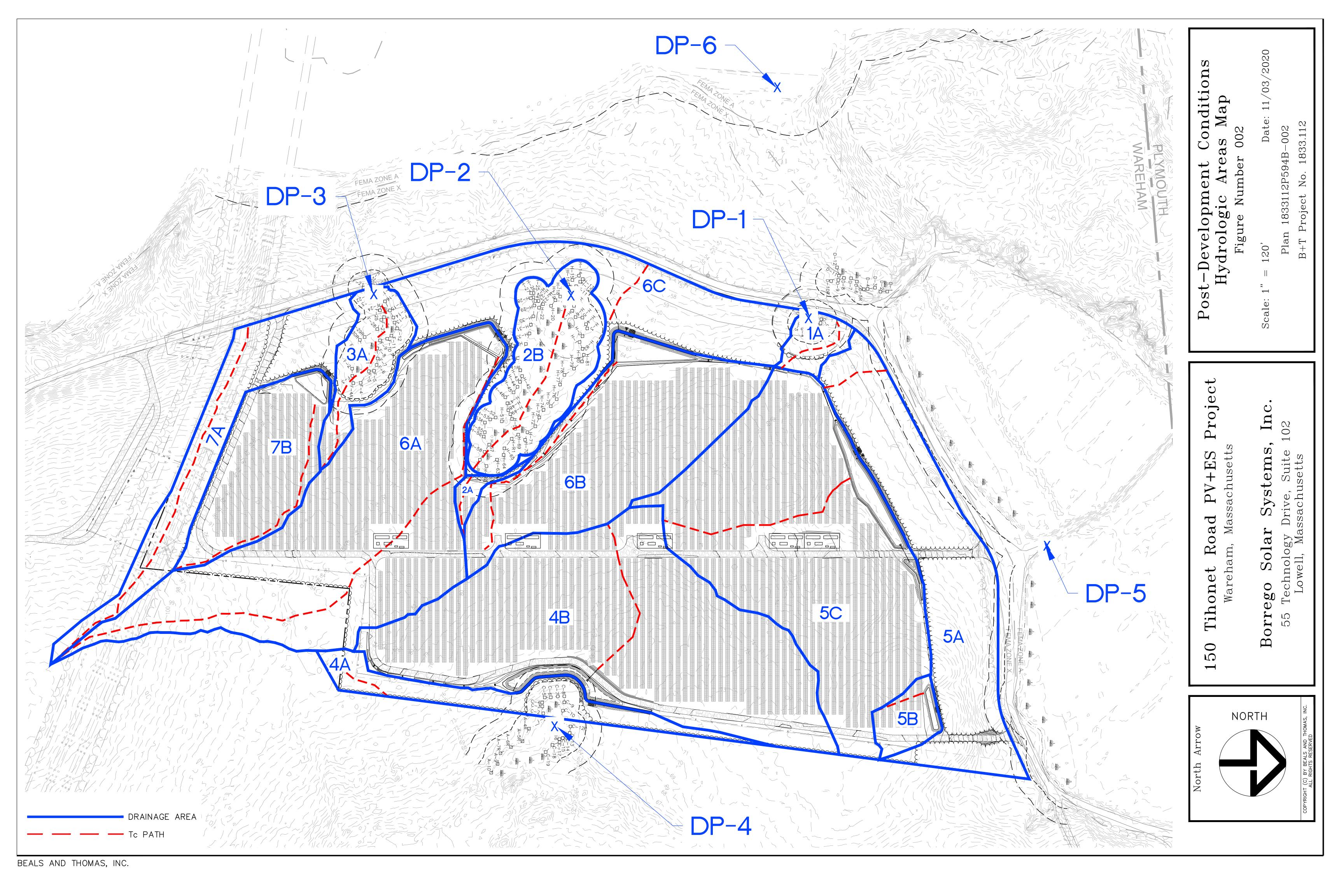
CONCLUSIONS:

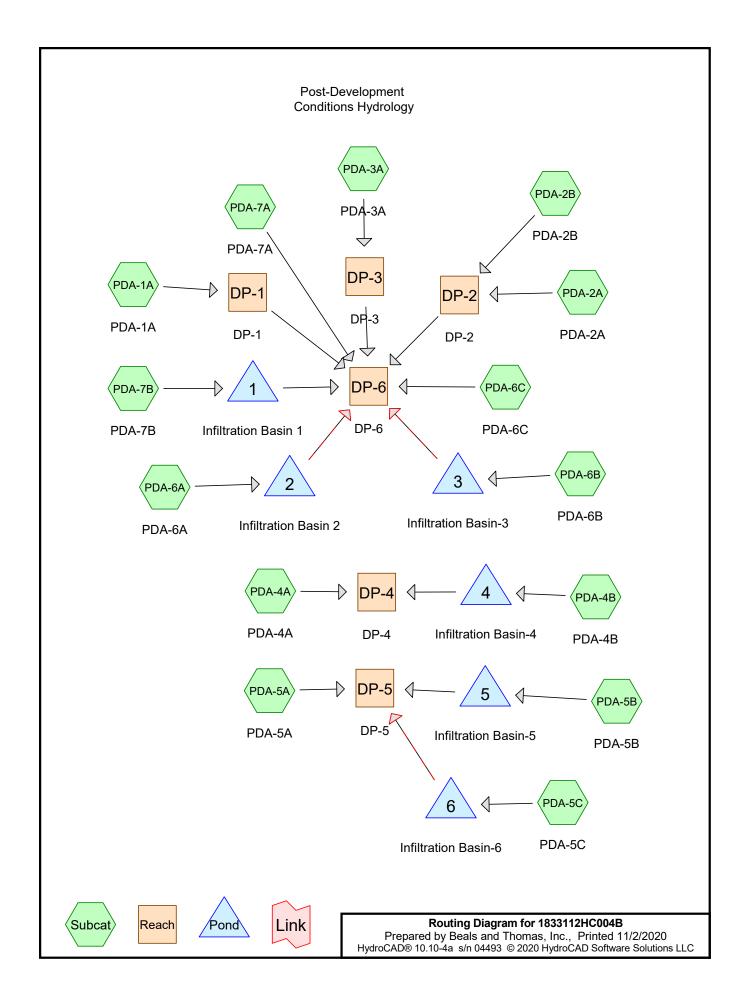
Storm Event	DP-1 (CFS)	DP-2 (CFS)	DP-3 (CFS)	DP-4 (CFS)	DP-5 (CFS)	DP-6 (CFS)
2-Year	0.1	2.3	0.0	0.0	0.0	2.3
10-Year	0.5	4.4	0.0	0.0	0.0	4.6
100-Year	1.6	8.8	0.1	0.1	1.0	12.8

REV	CALC. BY	DATE	CHECKED BY	DATE	APPROVED BY	DATE
0	EAE	5/29/2020	J. Murphy	06/02/2020	J. Murphy	06/02/2020
1	N. Bautz	10/30/2020	J. Murphy	11/03/2020	J. Murphy	11/03/2020

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Area Listing (selected nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
37.605	39	>75% Grass cover, Good, HSG A (PDA-2A, PDA-3A, PDA-4A, PDA-4B, PDA-5A,
		PDA-5B, PDA-5C, PDA-6A, PDA-6B, PDA-6C, PDA-7A, PDA-7B)
8.590	61	>75% Grass cover, Good, HSG B (PDA-1A, PDA-4A, PDA-4B, PDA-5A, PDA-5C,
		PDA-6B)
2.181	80	>75% Grass cover, Good, HSG D (PDA-2A, PDA-6A, PDA-6B, PDA-6C)
4.329	30	Brush, Good, HSG A (PDA-2A, PDA-3A, PDA-4A, PDA-4B, PDA-5A, PDA-5B,
		PDA-5C, PDA-6A, PDA-6C, PDA-7A, PDA-7B)
0.148	48	Brush, Good, HSG B (PDA-1A, PDA-4A, PDA-5A, PDA-6C)
0.280	73	Brush, Good, HSG D (PDA-2A, PDA-5A, PDA-6A, PDA-6B, PDA-6C)
0.147	98	Equipment Pad Area (PDA-4B, PDA-5C, PDA-6A, PDA-6B)
3.205	96	Gravel surface (PDA-4B, PDA-5A, PDA-5B, PDA-5C, PDA-6A, PDA-7A, PDA-7B)
0.093	96	Gravel surface, (PDA-6B)
17.269	30	Woods, Good, HSG A (PDA-1A, PDA-2B, PDA-3A, PDA-4A, PDA-5A, PDA-6A,
		PDA-6C, PDA-7A)
1.103	55	Woods, Good, HSG B (PDA-1A, PDA-5A, PDA-6C)
4.258	77	Woods, Good, HSG D (PDA-2B, PDA-5A, PDA-6C)
79.208	45	TOTAL AREA

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Outflow=2.25 cfs 0.330 af

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PDA-1A: PDA-1A	Runoff Area=0.741 ac 0.00% Impervious Runoff Depth>0.31" Flow Length=273' Tc=7.8 min CN=55 Runoff=0.11 cfs 0.019 af
Subcatchment PDA-2A: PDA-2A	Runoff Area=0.292 ac 0.00% Impervious Runoff Depth>0.00" Flow Length=216' Tc=7.5 min CN=39 Runoff=0.00 cfs 0.000 af
Subcatchment PDA-2B: PDA-2B	Runoff Area=3.787 ac 0.00% Impervious Runoff Depth>1.05" Flow Length=575' Tc=36.9 min CN=72 Runoff=2.25 cfs 0.330 af
Subcatchment PDA-3A: PDA-3A	Runoff Area=1.888 ac 0.00% Impervious Runoff Depth=0.00" Flow Length=644' Tc=17.5 min CN=32 Runoff=0.00 cfs 0.000 af
Subcatchment PDA-4A: PDA-4A	Runoff Area=2.202 ac 0.00% Impervious Runoff Depth=0.00" Flow Length=198' Tc=12.7 min CN=32 Runoff=0.00 cfs 0.000 af
Subcatchment PDA-4B: PDA-4B	Runoff Area=14.295 ac 0.30% Impervious Runoff Depth>0.12" Flow Length=540' Tc=17.1 min CN=48 Runoff=0.26 cfs 0.149 af
Subcatchment PDA-5A: PDA-5A	Runoff Area=7.393 ac 0.00% Impervious Runoff Depth=0.00" Flow Length=216' Tc=12.0 min CN=34 Runoff=0.00 cfs 0.000 af
Subcatchment PDA-5B: PDA-5B	Runoff Area=0.850 ac 0.00% Impervious Runoff Depth>0.05" Flow Length=177' Tc=7.4 min CN=44 Runoff=0.01 cfs 0.004 af
Subcatchment PDA-5C: PDA-5C	Runoff Area=13.219 ac 0.48% Impervious Runoff Depth>0.12" Flow Length=677' Tc=14.8 min CN=48 Runoff=0.24 cfs 0.138 af
Subcatchment PDA-6A: PDA-6A	Runoff Area=12.931 ac 0.22% Impervious Runoff Depth>0.02" Flow Length=1,965' Tc=40.5 min CN=41 Runoff=0.03 cfs 0.018 af
Subcatchment PDA-6B: PDA-6B	Runoff Area=7.472 ac 0.15% Impervious Runoff Depth>0.31" Flow Length=818' Tc=12.1 min CN=55 Runoff=1.02 cfs 0.194 af
Subcatchment PDA-6C: PDA-6C	Runoff Area=6.153 ac 0.00% Impervious Runoff Depth>0.00" Flow Length=222' Tc=15.8 min CN=39 Runoff=0.01 cfs 0.002 af
Subcatchment PDA-7A: PDA-7A	Runoff Area=4.103 ac 0.00% Impervious Runoff Depth=0.00" Flow Length=1,265' Tc=25.6 min CN=37 Runoff=0.00 cfs 0.000 af
Subcatchment PDA-7B: PDA-7B	Runoff Area=3.882 ac 0.00% Impervious Runoff Depth>0.04" Flow Length=795' Tc=15.2 min CN=43 Runoff=0.02 cfs 0.013 af
Reach DP-1: DP-1	Inflow=0.11 cfs 0.019 af Outflow=0.11 cfs 0.019 af
Reach DP-2: DP-2	Inflow=2.25 cfs 0.330 af

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Reach DP-3: DP-3	Inflow=0.00 cfs 0.000 af
	Outflow=0.00 cfs 0.000 af
Reach DP-4: DP-4	Inflow=0.00 cfs 0.000 af
	Outflow=0.00 cfs 0.000 af
Reach DP-5: DP-5	Inflow=0.00 cfs 0.000 af
	Outflow=0.00 cfs 0.000 af
Reach DP-6: DP-6	Inflow=2.32 cfs 0.352 af
	Outflow=2.32 cfs 0.352 af
Pond 1: Infiltration Basin 1 Peak Ele	ev=70.00' Storage=7 cf Inflow=0.02 cfs 0.013 af
Discarded=0.02 cfs 0.013 af Prima	ary=0.00 cfs 0.000 af Outflow=0.02 cfs 0.013 af
Pond 2: Infiltration Basin 2 Peak Elev	/=64.00' Storage=16 cf Inflow=0.03 cfs 0.018 af
Discarded=0.03 cfs 0.018 af Primary=0.00 cfs 0.000 af Second	ary=0.00 cfs 0.000 af Outflow=0.03 cfs 0.018 af
Pond 3: Infiltration Basin-3 Peak Elev=	=62.06' Storage=651 cf Inflow=1.02 cfs 0.194 af
Discarded=0.62 cfs 0.193 af Primary=0.00 cfs 0.000 af Seconda	ary=0.00 cfs 0.000 af Outflow=0.62 cfs 0.193 af
Pond 4: Infiltration Basin-4 Peak Elev=	-66.53' Storage=158 cf Inflow=0.26 cfs 0.149 af
Discarded=0.25 cfs 0.147 af Prima	ary=0.00 cfs 0.000 af Outflow=0.25 cfs 0.147 af
Pond 5: Infiltration Basin-5 Peak Ele	ev=70.00' Storage=2 cf Inflow=0.01 cfs 0.004 af
Discarded=0.01 cfs 0.004 af Prima	ary=0.00 cfs 0.000 af Outflow=0.01 cfs 0.004 af
	=57.01' Storage=126 cf Inflow=0.24 cfs 0.138 af
Discarded=0.24 cfs 0.137 af Primary=0.00 cfs 0.000 af Second	ary=0.00 cfs 0.000 af Outflow=0.24 cfs 0.137 af

Total Runoff Area = 79.208 ac Runoff Volume = 0.866 af Average Runoff Depth = 0.13" 99.81% Pervious = 79.061 ac 0.19% Impervious = 0.147 ac

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Summary for Subcatchment PDA-1A: PDA-1A

Runoff = 0.11 cfs @ 12.32 hrs, Volume= 0.019 af, Depth> 0.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.40"

Area ((ac) C	N Des	cription				
0.	001 3	30 Woo	Woods, Good, HSG A				
0.	725	55 Woo	ods, Good,	HSG B			
0.	012 4	48 Brus	sh, Good, I	HSG B			
0.	003 6	31 >75°	% Grass c	over, Good	, HSG B		
0.	741	55 Wei	ghted Avei	age			
0.	741	100.	00% Pervi	ous Area			
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
4.1	50	0.0400	0.20		Sheet Flow,		
					Grass: Short n= 0.150 P2= 3.40"		
1.6	88	0.0340	0.92		Shallow Concentrated Flow,		
					Woodland Kv= 5.0 fps		
1.2	73	0.0410	1.01		Shallow Concentrated Flow,		
					Woodland Kv= 5.0 fps		
0.9	62	0.0480	1.10		Shallow Concentrated Flow,		
					Woodland Kv= 5.0 fps		
7.8	273	Total					

Summary for Subcatchment PDA-2A: PDA-2A

Runoff = 0.00 cfs @ 23.46 hrs, Volume= 0.000 af, Depth> 0.00"

	Area	(ac)	CN	Desc	cription		
0.051 30 Brush, Good, HSG A							
	0.	016	73	Brus	h, Good, F	ISG D	
	0.	224	39	>75%	% Grass co	over, Good	, HSG A
_	0.	001	80	>75%	% Grass co	over, Good	, HSG D
	0.	292	39	Weig	ghted Aver	age	
	0.	292		100.	00% Pervi	ous Area	
	Tc (min)	Lengtl (feet		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	5.5	50	0 0	.0200	0.15		Sheet Flow,
	2.0	160	6 0	.0390	1.38		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	7.5	210	3 T	otal		•	

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Summary for Subcatchment PDA-2B: PDA-2B

Runoff = 2.25 cfs @ 12.56 hrs, Volume= 0.330 af, Depth> 1.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.40"

_	Area	(ac) C	N Desc	cription		
				ds, Good,		
_	3.	396 7	77 Woo	ds, Good,	HSG D	
	3.	787 7	'2 Wei	ghted Aver	age	
	3.	787	100.	00% Pervi	ous Area	
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.8	50	0.0100	0.05		Sheet Flow, Tc-1
						Woods: Light underbrush n= 0.400 P2= 3.40"
	3.7	111	0.0100	0.50		Shallow Concentrated Flow, Tc-2
						Woodland Kv= 5.0 fps
	2.1	107	0.0300	0.87		Shallow Concentrated Flow, Tc-3
						Woodland Kv= 5.0 fps
	0.4	25	0.0400	1.00		Shallow Concentrated Flow, Tc-4
						Woodland Kv= 5.0 fps
	14.9	282	0.0040	0.32		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	36.9	575	Total			

Summary for Subcatchment PDA-3A: PDA-3A

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Area (ac)	CN	Description
1.372	30	Woods, Good, HSG A
0.143	30	Brush, Good, HSG A
 0.373	39	>75% Grass cover, Good, HSG A
1.888	32	Weighted Average
1.888		100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.5	, ,	0.0400	0.18	(010)	Sheet Flow,
2.0	21	0.0400	0.10		Grass: Short n= 0.150 P2= 3.40"
2.9	23	0.0200	0.13		Sheet Flow,
2.0	20	0.0200	0.10		Grass: Short n= 0.150 P2= 3.40"
0.4	23	0.0200	0.99		Shallow Concentrated Flow,
0.1	20	0.0200	0.00		Short Grass Pasture Kv= 7.0 fps
0.9	64	0.0300	1.21		Shallow Concentrated Flow,
0.0	01	0.0000	1.21		Short Grass Pasture Kv= 7.0 fps
0.3	37	0.0800	1.98		Shallow Concentrated Flow,
0.0	0.	0.0000			Short Grass Pasture Kv= 7.0 fps
0.3	30	0.0700	1.85		Shallow Concentrated Flow,
0.0		0.0.00			Short Grass Pasture Kv= 7.0 fps
0.3	32	0.0600	1.71		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.3	28	0.0400	1.40		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
2.5	129	0.0300	0.87		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.5	28	0.0400	1.00		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
1.1	48	0.0200	0.71		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.3	20	0.0500	1.12		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
5.2	155	0.0100	0.50		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
17.5	644	Total			

Summary for Subcatchment PDA-4A: PDA-4A

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

 Area (ac)	CN	Description
0.576	30	Woods, Good, HSG A
1.376	30	Brush, Good, HSG A
0.115	48	Brush, Good, HSG B
0.122	39	>75% Grass cover, Good, HSG A
 0.013	61	>75% Grass cover, Good, HSG B
 2.202	32	Weighted Average
2.202		100.00% Pervious Area

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 Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.1	16	0.0300	0.04		Sheet Flow,
					Woods: Dense underbrush n= 0.800 P2= 3.40"
3.4	34	0.0300	0.17		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.40"
1.6	90	0.0170	0.91		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.6	58	0.0600	1.71		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
12.7	198	Total			

Summary for Subcatchment PDA-4B: PDA-4B

Runoff = 0.26 cfs @ 13.76 hrs, Volume= 0.149 af, Depth> 0.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.40"

	Area	(ac) (CN [Desc	cription		
	0.	540	30 E	Brus	h, Good, F	ISG A	
	9.	812	39 >	>759	% Grass co	over, Good	, HSG A
	2.690 61 >75% Grass cover, Good,						, HSG B
* 1.210 96 Gravel surface							
*	0.	043	98 E	Equi	pment Pac	d Area	
			48 \	Weig	ghted Aver	age	
		252	(99.7	0% Pervio	us Area	
	0.	043	(0.30	% Impervi	ous Area	
	_		0.1				
	Tc	Length		ppe	Velocity	Capacity	Description
	(min)	(feet)	(ft	t/ft)	(ft/sec)	(cfs)	
	7.2	50	0.01	100	0.12		Sheet Flow,
							Grass: Short n= 0.150 P2= 3.40"
	3.7	258	0.02	270	1.15		Shallow Concentrated Flow,
							Short Grass Pasture Kv= 7.0 fps
	6.2	232	0.00	080	0.63		Shallow Concentrated Flow,
							Short Grass Pasture Kv= 7.0 fps
	17 1	540	Tota	al			

Summary for Subcatchment PDA-5A: PDA-5A

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

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	Area	(ac)	CN	Desc	cription		
	6.	130	30	Woo	ds, Good,	HSG A	
	0.	164	55	Woo	ds, Good,	HSG B	
	0.	104	77	Woo	ds, Good,	HSG D	
	0.	561	30	Brus	h, Good, H	HSG A	
	0.	019	48	Brus	h, Good, H	HSG B	
	0.	006	73	Brus	h, Good, F	HSG D	
		134	39			over, Good	
		009	61			over, Good	, HSG B
*	0.	266	96	Grav	<u>el surface</u>		
	7.	393	34	Weig	ghted Aver	age	
	7.	393		100.	00% Pervi	ous Area	
	Tc	Lengt		Slope	Velocity	Capacity	Description
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	2.9	1	6 (0.0100	0.09		Sheet Flow,
							Grass: Short n= 0.150 P2= 3.40"
	6.4	3	4 (0.0440	0.09		Sheet Flow,
							Woods: Light underbrush n= 0.400 P2= 3.40"
	2.7	16	6 (0.0420	1.02		Shallow Concentrated Flow,
							Woodland Kv= 5.0 fps
	12.0	21	6 7	Γotal			

Summary for Subcatchment PDA-5B: PDA-5B

Runoff = 0.01 cfs @ 15.22 hrs, Volume= 0.004 af, Depth> 0.05"

	Area	(ac)	CN	Desc	cription			
	0.067 30 Brush, Good, HSG A							
	0.	700	39	>75%	% Grass co	over, Good	, HSG A	
*	0.	083	96	Grav	el surface			
	0.850 44 Weighted Average							
	0.	850		100.0	00% Pervi	ous Area		
	Tc	Lengt	h :	Slope	Velocity	Capacity	Description	
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)		
	5.5	5	0 0	.0200	0.15		Sheet Flow,	
							Grass: Short n= 0.150 P2= 3.40"	
	1.9	12	7 0.	.0260	1.13		Shallow Concentrated Flow,	
							Short Grass Pasture Kv= 7.0 fps	
	7.4	17	7 T	otal				

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Summary for Subcatchment PDA-5C: PDA-5C

Runoff = 0.24 cfs @ 12.98 hrs, Volume= 0.138 af, Depth> 0.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.40"

	Area	(ac)	CN	Desc	cription		
	0.	171	30	Brus	h, Good, F	HSG A	
	8.	834	39	>75%	% Grass co	over, Good	, HSG A
	3.	489	61			over, Good	, HSG B
*	0.	661	96	_	el surface		
*	0.	064	98	Equi	pment Pac	d Area	
	13.	219	48	Weig	ghted Aver	age	
	_	155		99.5	2% Pervio	us Area	
	0.	064		0.48	% Impervi	ous Area	
	_		_				
	Tc	Lengt		Slope	Velocity	Capacity	Description
	(min)	(fee		(ft/ft)	(ft/sec)	(cfs)	
	5.5	5	0 0	0.0200	0.15		Sheet Flow,
							Grass: Short n= 0.150 P2= 3.40"
	5.2	30	6 (0.0200	0.99		Shallow Concentrated Flow,
							Short Grass Pasture Kv= 7.0 fps
	4.1	32	1 (0.0350	1.31		Shallow Concentrated Flow,
							Short Grass Pasture Kv= 7.0 fps
	14.8	67	7 1	Γotal			

Summary for Subcatchment PDA-6A: PDA-6A

Runoff = 0.03 cfs @ 21.55 hrs, Volume= 0.018 af, Depth> 0.02"

	Area (ac)	CN	Description				
	2.760	30	Woods, Good, HSG A				
	0.849	30	Brush, Good, HSG A				
	0.002	73	Brush, Good, HSG D				
	8.077	39	>75% Grass cover, Good, HSG A				
	0.650	80	>75% Grass cover, Good, HSG D				
*	0.564	96	Gravel surface				
*	0.029	98	Equipment Pad Area				
	12.931	41	Weighted Average				
	12.902		99.78% Pervious Area				
	0.029		0.22% Impervious Area				

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	10.2	50	0.0300	0.08	(0.0)	Sheet Flow,
	10.2	00	0.0000	0.00		Woods: Light underbrush n= 0.400 P2= 3.40"
	0.8	38	0.0260	0.81		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	3.5	257	0.0310	1.23		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	8.6	484	0.0350	0.94		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	6.1	202	0.0120	0.55		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.9	52	0.0190	0.96		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	1.1	58	0.0170	0.91		Shallow Concentrated Flow,
	4.0	4.40	0.0440	4.00		Short Grass Pasture Kv= 7.0 fps
	1.2	140	0.0140	1.90		Shallow Concentrated Flow,
	- 4	045	0.0000	4.04		Unpaved Kv= 16.1 fps
	5.1	315	0.0220	1.04		Shallow Concentrated Flow,
	0.0	000	0.0400	0.07		Short Grass Pasture Kv= 7.0 fps
	3.0	369	0.0190	2.07		Shallow Concentrated Flow,
_						Grassed Waterway Kv= 15.0 fps
	40.5	1,965	Total			

Summary for Subcatchment PDA-6B: PDA-6B

Runoff = 1.02 cfs @ 12.39 hrs, Volume= 0.194 af, Depth> 0.31"

	Area	(ac) (CN	Desc	ription		
_		045	73			16C D	
					h, Good, F		
3.466 39 >75% Grass cover, Good, HSG A							
	2.	386	61	>75%	ն Grass co	over, Good	, HSG B
	1.	471	80	>75%	% Grass co	over, Good	, HSG D
*	· · · ·						
*	0.	011	98		pment Pac	,	
	7.	472	55	Weig	hted Aver	age	
	7.	461		99.8	5% Pervio	us Area	
	0.011 0.15% Impervious Area					ous Area	
					•		
	Tc	Length	S	Slope	Velocity	Capacity	Description
	(min)	(feet)		(ft/ft)	(ft/sec)	(cfs)	·
	4.6	50	0.0	0300	0.18	,	Sheet Flow,
			-				Grass: Short n= 0.150 P2= 3.40"
	2.2	179	0.0	0360	1.33		Shallow Concentrated Flow,
			0.0		1.00		Short Grass Pasture Kv= 7.0 fps
	5.3	589	0.0	0150	1.84		Shallow Concentrated Flow,
	5.0	000	0.0		1.01		Grassed Waterway Kv= 15.0 fps
	10.1	010	т.	tal			Cladda tratorway itt io.o ipo
	12.1	818	- 10	otal			

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Summary for Subcatchment PDA-6C: PDA-6C

Runoff = 0.01 cfs @ 23.59 hrs, Volume= 0.002 af, Depth> 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.40"

_	Area	(ac)	CN	Desc	cription		
	4.	594	30	Woo	ds, Good,		
	0.	214	55	Woo	ds, Good,	HSG B	
	0.	758	77	Woo	ds, Good,	HSG D	
	0.	258	30	Brus	h, Good, F	HSG A	
	0.	002	48	Brus	h, Good, F	HSG B	
	0.	211	73	Brus	h, Good, F	HSG D	
	0.	057	39	>75%	% Grass co	over, Good	, HSG A
_	0.	059	80	>75%	% Grass co	over, Good	, HSG D
	6.	153	39	Weig	ghted Aver	age	
	6.	153		100.	00% Pervi	ous Area	
	Tc	Length	· S	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	12.0	50	0.	0200	0.07		Sheet Flow,
							Woods: Light underbrush n= 0.400 P2= 3.40"
	3.8	172	2 0.	0230	0.76		Shallow Concentrated Flow,
_							Woodland Kv= 5.0 fps
	15.8	222	. To	otal	•		

Summary for Subcatchment PDA-7A: PDA-7A

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

_	Area (ac)	CN	Description				
	1.445	30	Woods, Good, HSG A				
	0.240	30	Brush, Good, HSG A				
	2.280	39	>75% Grass cover, Good, HSG A				
	0.138	96	Gravel surface				
	4.103	37	Weighted Average				
	4.103		100.00% Pervious Area				

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	10.2	50	0.0300	0.08	, ,	Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.40"
	1.5	59	0.0170	0.65		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	13.9	1,156	0.0390	1.38		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	25.6	1,265	Total			

Summary for Subcatchment PDA-7B: PDA-7B

Runoff = 0.02 cfs @ 15.67 hrs, Volume= 0.013 af, Depth> 0.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.40"

_	Area	(ac) C	N Des	cription		
	0.	073 3	30 Brus	sh, Good, H	HSG A	
	3.	526	39 >75	% Grass c	over, Good	, HSG A
*	0.	283 9	96 Grav	vel surface		
	3.	882 4	43 Wei	ghted Avei	age	
	3.	882	100.	00% Pervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.5	50	0.0200	0.15		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.40"
	1.1	93	0.0430	1.45		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.2	33	0.0200	2.28		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	8.4	619	0.0310	1.23		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	15.2	795	Total			

Summary for Reach DP-1: DP-1

Inflow Area = 0.741 ac, 0.00% Impervious, Inflow Depth > 0.31" for 2-Year event

Inflow = 0.11 cfs @ 12.32 hrs, Volume= 0.019 af

Outflow = 0.11 cfs @ 12.32 hrs, Volume= 0.019 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Summary for Reach DP-2: DP-2

Inflow Area = 4.079 ac, 0.00% Impervious, Inflow Depth > 0.97" for 2-Year event

Inflow = 2.25 cfs @ 12.56 hrs, Volume= 0.330 af

Outflow = 2.25 cfs @ 12.56 hrs, Volume= 0.330 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-3: DP-3

Inflow Area = 1.888 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Year event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-4: DP-4

Inflow Area = 16.497 ac, 0.26% Impervious, Inflow Depth = 0.00" for 2-Year event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-5: DP-5

Inflow Area = 21.462 ac, 0.30% Impervious, Inflow Depth = 0.00" for 2-Year event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-6: DP-6

Inflow Area = 41.249 ac, 0.10% Impervious, Inflow Depth > 0.10" for 2-Year event

Inflow = 2.32 cfs @ 12.55 hrs, Volume= 0.352 af

Outflow = 2.32 cfs @ 12.55 hrs, Volume= 0.352 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Pond 1: Infiltration Basin 1

Inflow Area = 3.882 ac, 0.00% Impervious, Inflow Depth > 0.04" for 2-Year event

Inflow = 0.02 cfs @ 15.67 hrs, Volume= 0.013 af

Outflow = 0.02 cfs @ 15.76 hrs, Volume= 0.013 af, Atten= 0%, Lag= 5.8 min

Discarded = 0.02 cfs @ 15.76 hrs, Volume= 0.013 af Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Volume

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Peak Elev= 70.00' @ 15.76 hrs Surf.Area= 5,631 sf Storage= 7 cf

Plug-Flow detention time= 5.9 min calculated for 0.013 af (99% of inflow)

Center-of-Mass det. time= 3.5 min (1,128.0 - 1,124.5)

Volume	Invert	Avail.Sto	rage Storage [Description	
#1	70.00'	14,70	3 cf Custom	Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio (fee 70.0 71.0 72.0)0 00 00	urf.Area (sq-ft) 5,630 6,602 10,572	Inc.Store (cubic-feet) 0 6,116 8,587	Cum.Store (cubic-feet) 0 6,116 14,703	
Device #1 #2	Routing Discarded Primary	70.00' 71.00'	Outlet Devices 2.410 in/hr Ext 20.0' long x 0	filtration over	Surface area pad-Crested Rectangular Weir
,,_	·····ary	71.00	Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32		

Discarded OutFlow Max=0.31 cfs @ 15.76 hrs HW=70.00' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.31 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=70.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2: Infiltration Basin 2

Inflow Area =	12.931 ac,	0.22% Impervious, Inflow D	epth > 0.02" for 2-Year event
Inflow =	0.03 cfs @	21.55 hrs, Volume=	0.018 af
Outflow =	0.03 cfs @	21.68 hrs, Volume=	0.018 af, Atten= 0%, Lag= 8.2 min
Discarded =	0.03 cfs @	21.68 hrs, Volume=	0.018 af
Primary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 64.00' @ 21.68 hrs Surf.Area= 4,964 sf Storage= 16 cf

Plug-Flow detention time= 8.8 min calculated for 0.018 af (98% of inflow) Center-of-Mass det. time= 4.5 min (1,209.8 - 1,205.3)

Avail Storage Storage Description

VOIGITIC	IIIVOIT / TV	an.otorage	Otorag	2 Description		
#1	64.00'	28,548 cf	Custor	n Stage Data (Pris	matic) Listed b	elow (Recal
Elevation (feet)	Surf.Area (sa-ft		:.Store c-feet)	Cum.Store (cubic-feet)		
		, , , , , , , , , , , , , , , , , , , ,	c-ieet)	(cubic-leet)		
64.00	4,958		0	0		
65.00	6,944	4	5,951	5,951		
66.00	10,286	3	8,615	14,566		
67.00	17,678	3	13,982	28,548		

Volume

Invert

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Device	Routing	Invert	Outlet Devices
#1	Discarded	64.00'	2.410 in/hr Exfiltration over Surface area
#2	Primary	65.33'	8.0" Round Culvert
	•		L= 21.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 65.33' / 65.00' S= 0.0157 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Secondary	66.00'	20.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.28 cfs @ 21.68 hrs HW=64.00' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.28 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=64.00' (Free Discharge) 2=Culvert (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=64.00' (Free Discharge) 3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 3: Infiltration Basin-3

Inflow Area = 7.472 ac,		0.15% Impervious, Inflow I	Depth > 0.31" for 2-Year event
Inflow =	1.02 cfs @	12.39 hrs, Volume=	0.194 af
Outflow =	0.62 cfs @	12.66 hrs, Volume=	0.193 af, Atten= 39%, Lag= 16.2 min
Discarded =	0.62 cfs @	12.66 hrs, Volume=	0.193 af
Primary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 62.06' @ 12.66 hrs Surf.Area= 11,093 sf Storage= 651 cf

Plug-Flow detention time= 10.1 min calculated for 0.192 af (99% of inflow) Center-of-Mass det. time= 7.9 min (954.5 - 946.6)

Avail Storage Storage Description

VOIGITIC	IIIVCIL	Avaii.Oto	rage otorage	Description	
#1	62.00'	43,2	11 cf Custom	Stage Data (Pri	smatic) Listed below (Recalc)
Elevation	on Su	urf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
62.0	00	10,997	0	0	
63.0	00	12,633	11,815	11,815	
64.0	00	15,318	13,976	25,791	
65.0	00	19,523	17,421	43,211	
Device	Routing	Invert	Outlet Devices	S	
#1	Discarded	62.00'	2.410 in/hr Ex	filtration over S	Surface area
#2	Primary	63.00'	6.0" Round 0	Culvert	
			L= 23.5' CPF	P, projecting, no	headwall, Ke= 0.900
			Inlet / Outlet In	nvert= 63.00' / 6	2.50' S= 0.0213 '/' Cc= 0.900
			n= 0.013 Cor	rugated PE, smo	ooth interior, Flow Area= 0.20 sf
#3	Secondary	64.00'	20.0' long x 0	0.5' breadth Bro	ad-Crested Rectangular Weir

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Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.62 cfs @ 12.66 hrs HW=62.06' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.62 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=62.00' (Free Discharge) 2=Culvert (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=62.00' (Free Discharge) 3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 4: Infiltration Basin-4

Inflow Area =	14.295 ac,	0.30% Impervious, Inflow	Depth > 0.12"	for 2-Year event
Inflow =	0.26 cfs @	13.76 hrs, Volume=	0.149 af	
Outflow =	0.25 cfs @	13.90 hrs, Volume=	0.147 af, Atte	en= 0%, Lag= 8.5 min
Discarded =	0.25 cfs @	13.90 hrs, Volume=	0.147 af	
Primary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 66.53' @ 13.90 hrs Surf.Area= 5,548 sf Storage= 158 cf

Plug-Flow detention time= 10.3 min calculated for 0.147 af (99% of inflow) Center-of-Mass det. time= 6.9 min (1,029.6 - 1,022.6)

Volume	Invert	Avail.Storage	Storage Description
#1	66.50'	35,427 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	67.00'	80,433 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
		115,859 cf	Total Available Storage

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
66.50	5,477	0	0
67.00	6,706	3,046	3,046
68.00	9,279	7,993	11,038
69.00	11,959	10,619	21,657
70.00	15,580	13,770	35,427
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
67.00	1,364	0	0
68.00	12,524	6,944	6,944
69.00	34,019	23,272	30,216
70.00	66,415	50,217	80,433

Device	Routing	Invert	Outlet Devices
#1	Discarded	66.50'	2.410 in/hr Exfiltration over Surface area
#2	Primary	69.00'	20.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00

Coef. (English) 2.80 2.92 3.08 3.30 3.32

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Discarded OutFlow Max=0.31 cfs @ 13.90 hrs HW=66.53' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.31 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=66.50' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 5: Infiltration Basin-5

Inflow Area =	0.850 ac,	0.00% Impervious, Inflo	w Depth > 0.05"	for 2-Year event
Inflow =	0.01 cfs @	15.22 hrs, Volume=	0.004 af	
Outflow =	0.01 cfs @	15.32 hrs, Volume=	0.004 af, Atte	en= 0%, Lag= 6.1 min
Discarded =	0.01 cfs @	15.32 hrs, Volume=	0.004 af	
Primary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 70.00' @ 15.32 hrs Surf.Area= 2,046 sf Storage= 2 cf

Plug-Flow detention time= 5.9 min calculated for 0.004 af (99% of inflow) Center-of-Mass det. time= 3.6 min (1,097.1 - 1,093.4)

Volume	Invert	Avail.S	torage	Storage	Description	
#1	70.00'	5,	987 cf	Custon	n Stage Data (Pris	matic) Listed below (Recalc)
Elevation (feet)	Surf./	Area sq-ft)		:Store c-feet)	Cum.Store (cubic-feet)	
70.00	2	,045		0	0	
71.00	2	,980		2,513	2,513	
72.00	3	,968		3,474	5,987	

Device	Routing	Invert	Outlet Devices	
#1	Discarded	70.00'	2.410 in/hr Exfiltration over Surface area	
#2	Primary	71.00'	20.0' long x 0.5' breadth Broad-Crested Rectangular Weir	
			Head (feet) 0.20 0.40 0.60 0.80 1.00	
			Coef. (English) 2.80 2.92 3.08 3.30 3.32	

Discarded OutFlow Max=0.11 cfs @ 15.32 hrs HW=70.00' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.11 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=70.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 6: Infiltration Basin-6

Inflow Area =	13.219 ac,	0.48% Impervious, Inflo	w Depth > 0.12" for 2-Year event
Inflow =	0.24 cfs @	12.98 hrs, Volume=	0.138 af
Outflow =	0.24 cfs @	13.85 hrs, Volume=	0.137 af, Atten= 0%, Lag= 52.2 min
Discarded =	0.24 cfs @	13.85 hrs, Volume=	0.137 af
Primary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af

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Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 57.01' @ 13.85 hrs Surf.Area= 13,462 sf Storage= 126 cf

Plug-Flow detention time= 8.9 min calculated for 0.136 af (99% of inflow)

Center-of-Mass det. time= 6.0 min (1,027.2 - 1,021.2)

Volume	Invert	Avail.Storage	Storage Description
#1	57.00'	53,030 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	57.00'	2,077 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
		55,107 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
57.00	13,283	0	0
58.00	16,082	14,683	14,683
59.00	18,971	17,527	32,209
60.00	22,670	20,821	53,030
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
57.00	150	0	0
58.00	398	274	274
59.00	851	625	899
60.00	1.506	1.179	2.077

Device	Routing	Invert	Outlet Devices
#1	Discarded	57.00'	2.410 in/hr Exfiltration over Surface area
#2	Primary	58.33'	6.0" Round Culvert
	_		L= 21.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 58.33' / 58.00' S= 0.0157 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Secondary	59.00'	20.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.75 cfs @ 13.85 hrs HW=57.01' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.75 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=57.00' (Free Discharge) 2=Culvert (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=57.00' (Free Discharge) 3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PDA-1A: PDA-1A	Runoff Area=0.741 ac 0.00% Impervious Runoff Depth>0.83" Flow Length=273' Tc=7.8 min CN=55 Runoff=0.50 cfs 0.051 af
Subcatchment PDA-2A: PDA-2A	Runoff Area=0.292 ac 0.00% Impervious Runoff Depth>0.14" Flow Length=216' Tc=7.5 min CN=39 Runoff=0.01 cfs 0.003 af
Subcatchment PDA-2B: PDA-2B	Runoff Area=3.787 ac 0.00% Impervious Runoff Depth>1.95" Flow Length=575' Tc=36.9 min CN=72 Runoff=4.40 cfs 0.616 af
Subcatchment PDA-3A: PDA-3A	Runoff Area=1.888 ac 0.00% Impervious Runoff Depth>0.01" Flow Length=644' Tc=17.5 min CN=32 Runoff=0.00 cfs 0.001 af
Subcatchment PDA-4A: PDA-4A	Runoff Area=2.202 ac 0.00% Impervious Runoff Depth>0.01" Flow Length=198' Tc=12.7 min CN=32 Runoff=0.00 cfs 0.002 af
Subcatchment PDA-4B: PDA-4B	Runoff Area=14.295 ac 0.30% Impervious Runoff Depth>0.48" Flow Length=540' Tc=17.1 min CN=48 Runoff=3.02 cfs 0.568 af
Subcatchment PDA-5A: PDA-5A	Runoff Area=7.393 ac 0.00% Impervious Runoff Depth>0.03" Flow Length=216' Tc=12.0 min CN=34 Runoff=0.03 cfs 0.020 af
Subcatchment PDA-5B: PDA-5B	Runoff Area=0.850 ac 0.00% Impervious Runoff Depth>0.31" Flow Length=177' Tc=7.4 min CN=44 Runoff=0.09 cfs 0.022 af
Subcatchment PDA-5C: PDA-5C	Runoff Area=13.219 ac 0.48% Impervious Runoff Depth>0.48" Flow Length=677' Tc=14.8 min CN=48 Runoff=2.87 cfs 0.526 af
Subcatchment PDA-6A: PDA-6A	Runoff Area=12.931 ac 0.22% Impervious Runoff Depth>0.20" Flow Length=1,965' Tc=40.5 min CN=41 Runoff=0.40 cfs 0.215 af
Subcatchment PDA-6B: PDA-6B	Runoff Area=7.472 ac 0.15% Impervious Runoff Depth>0.83" Flow Length=818' Tc=12.1 min CN=55 Runoff=4.45 cfs 0.518 af
Subcatchment PDA-6C: PDA-6C	Runoff Area=6.153 ac 0.00% Impervious Runoff Depth>0.14" Flow Length=222' Tc=15.8 min CN=39 Runoff=0.12 cfs 0.073 af
Subcatchment PDA-7A: PDA-7A	Runoff Area=4.103 ac 0.00% Impervious Runoff Depth>0.09" Flow Length=1,265' Tc=25.6 min CN=37 Runoff=0.05 cfs 0.031 af
Subcatchment PDA-7B: PDA-7B	Runoff Area=3.882 ac 0.00% Impervious Runoff Depth>0.27" Flow Length=795' Tc=15.2 min CN=43 Runoff=0.30 cfs 0.088 af
Reach DP-1: DP-1	Inflow=0.50 cfs 0.051 af Outflow=0.50 cfs 0.051 af
Reach DP-2: DP-2	Inflow=4.40 cfs 0.620 af Outflow=4.40 cfs 0.620 af

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	<u> </u>
Reach DP-3: DP-3	Inflow=0.00 cfs 0.001 af Outflow=0.00 cfs 0.001 af
	Outnow-0.00 613 0.001 at
Reach DP-4: DP-4	Inflow=0.00 cfs 0.002 af
	Outflow=0.00 cfs 0.002 af
Reach DP-5: DP-5	Inflow=0.03 cfs 0.020 af
	Outflow=0.03 cfs 0.020 af
Reach DP-6: DP-6	Inflow=4.64 cfs 0.776 af
1100011 21 21 2	Outflow=4.64 cfs 0.776 af
Pond 1: Infiltration Basin 1 Peak Elev=70.0	12! Storago=07 of Inflow=0.20 of 0.000 of
	02' Storage=97 cf Inflow=0.30 cfs 0.088 af 00 cfs 0.000 af Outflow=0.27 cfs 0.088 af
Pond 2: Infiltration Basin 2 Peak Elev=64.18 Discarded=0.30 cfs 0.213 af Primary=0.00 cfs 0.000 af Secondary=0.	3' Storage=923 cf Inflow=0.40 cfs 0.215 af
Discarded-0.30 cis 0.213 ai Friinary-0.00 cis 0.000 ai Secondary-0.	00 cis 0.000 ai Outilow-0.30 cis 0.213 ai
	Storage=7,217 cf Inflow=4.45 cfs 0.518 af
Discarded=0.67 cfs 0.516 af Primary=0.00 cfs 0.000 af Secondary=0.	00 cts 0.000 at Outflow=0.67 cts 0.516 at
Pond 4: Infiltration Basin-4 Peak Elev=67.36'	Storage=6,845 cf Inflow=3.02 cfs 0.568 af
Discarded=0.73 cfs 0.532 af Primary=0.	00 cfs 0.000 af Outflow=0.73 cfs 0.532 af
Pond 5: Infiltration Basin-5 Peak Elev=70.0	01' Storage=30 cf Inflow=0.09 cfs 0.022 af
	00 cfs 0.000 af Outflow=0.09 cfs 0.022 af
Pond 6: Infiltration Basin-6 Peak Elev=57.32'	Storage=4,520 cf Inflow=2.87 cfs 0.526 af
Discarded=0.80 cfs 0.523 af Primary=0.00 cfs 0.000 af Secondary=0.	•

Total Runoff Area = 79.208 ac Runoff Volume = 2.734 af Average Runoff Depth = 0.41" 99.81% Pervious = 79.061 ac 0.19% Impervious = 0.147 ac

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Summary for Subcatchment PDA-1A: PDA-1A

Runoff = 0.50 cfs @ 12.15 hrs, Volume= 0.051 af, Depth> 0.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Area	(ac) C	N Desc	cription			
0.001 30 Woods, Good, HSG A							
	0.725 55 Woods, Good, HSG B						
	0.012 48 Brush, Good, HSG B						
0.003 61 >75% Grass cover, Good, HSG B							
	0.	741 5	55 Weig	ghted Aver	age		_
	0.	741	100.	00% Pervi	ous Area		
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		_
	4.1	50	0.0400	0.20		Sheet Flow,	
						Grass: Short n= 0.150 P2= 3.40"	
	1.6	88	0.0340	0.92		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
	1.2	73	0.0410	1.01		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
	0.9	62	0.0480	1.10		Shallow Concentrated Flow,	
_						Woodland Kv= 5.0 fps	_
	7.8	273	Total				

Summary for Subcatchment PDA-2A: PDA-2A

Runoff = 0.01 cfs @ 13.78 hrs, Volume= 0.003 af, Depth> 0.14"

	Area	(ac)	CN	Desc	cription		
	0.051 30 Brush, Good, HSG A						
	0.	016	73	Brus	h, Good, F	ISG D	
	0.	224	39	>75%	% Grass co	over, Good	, HSG A
_	0.	001	80	>75%	% Grass co	over, Good	, HSG D
	0.	292	39	Weig	ghted Aver	age	
	0.	292		100.	00% Pervi	ous Area	
	Tc (min)	Lengtl (feet		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	5.5	50	0 0	.0200	0.15		Sheet Flow,
	2.0	160	6 0	.0390	1.38		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	7.5	210	3 T	otal	•	•	

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Summary for Subcatchment PDA-2B: PDA-2B

Runoff = 4.40 cfs @ 12.53 hrs, Volume= 0.616 af, Depth> 1.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Area	(ac) C	N Desc	cription		
_				ds, Good,		
	3.	396 7	77 Woo	ods, Good,	HSG D	
	3.	787 7	72 Wei	ghted Aver	age	
	3.	787	100.	00% Pervi	ous Area	
	_					
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.8	50	0.0100	0.05		Sheet Flow, Tc-1
						Woods: Light underbrush n= 0.400 P2= 3.40"
	3.7	111	0.0100	0.50		Shallow Concentrated Flow, Tc-2
						Woodland Kv= 5.0 fps
	2.1	107	0.0300	0.87		Shallow Concentrated Flow, Tc-3
						Woodland Kv= 5.0 fps
	0.4	25	0.0400	1.00		Shallow Concentrated Flow, Tc-4
						Woodland Kv= 5.0 fps
	14.9	282	0.0040	0.32		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	36.9	575	Total			

Summary for Subcatchment PDA-3A: PDA-3A

Runoff = 0.00 cfs @ 23.09 hrs, Volume= 0.001 af, Depth> 0.01"

Area (ac)	CN	Description
1.372	30	Woods, Good, HSG A
0.143	30	Brush, Good, HSG A
 0.373	39	>75% Grass cover, Good, HSG A
1.888	32	Weighted Average
1.888		100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.5	, ,	0.0400	0.18	(010)	Sheet Flow,
2.0	21	0.0400	0.10		Grass: Short n= 0.150 P2= 3.40"
2.9	23	0.0200	0.13		Sheet Flow,
2.0	20	0.0200	0.10		Grass: Short n= 0.150 P2= 3.40"
0.4	23	0.0200	0.99		Shallow Concentrated Flow,
0.1	20	0.0200	0.00		Short Grass Pasture Kv= 7.0 fps
0.9	64	0.0300	1.21		Shallow Concentrated Flow,
0.0	01	0.0000	1.21		Short Grass Pasture Kv= 7.0 fps
0.3	37	0.0800	1.98		Shallow Concentrated Flow,
0.0	0.	0.0000			Short Grass Pasture Kv= 7.0 fps
0.3	30	0.0700	1.85		Shallow Concentrated Flow,
0.0		0.0.00			Short Grass Pasture Kv= 7.0 fps
0.3	32	0.0600	1.71		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.3	28	0.0400	1.40		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
2.5	129	0.0300	0.87		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.5	28	0.0400	1.00		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
1.1	48	0.0200	0.71		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.3	20	0.0500	1.12		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
5.2	155	0.0100	0.50		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
17.5	644	Total			

Summary for Subcatchment PDA-4A: PDA-4A

Runoff = 0.00 cfs @ 22.98 hrs, Volume= 0.002 af, Depth> 0.01"

	Area (ac)	CN	Description
	0.576	30	Woods, Good, HSG A
	1.376	30	Brush, Good, HSG A
	0.115	48	Brush, Good, HSG B
	0.122	39	>75% Grass cover, Good, HSG A
	0.013	61	>75% Grass cover, Good, HSG B
_	2.202	32	Weighted Average
	2.202		100.00% Pervious Area

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_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	7.1	16	0.0300	0.04		Sheet Flow,
						Woods: Dense underbrush n= 0.800 P2= 3.40"
	3.4	34	0.0300	0.17		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.40"
	1.6	90	0.0170	0.91		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.6	58	0.0600	1.71		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	12.7	198	Total			

Summary for Subcatchment PDA-4B: PDA-4B

Runoff = 3.02 cfs @ 12.44 hrs, Volume= 0.568 af, Depth> 0.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Area	(ac)	CN	Desc	cription				
	0.	540	30						
	9.	812	39	>75%	% Grass co	over, Good	, HSG A		
	2.690 61 >75% Grass cover, Good, HSG B								
*	* 1.210 96 Gravel surface								
*	0.	043	98	Equi	pment Pac	d Area			
	14.	295	48	Weig	ghted Aver	age			
	14.252 99.70% Pervious Area								
	0.043 0.30% Impervious Area								
	Tc	Lengt	h	Slope	Velocity	Capacity	Description		
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)			
	7.2	5	0 0	.0100	0.12		Sheet Flow,		
							Grass: Short n= 0.150 P2= 3.40"		
	3.7	25	8 0	.0270	1.15		Shallow Concentrated Flow,		
							Short Grass Pasture Kv= 7.0 fps		
	6.2	23	2 0	.0080	0.63		Shallow Concentrated Flow,		
							Short Grass Pasture Kv= 7.0 fps		
	17.1	54	0 T	otal					

Summary for Subcatchment PDA-5A: PDA-5A

Runoff = 0.03 cfs @ 17.22 hrs, Volume= 0.020 af, Depth> 0.03"

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	Area	(ac)	CN	Desc	cription			
	6.	130	30	Woo	ds, Good,	HSG A		
	0.	164	55	Woo	ds, Good,	HSG B		
0.104 77 Woods, Good, HSG D								
0.561 30 Brush, Good, HSG A								
	0.	019	48	Brus	h, Good, H	HSG B		
	0.	006	73	Brus	h, Good, F	HSG D		
		134	39			over, Good		
		009	61			over, Good	, HSG B	
*	0.	266	96	Grav	<u>el surface</u>	:		
	7.393 34 Weighted Average							
	7.	393		100.	00% Pervi	ous Area		
	Tc	Lengt		Slope	Velocity	Capacity	Description	
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)		
	2.9	1	6 (0.0100	0.09		Sheet Flow,	
							Grass: Short n= 0.150 P2= 3.40"	
	6.4	3	4 (0.0440	0.09		Sheet Flow,	
							Woods: Light underbrush n= 0.400 P2= 3.40"	
	2.7	16	6 (0.0420	1.02		Shallow Concentrated Flow,	
							Woodland Kv= 5.0 fps	
	12.0	21	6	Γotal				

Summary for Subcatchment PDA-5B: PDA-5B

Runoff = 0.09 cfs @ 12.39 hrs, Volume= 0.022 af, Depth> 0.31"

	Area	(ac) C	N Des	cription				
0.067 30 Brush, Good, HSG A								
0.700 39 >75% Grass cover, Good, HSG A								
* 0.083 96 Gravel surface								
	0.850 44 Weighted Average							
	0.850 100.00% Pervious Area							
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	5.5	50	0.0200	0.15		Sheet Flow,		
						Grass: Short n= 0.150 P2= 3.40"		
	1.9	127	0.0260	1.13		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	7.4	177	Total			·		

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Summary for Subcatchment PDA-5C: PDA-5C

Runoff = 2.87 cfs @ 12.41 hrs, Volume= 0.526 af, Depth> 0.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Area	(ac)	CN	Desc	cription			
	0.	171	30	Brus	h, Good, F	HSG A		
	8.	834	39	>75%	% Grass co	over, Good	, HSG A	
	3.	489	61			over, Good	, HSG B	
*	0.	661	96	_	el surface			
*	0.	064	98	Equi	pment Pac	d Area		
	13.219 48 Weighted Average							
	13.155 99.52% Pervious Area							
	0.064 0.48% Impervious Area							
	_		_					
	Tc	Lengt		Slope	Velocity	Capacity	Description	
	(min)	(fee		(ft/ft)	(ft/sec)	(cfs)		
	5.5	5	0 0	0.0200	0.15		Sheet Flow,	
							Grass: Short n= 0.150 P2= 3.40"	
	5.2	30	6 (0.0200	0.99		Shallow Concentrated Flow,	
							Short Grass Pasture Kv= 7.0 fps	
	4.1	32	1 (0.0350	1.31		Shallow Concentrated Flow,	
							Short Grass Pasture Kv= 7.0 fps	
	14.8	67	7 1	Γotal				

Summary for Subcatchment PDA-6A: PDA-6A

Runoff = 0.40 cfs @ 13.31 hrs, Volume= 0.215 af, Depth> 0.20"

	Area (ac)	CN	Description
	2.760	30	Woods, Good, HSG A
	0.849	30	Brush, Good, HSG A
	0.002	73	Brush, Good, HSG D
	8.077	39	>75% Grass cover, Good, HSG A
	0.650	80	>75% Grass cover, Good, HSG D
*	0.564	96	Gravel surface
*	0.029	98	Equipment Pad Area
	12.931	41	Weighted Average
	12.902		99.78% Pervious Area
	0.029		0.22% Impervious Area

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Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
10.2	50	0.0300	0.08		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.40"
0.8	38	0.0260	0.81		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
3.5	257	0.0310	1.23		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
8.6	484	0.0350	0.94		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
6.1	202	0.0120	0.55		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.9	52	0.0190	0.96		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
1.1	58	0.0170	0.91		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
1.2	140	0.0140	1.90		Shallow Concentrated Flow,
- 4	0.45	0.0000	4.04		Unpaved Kv= 16.1 fps
5.1	315	0.0220	1.04		Shallow Concentrated Flow,
0.0	000	0.0400	0.07		Short Grass Pasture Kv= 7.0 fps
3.0	369	0.0190	2.07		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
40.5	1,965	Total			

Summary for Subcatchment PDA-6B: PDA-6B

Runoff = 4.45 cfs @ 12.21 hrs, Volume= 0.518 af, Depth> 0.83"

	Area	(ac)	CN	Desc	ription			
	0.	045	73	Brus	h, Good, F	HSG D		
	3.	466	39			over, Good	, HSG A	
	2.	386	61	>75%	√ Grass co	over, Good	, HSG B	
	1.471 80 >75% Grass cover, Good, HSG D							
*	0.	093	96	Grav	el surface	,		
*	0.	011	98	Equi	pment Pac	d Area		
	7.472 55 Weighted Average							
	7.461 99.85% Pervious Area							
	0.011 0.15% Impervious Area					ous Area		
	Тс	Lengt	:h	Slope	Velocity	Capacity	Description	
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)		
	4.6	5	0 (0.0300	0.18		Sheet Flow,	
							Grass: Short n= 0.150 P2= 3.40"	
	2.2	17	9 (0.0360	1.33		Shallow Concentrated Flow,	
							Short Grass Pasture Kv= 7.0 fps	
	5.3	58	9 (0.0150	1.84		Shallow Concentrated Flow,	
							Grassed Waterway Kv= 15.0 fps	
	12.1	81	8 -	Total				

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Summary for Subcatchment PDA-6C: PDA-6C

Runoff = 0.12 cfs @ 13.91 hrs, Volume= 0.073 af, Depth> 0.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Area	(ac)	CN	Desc	cription			
4.594 30 Woods, Good, HSG A								
0.214 55 Woods, Good, HSG B								
0.758 77 Woods, Good, HSG D								
	0.	258	30	Brus	h, Good, F	HSG A		
	0.	002	48	Brus	h, Good, F	HSG B		
	0.	211	73	Brus	h, Good, F	HSG D		
	0.	057	39	>759	% Grass co	over, Good,	, HSG A	
	0.059 80 >75% Grass cover, Good, HSG D							
	6.153 39 Weighted Average							
	6.	153		100.	00% Pervi	ous Area		
	Tc	Length	າ S	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	12.0	50	0.	0200	0.07		Sheet Flow,	
							Woods: Light underbrush n= 0.400 P2= 3.40"	
	3.8	172	2 0.	0230	0.76		Shallow Concentrated Flow,	
							Woodland Kv= 5.0 fps	
	15.8	222	2 To	otal				

Summary for Subcatchment PDA-7A: PDA-7A

Runoff = 0.05 cfs @ 15.28 hrs, Volume= 0.031 af, Depth> 0.09"

_	Area (ac)	CN	Description				
	1.445	30	Woods, Good, HSG A				
	0.240	30	rush, Good, HSG A				
	2.280	39	>75% Grass cover, Good, HSG A				
*	0.138	96	Gravel surface				
	4.103	37	Weighted Average				
	4.103		100.00% Pervious Area				

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	10.2	50	0.0300	0.08	, ,	Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.40"
	1.5	59	0.0170	0.65		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	13.9	1,156	0.0390	1.38		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	25.6	1,265	Total			

Summary for Subcatchment PDA-7B: PDA-7B

Runoff = 0.30 cfs @ 12.53 hrs, Volume= 0.088 af, Depth> 0.27"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Area	(ac) C	N Des	cription		
	0.	073	30 Brus	sh, Good, F	HSG A	
	3.	526	39 >75°	% Grass c	over, Good	, HSG A
*	0.	283	96 Grav	vel surface	•	,
_	3.	882	43 Wei	ghted Aver	age	
	_	882	,	00% Pervi	•	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
	5.5	50	0.0200	0.15		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.40"
	1.1	93	0.0430	1.45		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.2	33	0.0200	2.28		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	8.4	619	0.0310	1.23		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	15.2	795	Total			

Summary for Reach DP-1: DP-1

Inflow Area = 0.741 ac, 0.00% Impervious, Inflow Depth > 0.83" for 10-Year event

Inflow = 0.50 cfs @ 12.15 hrs, Volume= 0.051 af

Outflow = 0.50 cfs @ 12.15 hrs, Volume= 0.051 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Summary for Reach DP-2: DP-2

Inflow Area = 4.079 ac, 0.00% Impervious, Inflow Depth > 1.82" for 10-Year event

Inflow = 4.40 cfs @ 12.53 hrs, Volume= 0.620 af

Outflow = 4.40 cfs @ 12.53 hrs, Volume= 0.620 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-3: DP-3

Inflow Area = 1.888 ac, 0.00% Impervious, Inflow Depth > 0.01" for 10-Year event

Inflow = 0.00 cfs @ 23.09 hrs, Volume= 0.001 af

Outflow = 0.00 cfs @ 23.09 hrs, Volume= 0.001 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-4: DP-4

Inflow Area = 16.497 ac, 0.26% Impervious, Inflow Depth > 0.00" for 10-Year event

Inflow = 0.00 cfs @ 22.98 hrs, Volume= 0.002 af

Outflow = 0.00 cfs @ 22.98 hrs, Volume= 0.002 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-5: DP-5

Inflow Area = 21.462 ac, 0.30% Impervious, Inflow Depth > 0.01" for 10-Year event

Inflow = 0.03 cfs @ 17.22 hrs, Volume= 0.020 af

Outflow = 0.03 cfs @ 17.22 hrs, Volume= 0.020 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-6: DP-6

Inflow Area = 41.249 ac, 0.10% Impervious, Inflow Depth > 0.23" for 10-Year event

Inflow = 4.64 cfs @ 12.52 hrs, Volume= 0.776 af

Outflow = 4.64 cfs @ 12.52 hrs, Volume= 0.776 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Pond 1: Infiltration Basin 1

Inflow Area = 3.882 ac, 0.00% Impervious, Inflow Depth > 0.27" for 10-Year event

Inflow = 0.30 cfs @ 12.53 hrs, Volume= 0.088 af

Outflow = 0.27 cfs @ 12.64 hrs, Volume= 0.088 af, Atten= 9%, Lag= 6.2 min

Discarded = 0.27 cfs @ 12.64 hrs, Volume= 0.088 af Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Peak Elev= 70.02' @ 12.64 hrs Surf.Area= 5,647 sf Storage= 97 cf

Plug-Flow detention time= 6.0 min calculated for 0.088 af (100% of inflow)

Center-of-Mass det. time= 4.2 min (987.8 - 983.5)

Volume	Invert	Avail.Sto	rage Storage [Description	
#1	70.00'	14,70	03 cf Custom S	Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio (fee 70.0 71.0 72.0)0 00 00	urf.Area (sq-ft) 5,630 6,602 10,572	Inc.Store (cubic-feet) 0 6,116 8,587	Cum.Store (cubic-feet) 0 6,116 14,703	
Device #1	Routing Discarded	Invert 70.00'	Outlet Devices 2.410 in/hr Ext		Surface area
#2	Primary	71.00'	-	.5' breadth Bro 20 0.40 0.60	pad-Crested Rectangular Weir 0.80 1.00

Discarded OutFlow Max=0.32 cfs @ 12.64 hrs HW=70.02' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.32 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=70.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2: Infiltration Basin 2

Inflow Area =	12.931 ac, 0.22% Impervious, Inflov	v Depth > 0.20" for 10-Year event
Inflow =	0.40 cfs @ 13.31 hrs, Volume=	0.215 af
Outflow =	0.30 cfs @ 16.04 hrs, Volume=	0.213 af, Atten= 25%, Lag= 163.6 min
Discarded =	0.30 cfs @ 16.04 hrs, Volume=	0.213 af
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 64.18' @ 16.04 hrs Surf.Area= 5,315 sf Storage= 923 cf

Plug-Flow detention time= 28.1 min calculated for 0.213 af (99% of inflow) Center-of-Mass det. time= 25.2 min (1,048.8 - 1,023.6)

Volume	Invert /	Avail.Storage	Storage	e Description
#1	64.00'	28,548 cf	Custom	n Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Ar (sq		:Store c-feet)	Cum.Store (cubic-feet)
64.00	4.9	958	0	0

(reet)	(sq-π)	(cubic-reet)	(cubic-reet)
64.00	4,958	0	0
65.00	6,944	5,951	5,951
66.00	10,286	8,615	14,566
67.00	17,678	13,982	28,548

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Device	Routing	Invert	Outlet Devices
#1	Discarded	64.00'	2.410 in/hr Exfiltration over Surface area
#2	Primary	65.33'	8.0" Round Culvert
	-		L= 21.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 65.33' / 65.00' S= 0.0157 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Secondary	66.00'	20.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.30 cfs @ 16.04 hrs HW=64.18' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.30 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=64.00' (Free Discharge) 2=Culvert (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=64.00' (Free Discharge) -3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 3: Infiltration Basin-3

Inflow Area =	7.472 ac, 0.15% Impervious, Inflo	w Depth > 0.83" for 10-Year event
Inflow =	4.45 cfs @ 12.21 hrs, Volume=	0.518 af
Outflow =	0.67 cfs @ 14.12 hrs, Volume=	0.516 af, Atten= 85%, Lag= 114.6 min
Discarded =	0.67 cfs @ 14.12 hrs, Volume=	0.516 af
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 62.63' @ 14.12 hrs Surf.Area= 12,023 sf Storage= 7,217 cf

Plug-Flow detention time= 111.4 min calculated for 0.515 af (99% of inflow) Center-of-Mass det. time= 109.3 min (1,011.7 - 902.5)

Volume	Inver	t Avail.Sto	rage Storage	Description	
#1	62.00	43,2	11 cf Custon	n Stage Data (Pri	smatic) Listed below (Recalc)
Elevatio	on S	urf.Area	Inc.Store	Cum.Store	
(fee	-	(sq-ft)	(cubic-feet)	(cubic-feet)	
62.0	00	10,997	0	0	
63.0	00	12,633	11,815	11,815	
64.0	00	15,318	13,976	25,791	
65.0	00	19,523	17,421	43,211	
Device	Routing	Invert	Outlet Device	es	
#1	Discarded	62.00'	2.410 in/hr E	xfiltration over S	Gurface area
#2	Primary	63.00'	6.0" Round	Culvert	
			L= 23.5' CF	PP, projecting, no	headwall, Ke= 0.900
					2.50' S= 0.0213 '/' Cc= 0.900
				•	ooth interior, Flow Area= 0.20 sf
#3	Secondary	<i>'</i> 64.00'	20.0' long x	0.5' breadth Bro	ad-Crested Rectangular Weir

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Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.67 cfs @ 14.12 hrs HW=62.63' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.67 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=62.00' (Free Discharge) 2=Culvert (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=62.00' (Free Discharge) 3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 4: Infiltration Basin-4

Inflow Area =	14.295 ac, 0.30% Impervio	ous, Inflow Depth > 0.48"	for 10-Year event
Inflow =	3.02 cfs @ 12.44 hrs, Vol	ume= 0.568 af	
Outflow =	0.73 cfs @ 15.00 hrs, Vol	ume= 0.532 af, Atte	en= 76%, Lag= 153.4 min
Discarded =	0.73 cfs @ 15.00 hrs, Vol	ume= 0.532 af	
Primary =	0.00 cfs @ 0.00 hrs, Vol	ume= 0.000 af	

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 67.36' @ 15.00 hrs Surf.Area= 13,018 sf Storage= 6,845 cf

Plug-Flow detention time= 130.7 min calculated for 0.531 af (94% of inflow) Center-of-Mass det. time= 101.2 min (1,043.8 - 942.6)

Volume	Invert	Avail.Storage	Storage Description
#1	66.50'	35,427 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	67.00'	80,433 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

	115,859 cf	Total Available Storage

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
66.50	5,477	0	0
67.00	6,706	3,046	3,046
68.00	9,279	7,993	11,038
69.00	11,959	10,619	21,657
70.00	15,580	13,770	35,427
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
67.00	1,364	0	0
68.00	12,524	6,944	6,944
69.00	34,019	23,272	30,216
70.00	66,415	50,217	80,433
Device Routing	ı Invei	t Outlet Devices	

DEVICE	Routing	IIIVEIL	Outlet Devices
#1	Discarded	66.50'	2.410 in/hr Exfiltration over Surface area
#2	Primary	69.00'	20.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00

Coef. (English) 2.80 2.92 3.08 3.30 3.32

Volume

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Discarded OutFlow Max=0.73 cfs @ 15.00 hrs HW=67.36' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.73 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=66.50' (Free Discharge)

2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 5: Infiltration Basin-5

Inflow Area =	0.850 ac,	0.00% Impervious, Inflow De	epth > 0.31" for 10-Year event
Inflow =	0.09 cfs @	12.39 hrs, Volume=	0.022 af
Outflow =	0.09 cfs @	12.49 hrs, Volume=	0.022 af, Atten= 10%, Lag= 5.8 min
Discarded =	0.09 cfs @	12.49 hrs, Volume=	0.022 af
Primary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 70.01' @ 12.49 hrs Surf.Area= 2,059 sf Storage= 30 cf

Plug-Flow detention time= 5.9 min calculated for 0.022 af (99% of inflow) Center-of-Mass det. time= 4.3 min (972.4 - 968.1)

Invert Avail.Storage Storage Description

#1	70.00'	5,9	87 cf Custom	Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
70.0	00	2,045	0	0	
71.0	00	2,980	2,513	2,513	
72.0	00	3,968	3,474	5,987	
Device	Routing	Invert	Outlet Devices	3	
#1	Discarded	70.00'	2.410 in/hr Ex	filtration over	Surface area
#2	Primary	71.00'		0.5' breadth Bro 0.20 0.40 0.60	oad-Crested Rectangular Weir 0.80 1.00

Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.11 cfs @ 12.49 hrs HW=70.01' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.11 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=70.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 6: Infiltration Basin-6

Inflow Area =	13.219 ac,	0.48% Impervious, Inflow	Depth > 0.48" for 10-Year event
Inflow =	2.87 cfs @	12.41 hrs, Volume=	0.526 af
Outflow =	0.80 cfs @	14.05 hrs, Volume=	0.523 af, Atten= 72%, Lag= 98.4 min
Discarded =	0.80 cfs @	14.05 hrs, Volume=	0.523 af
Primary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af

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Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 57.32' @ 14.05 hrs Surf.Area= 14,422 sf Storage= 4,520 cf

Plug-Flow detention time= 53.6 min calculated for 0.523 af (100% of inflow)

Center-of-Mass det. time= 51.4 min (992.4 - 941.0)

Volume	Invert	Avail.Storage	Storage Description
#1	57.00'	53,030 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	57.00'	2,077 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

55,107 cf Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
57.00	13,283	0	0
58.00	16,082	14,683	14,683
59.00	18,971	17,527	32,209
60.00	22,670	20,821	53,030
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
57.00	150	0	0
58.00	398	274	274
59.00	851	625	899
60.00	1,506	1,179	2,077

Device	Routing	Invert	Outlet Devices
#1	Discarded	57.00'	2.410 in/hr Exfiltration over Surface area
#2	Primary	58.33'	6.0" Round Culvert
	-		L= 21.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 58.33' / 58.00' S= 0.0157 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Secondary	59.00'	20.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.80 cfs @ 14.05 hrs HW=57.32' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.80 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=57.00' (Free Discharge) 2=Culvert (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=57.00' (Free Discharge) 3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PDA-1A: PDA-1A	Runoff Area=0.741 ac 0.00% Impervious Runoff Depth>2.12" Flow Length=273' Tc=7.8 min CN=55 Runoff=1.59 cfs 0.131 af
Subcatchment PDA-2A: PDA-2A	Runoff Area=0.292 ac 0.00% Impervious Runoff Depth>0.77" Flow Length=216' Tc=7.5 min CN=39 Runoff=0.12 cfs 0.019 af
Subcatchment PDA-2B: PDA-2B	Runoff Area=3.787 ac 0.00% Impervious Runoff Depth>3.80" Flow Length=575' Tc=36.9 min CN=72 Runoff=8.71 cfs 1.199 af
Subcatchment PDA-3A: PDA-3A	Runoff Area=1.888 ac 0.00% Impervious Runoff Depth>0.31" Flow Length=644' Tc=17.5 min CN=32 Runoff=0.11 cfs 0.049 af
Subcatchment PDA-4A: PDA-4A	Runoff Area=2.202 ac 0.00% Impervious Runoff Depth>0.31" Flow Length=198' Tc=12.7 min CN=32 Runoff=0.14 cfs 0.057 af
Subcatchment PDA-4B: PDA-4B	Runoff Area=14.295 ac 0.30% Impervious Runoff Depth>1.48" Flow Length=540' Tc=17.1 min CN=48 Runoff=14.69 cfs 1.767 af
Subcatchment PDA-5A: PDA-5A	Runoff Area=7.393 ac 0.00% Impervious Runoff Depth>0.43" Flow Length=216' Tc=12.0 min CN=34 Runoff=1.00 cfs 0.264 af
Subcatchment PDA-5B: PDA-5B	Runoff Area=0.850 ac 0.00% Impervious Runoff Depth>1.15" Flow Length=177' Tc=7.4 min CN=44 Runoff=0.77 cfs 0.082 af
Subcatchment PDA-5C: PDA-5C	Runoff Area=13.219 ac 0.48% Impervious Runoff Depth>1.48" Flow Length=677' Tc=14.8 min CN=48 Runoff=14.33 cfs 1.636 af
Subcatchment PDA-6A: PDA-6A	Runoff Area=12.931 ac 0.22% Impervious Runoff Depth>0.90" Flow Length=1,965' Tc=40.5 min CN=41 Runoff=4.51 cfs 0.973 af
Subcatchment PDA-6B: PDA-6B	Runoff Area=7.472 ac 0.15% Impervious Runoff Depth>2.12" Flow Length=818' Tc=12.1 min CN=55 Runoff=13.95 cfs 1.319 af
Subcatchment PDA-6C: PDA-6C	Runoff Area=6.153 ac 0.00% Impervious Runoff Depth>0.76" Flow Length=222' Tc=15.8 min CN=39 Runoff=2.22 cfs 0.391 af
Subcatchment PDA-7A: PDA-7A	Runoff Area=4.103 ac 0.00% Impervious Runoff Depth>0.62" Flow Length=1,265' Tc=25.6 min CN=37 Runoff=0.92 cfs 0.212 af
Subcatchment PDA-7B: PDA-7B	Runoff Area=3.882 ac 0.00% Impervious Runoff Depth>1.07" Flow Length=795' Tc=15.2 min CN=43 Runoff=2.49 cfs 0.346 af
Reach DP-1: DP-1	Inflow=1.59 cfs 0.131 af Outflow=1.59 cfs 0.131 af
Reach DP-2: DP-2	Inflow=8.79 cfs 1.218 af Outflow=8.79 cfs 1.218 af

1833112HC004B Prepared by Beals and Thomas, Inc. HydroCAD® 10.10-4a s/n 04493 © 2020 HydroCAD Software Solu	Type III 24-hr 100-Year Rainfall=7.00" Printed 11/2/2020 tions LLC Page 38
Reach DP-3: DP-3	Inflow=0.11 cfs 0.049 af Outflow=0.11 cfs 0.049 af
Reach DP-4: DP-4	Inflow=0.14 cfs 0.057 af Outflow=0.14 cfs 0.057 af
Reach DP-5: DP-5	Inflow=1.00 cfs 0.470 af Outflow=1.00 cfs 0.470 af
Reach DP-6: DP-6	Inflow=12.79 cfs 2.596 af Outflow=12.79 cfs 2.596 af
	9' Storage=5,366 cf Inflow=2.49 cfs 0.346 af =0.00 cfs 0.000 af Outflow=0.36 cfs 0.342 af
Pond 2: Infiltration Basin 2 Peak Elev=65.97 Discarded=0.57 cfs 0.489 af Primary=0.74 cfs 0.287 af Secondary	' Storage=14,234 cf Inflow=4.51 cfs 0.973 af =0.00 cfs 0.000 af Outflow=1.30 cfs 0.776 af
Pond 3: Infiltration Basin-3 Discarded=0.85 cfs 0.793 af Primary=0.62 cfs 0.307 af Secondary	Storage=25,046 cf Inflow=13.95 cfs 1.319 af =0.00 cfs 0.000 af Outflow=1.47 cfs 1.100 af
	Storage=32,120 cf Inflow=14.69 cfs 1.767 af =0.00 cfs 0.000 af Outflow=1.90 cfs 1.489 af
	.42' Storage=932 cf Inflow=0.77 cfs 0.082 af =0.00 cfs 0.000 af Outflow=0.14 cfs 0.081 af
Pond 6: Infiltration Basin-6 Peak Elev=58.95' Discarded=1.10 cfs 1.030 af Primary=0.45 cfs 0.206 af Secondary	Storage=32,035 cf Inflow=14.33 cfs 1.636 af =0.00 cfs 0.000 af Outflow=1.55 cfs 1.236 af

Total Runoff Area = 79.208 ac Runoff Volume = 8.445 af Average Runoff Depth = 1.28" 99.81% Pervious = 79.061 ac 0.19% Impervious = 0.147 ac

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Summary for Subcatchment PDA-1A: PDA-1A

Runoff = 1.59 cfs @ 12.12 hrs, Volume= 0.131 af, Depth> 2.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=7.00"

	Area	(ac) C	N Desc	cription			
0.001 30 Woods, Good, HSG A					HSG A		
	0.	725 5	55 Woo	ds, Good,	HSG B		
	0.	012 4	l8 Brus	h, Good, F	HSG B		
	0.	003 6	31 >75°	% Grass co	over, Good	, HSG B	
	0.	741 5	55 Weig	ghted Aver	age		_
	0.	741	100.	00% Pervi	ous Area		
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		_
	4.1	50	0.0400	0.20		Sheet Flow,	
						Grass: Short n= 0.150 P2= 3.40"	
	1.6	88	0.0340	0.92		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
	1.2	73	0.0410	1.01		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
	0.9	62	0.0480	1.10		Shallow Concentrated Flow,	
_						Woodland Kv= 5.0 fps	_
	7.8	273	Total				

Summary for Subcatchment PDA-2A: PDA-2A

Runoff = 0.12 cfs @ 12.20 hrs, Volume= 0.019 af, Depth> 0.77"

_	Area	(ac)	CN	Desc	ription		
0.051 30 Brush, Good, HSG A						ISG A	
0.016 73 Brush, Good, HSG D						ISG D	
	0.	224	39	>75%	√ Grass co	over, Good	, HSG A
_	0.	.001	80	>75%	√ Grass co	over, Good	, HSG D
	0.	292	39	Weig	hted Aver	age	
	0.	292		100.0	00% Pervi	ous Area	
_	Tc (min)	Length (feet)		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	5.5	50	0.0	0200	0.15		Sheet Flow,
	2.0	166	0.0	0390	1.38		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	7.5	216	To	otal			

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Summary for Subcatchment PDA-2B: PDA-2B

Runoff = 8.71 cfs @ 12.51 hrs, Volume= 1.199 af, Depth> 3.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=7.00"

_	Area	(ac) C	N Desc	cription		
				ds, Good,		
_	3.	396 7	77 Woo	ds, Good,	HSG D	
	3.	787 7	'2 Wei	ghted Aver	age	
	3.	787	100.	00% Pervi	ous Area	
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.8	50	0.0100	0.05		Sheet Flow, Tc-1
						Woods: Light underbrush n= 0.400 P2= 3.40"
	3.7	111	0.0100	0.50		Shallow Concentrated Flow, Tc-2
						Woodland Kv= 5.0 fps
	2.1	107	0.0300	0.87		Shallow Concentrated Flow, Tc-3
						Woodland Kv= 5.0 fps
	0.4	25	0.0400	1.00		Shallow Concentrated Flow, Tc-4
						Woodland Kv= 5.0 fps
	14.9	282	0.0040	0.32		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	36.9	575	Total			

Summary for Subcatchment PDA-3A: PDA-3A

Runoff = 0.11 cfs @ 12.64 hrs, Volume= 0.049 af, Depth> 0.31"

Area (ac)	CN	Description						
1.372	30	Woods, Good, HSG A						
0.143	30	Brush, Good, HSG A						
 0.373	39	>75% Grass cover, Good, HSG A						
1.888	32	Weighted Average						
1.888		100.00% Pervious Area						

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	2.5	27	0.0400	0.18	(615)	Shoot Flow
	2.5	21	0.0400	0.16		Sheet Flow, Grass: Short n= 0.150 P2= 3.40"
	2.9	23	0.0200	0.13		Sheet Flow,
	2.9	23	0.0200	0.13		Grass: Short n= 0.150 P2= 3.40"
	0.4	23	0.0200	0.99		Shallow Concentrated Flow,
	0.4	23	0.0200	0.99		Short Grass Pasture Kv= 7.0 fps
	0.9	64	0.0300	1.21		Shallow Concentrated Flow,
	0.9	04	0.0300	1.21		Short Grass Pasture Kv= 7.0 fps
	0.3	37	0.0800	1.98		Shallow Concentrated Flow,
	0.3	31	0.0000	1.90		Short Grass Pasture Kv= 7.0 fps
	0.3	30	0.0700	1.85		Shallow Concentrated Flow,
	0.5	30	0.0700	1.05		Short Grass Pasture Kv= 7.0 fps
	0.3	32	0.0600	1.71		Shallow Concentrated Flow,
	0.5	32	0.0000	1.7 1		Short Grass Pasture Kv= 7.0 fps
	0.3	28	0.0400	1.40		Shallow Concentrated Flow,
	0.5	20	0.0400	1.40		Short Grass Pasture Kv= 7.0 fps
	2.5	129	0.0300	0.87		Shallow Concentrated Flow,
	2.0	123	0.0300	0.07		Woodland Kv= 5.0 fps
	0.5	28	0.0400	1.00		Shallow Concentrated Flow,
	0.5	20	0.0400	1.00		Woodland Kv= 5.0 fps
	1.1	48	0.0200	0.71		Shallow Concentrated Flow,
	1.1	70	0.0200	0.7 1		Woodland Kv= 5.0 fps
	0.3	20	0.0500	1.12		Shallow Concentrated Flow,
	0.0	20	0.0000	1.12		Woodland Kv= 5.0 fps
	5.2	155	0.0100	0.50		Shallow Concentrated Flow,
	5.2	100	0.0100	0.50		Woodland Kv= 5.0 fps
-	17.5	644	Total			77 O G G G G G G G G G G G G G G G G G G
	17.5	044	ı Ulai			

Summary for Subcatchment PDA-4A: PDA-4A

Runoff = 0.14 cfs @ 12.56 hrs, Volume= 0.057 af, Depth> 0.31"

 Area (ac)	CN	Description
0.576	30	Woods, Good, HSG A
1.376	30	Brush, Good, HSG A
0.115	48	Brush, Good, HSG B
0.122	39	>75% Grass cover, Good, HSG A
 0.013	61	>75% Grass cover, Good, HSG B
2.202	32	Weighted Average
2.202		100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.1	16	0.0300	0.04		Sheet Flow,
					Woods: Dense underbrush n= 0.800 P2= 3.40"
3.4	34	0.0300	0.17		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.40"
1.6	90	0.0170	0.91		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.6	58	0.0600	1.71		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
12.7	198	Total			

Summary for Subcatchment PDA-4B: PDA-4B

Runoff = 14.69 cfs @ 12.28 hrs, Volume= 1.767 af, Depth> 1.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=7.00"

	Area	(ac)	CN	Desc	cription				
	0.	540	30	Brus	h, Good, F	ISG A			
	9.	812	39	>75%	% Grass co	over, Good	, HSG A		
	2.	690	61	>759	% Grass co	over, Good	, HSG B		
*	* 1.210 96 Gravel surface								
*	0.	043	98	Equi	pment Pac	d Area			
	14.295 48 Weighted Average								
	14.252 99.70% Pervious Area								
	0.043			0.30	0.30% Impervious Area				
	_		_						
	Tc	Length		Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	7.2	50	0.	0100	0.12		Sheet Flow,		
							Grass: Short n= 0.150 P2= 3.40"		
	3.7	258	0.	0270	1.15		Shallow Concentrated Flow,		
							Short Grass Pasture Kv= 7.0 fps		
	6.2	232	0.	0800	0.63		Shallow Concentrated Flow,		
_							Short Grass Pasture Kv= 7.0 fps		
	17.1	540) To	otal					

Summary for Subcatchment PDA-5A: PDA-5A

Runoff = 1.00 cfs @ 12.48 hrs, Volume= 0.264 af, Depth> 0.43"

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Area (ac) CN Description								
	6.	130	30	Woo	ds, Good,	HSG A		
	0.	164	55	Woo	ds, Good,	HSG B		
0.104 77 Woods, Good, HSG D								
	0.	019	48	Brus	h, Good, H	HSG B		
	0.	006	73	Brus	h, Good, F	HSG D		
		134	39			over, Good		
		009	61			over, Good	, HSG B	
* 0.266 96 Gravel surface								
7.393 34 Weighted Average						age		
	7.	393	100.		00% Pervi	ous Area		
	Tc	Lengt		Slope	Velocity	Capacity	Description	
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)		
	2.9	1	6 (0.0100	0.09		Sheet Flow,	
							Grass: Short n= 0.150 P2= 3.40"	
	6.4	3	4 (0.0440	0.09		Sheet Flow,	
							Woods: Light underbrush n= 0.400 P2= 3.40"	
	2.7	16	6 (0.0420	1.02		Shallow Concentrated Flow,	
							Woodland Kv= 5.0 fps	
	12.0	21	6	Γotal				

Summary for Subcatchment PDA-5B: PDA-5B

Runoff = 0.77 cfs @ 12.14 hrs, Volume= 0.082 af, Depth> 1.15"

_	Area	(ac) C	N Desc	cription				
	0.067 30 Brush, Good, HSG A				HSG A			
0.700 39 >75% Grass cover, Good, HSG A					, HSG A			
* 0.083 96 Gravel surface								
	0.850 44 Weighted Average							
	0.	850	100.	00% Pervi	ous Area			
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	5.5	50	0.0200	0.15		Sheet Flow,		
						Grass: Short n= 0.150 P2= 3.40"		
	1.9	127	0.0260	1.13		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	7.4	177	Total					

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Summary for Subcatchment PDA-5C: PDA-5C

Runoff = 14.33 cfs @ 12.25 hrs, Volume= 1.636 af, Depth> 1.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=7.00"

	Area	(ac)	CN	Desc	cription			
	0.	171	30	Brus	h, Good, F	ISG A		
	8.	834	39	>75%	% Grass co	over, Good	, HSG A	
	3.	489	61	>75%	% Grass co	over, Good	, HSG B	
*	* 0.661 96 Gravel surface							
*	0.	064	98	Equi	pment Pac	d Area		
	13.219 48 Weighted Average							
13.155 99.52% Pervious Area								
	0.064 0.48% Impervious Area							
	т.			N	\/-l:\h.	Oih.	Description	
	Tc	Length		Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	5.5	50	0.	0200	0.15		Sheet Flow,	
							Grass: Short n= 0.150 P2= 3.40"	
	5.2	306	6 0.	0200	0.99		Shallow Concentrated Flow,	
							Short Grass Pasture Kv= 7.0 fps	
	4.1	321	0.	0350	1.31		Shallow Concentrated Flow,	
_							Short Grass Pasture Kv= 7.0 fps	
	14 8	677	, To	ntal				

Summary for Subcatchment PDA-6A: PDA-6A

Runoff = 4.51 cfs @ 12.73 hrs, Volume= 0.973 af, Depth> 0.90"

	Area (ac)	CN	Description
	2.760	30	Woods, Good, HSG A
	0.849	30	Brush, Good, HSG A
	0.002	73	Brush, Good, HSG D
	8.077	39	>75% Grass cover, Good, HSG A
	0.650	80	>75% Grass cover, Good, HSG D
*	0.564	96	Gravel surface
*	0.029	98	Equipment Pad Area
	12.931	41	Weighted Average
	12.902		99.78% Pervious Area
	0.029		0.22% Impervious Area

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	10.2	50	0.0300	0.08	(0.0)	Sheet Flow,
	10.2	00	0.0000	0.00		Woods: Light underbrush n= 0.400 P2= 3.40"
	0.8	38	0.0260	0.81		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	3.5	257	0.0310	1.23		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	8.6	484	0.0350	0.94		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	6.1	202	0.0120	0.55		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.9	52	0.0190	0.96		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	1.1	58	0.0170	0.91		Shallow Concentrated Flow,
	4.0	4.40	0.0440	4.00		Short Grass Pasture Kv= 7.0 fps
	1.2	140	0.0140	1.90		Shallow Concentrated Flow,
	- 4	045	0.0000	4.04		Unpaved Kv= 16.1 fps
	5.1	315	0.0220	1.04		Shallow Concentrated Flow,
	0.0	000	0.0400	0.07		Short Grass Pasture Kv= 7.0 fps
	3.0	369	0.0190	2.07		Shallow Concentrated Flow,
_						Grassed Waterway Kv= 15.0 fps
	40.5	1,965	Total			

Summary for Subcatchment PDA-6B: PDA-6B

Runoff = 13.95 cfs @ 12.19 hrs, Volume= 1.319 af, Depth> 2.12"

	Area	(ac)	CN	Desc	ription			
	0.	045	73	Brus	h, Good, F			
3.466 39 >75% Grass cover, Good, HSG A							, HSG A	
	2.386 61 >75% Grass cover, Good, HSG B							
	1.471 80 >75% Grass cover, Good, HSG D							
*	* 0.093 96 Gravel surface,							
*	0.	011	98	Equi	pment Pac	d Area		
	7.472 55 Weighted Average							
	7.461 99.85% Pervious Area							
	0.011 0.15% Impervious Area							
	Тс	Length	n S	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	4.6	50	0.0	0300	0.18		Sheet Flow,	
							Grass: Short n= 0.150 P2= 3.40"	
	2.2	179	0.0	0360	1.33		Shallow Concentrated Flow,	
							Short Grass Pasture Kv= 7.0 fps	
	5.3	589	0.0	0150	1.84		Shallow Concentrated Flow,	
							Grassed Waterway Kv= 15.0 fps	
	12.1	818	3 To	otal				

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Summary for Subcatchment PDA-6C: PDA-6C

Runoff = 2.22 cfs @ 12.40 hrs, Volume= 0.391 af, Depth> 0.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=7.00"

_	Area	(ac)	CN	Desc	cription			
	4.	594	30	Woo	ds, Good,	HSG A		
0.214 55 Woods, Good, HSG B								
	0.							
0.258 30 Brush, Good, HSG A								
	0.	002	48	Brus	h, Good, F	HSG B		
	0.	211	73	Brus	h, Good, F	HSG D		
	0.	057	39	>75%	% Grass co	over, Good	, HSG A	
_	0.	059	80	>75%	% Grass co	over, Good	, HSG D	
	6.	153	39	Weig	ghted Aver	age		
	6.	153		100.	00% Pervi	ous Area		
	Tc	Length	· S	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	12.0	50	0.	0200	0.07		Sheet Flow,	
							Woods: Light underbrush n= 0.400 P2= 3.40"	
	3.8	172	2 0.	0230	0.76		Shallow Concentrated Flow,	
_							Woodland Kv= 5.0 fps	
	15.8	222	· To	otal				

Summary for Subcatchment PDA-7A: PDA-7A

Runoff = 0.92 cfs @ 12.59 hrs, Volume= 0.212 af, Depth> 0.62"

	Area (ac)	CN	Description
	1.445	30	Woods, Good, HSG A
	0.240	30	Brush, Good, HSG A
	2.280	39	>75% Grass cover, Good, HSG A
*	0.138	96	Gravel surface
	4.103	37	Weighted Average
	4.103		100.00% Pervious Area

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	10.2	50	0.0300	0.08		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.40"
	1.5	59	0.0170	0.65		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	13.9	1,156	0.0390	1.38		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	25.6	1,265	Total			

Summary for Subcatchment PDA-7B: PDA-7B

Runoff = 2.49 cfs @ 12.29 hrs, Volume= 0.346 af, Depth> 1.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=7.00"

	Area	(ac) C	N Des	cription		
	0.	073	30 Brus	sh, Good, F	HSG A	
	3.	526	39 >75°	% Grass c	over, Good	, HSG A
*	0.	283	96 Grav	vel surface	•	,
_	3.	882	43 Wei	ghted Aver	age	
	_	882	,	00% Pervi	•	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
	5.5	50	0.0200	0.15		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.40"
	1.1	93	0.0430	1.45		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.2	33	0.0200	2.28		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	8.4	619	0.0310	1.23		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	15.2	795	Total			

Summary for Reach DP-1: DP-1

Inflow Area = 0.741 ac, 0.00% Impervious, Inflow Depth > 2.12" for 100-Year event Inflow = 0.131 af

Outflow = 1.59 cfs @ 12.12 hrs, Volume= 0.131 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Summary for Reach DP-2: DP-2

Inflow Area = 4.079 ac, 0.00% Impervious, Inflow Depth > 3.58" for 100-Year event

Inflow = 8.79 cfs @ 12.51 hrs, Volume= 1.218 af

Outflow = 8.79 cfs @ 12.51 hrs, Volume= 1.218 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-3: DP-3

Inflow Area = 1.888 ac, 0.00% Impervious, Inflow Depth > 0.31" for 100-Year event

Inflow = 0.11 cfs @ 12.64 hrs, Volume= 0.049 af

Outflow = 0.11 cfs @ 12.64 hrs, Volume= 0.049 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-4: DP-4

Inflow Area = 16.497 ac, 0.26% Impervious, Inflow Depth > 0.04" for 100-Year event

Inflow = 0.14 cfs @ 12.56 hrs, Volume= 0.057 af

Outflow = 0.14 cfs @ 12.56 hrs, Volume= 0.057 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-5: DP-5

Inflow Area = 21.462 ac, 0.30% Impervious, Inflow Depth > 0.26" for 100-Year event

Inflow = 1.00 cfs @ 12.48 hrs, Volume= 0.470 af

Outflow = 1.00 cfs @ 12.48 hrs, Volume= 0.470 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Reach DP-6: DP-6

Inflow Area = 41.249 ac, 0.10% Impervious, Inflow Depth > 0.76" for 100-Year event

Inflow = 12.79 cfs @ 12.50 hrs, Volume= 2.596 af

Outflow = 12.79 cfs @ 12.50 hrs, Volume= 2.596 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Pond 1: Infiltration Basin 1

Inflow Area = 3.882 ac, 0.00% Impervious, Inflow Depth > 1.07" for 100-Year event

Inflow = 2.49 cfs @ 12.29 hrs, Volume= 0.346 af

Outflow = 0.36 cfs @ 15.33 hrs, Volume= 0.342 af, Atten= 85%, Lag= 182.4 min

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Peak Elev= 70.89' @ 15.33 hrs Surf.Area= 6,491 sf Storage= 5,366 cf

Plug-Flow detention time= 167.8 min calculated for 0.341 af (99% of inflow)

Center-of-Mass det. time= 162.1 min (1,076.1 - 913.9)

Volume	Invert	Avail.Sto	rage Storage [Description	
#1	70.00'	14,70	03 cf Custom S	Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio (fee		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
70.0 71.0 72.0	0	5,630 6,602 10,572	0 6,116 8,587	0 6,116 14,703	
Device	Routing	Invert	Outlet Devices		
#1 #2	Discarded Primary	70.00' 71.00'	2.410 in/hr Ext 20.0' long x 0. Head (feet) 0 Coef. (English)	. 5' breadth Bro 20 0.40 0.60	oad-Crested Rectangular Weir 0.80 1.00

Discarded OutFlow Max=0.36 cfs @ 15.33 hrs HW=70.89' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.36 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=70.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2: Infiltration Basin 2

Inflow Area =	12.931 ac, 0	0.22% Impervious, Inflow De	epth > 0.90" for 100-Year event
Inflow =	4.51 cfs @ '	12.73 hrs, Volume=	0.973 af
Outflow =	1.30 cfs @ '	14.85 hrs, Volume=	0.776 af, Atten= 71%, Lag= 127.4 min
Discarded =	0.57 cfs @ '	14.85 hrs, Volume=	0.489 af
Primary =	0.74 cfs @ '	14.85 hrs, Volume=	0.287 af
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 65.97' @ 14.85 hrs Surf.Area= 10,178 sf Storage= 14,234 cf

Plug-Flow detention time= 181.3 min calculated for 0.774 af (80% of inflow) Center-of-Mass det. time= 100.4 min (1,042.6 - 942.1)

Volume	Invert	Avail.Storage	Storage	Description
#1	64.00'	28,548 cf	Custom	Stage Data (Prismatic) Listed below (Recalc)
Elevation	Surf.A	Area In	c.Store	Cum.Store
(feet)	(so	q-ft) (cub	ic-feet)	(cubic-feet)

(cubic-feet)	(cubic-feet)	(sq-ft)	(feet)
0	0	4,958	64.00
5,951	5,951	6,944	65.00
14,566	8,615	10,286	66.00
28 548	13 982	17 678	67.00

Volume

Invert

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Device	Routing	Invert	Outlet Devices
#1	Discarded	64.00'	2.410 in/hr Exfiltration over Surface area
#2	Primary	65.33'	8.0" Round Culvert
	-		L= 21.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 65.33' / 65.00' S= 0.0157 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Secondary	66.00'	20.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.57 cfs @ 14.85 hrs HW=65.97' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.57 cfs)

Primary OutFlow Max=0.74 cfs @ 14.85 hrs HW=65.97' (Free Discharge) —2=Culvert (Inlet Controls 0.74 cfs @ 2.15 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=64.00' (Free Discharge) 3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 3: Infiltration Basin-3

Inflow Area =	7.472 ac, 0.15% Impervious,	Inflow Depth > 2.12" for 100-Year event
Inflow =	13.95 cfs @ 12.19 hrs, Volume	= 1.319 af
Outflow =	1.47 cfs @ 14.12 hrs, Volume	= 1.100 af, Atten= 89%, Lag= 116.0 min
Discarded =	0.85 cfs @ 14.12 hrs, Volume	= 0.793 af
Primary =	0.62 cfs @ 14.12 hrs, Volume	= 0.307 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume	= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 63.95' @ 14.12 hrs Surf.Area= 15,187 sf Storage= 25,046 cf

Plug-Flow detention time= 226.7 min calculated for 1.100 af (83% of inflow) Center-of-Mass det. time= 155.6 min (1,025.7 - 870.1)

Avail Storage Storage Description

Volume	IIIVEIL	Avaii.Sto	age Storage	Description	
#1	62.00'	43,21	1 cf Custom	Stage Data (Pri	smatic) Listed below (Recalc)
Elevation	on Si	urf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
62.0	00	10,997	0	0	
63.0	00	12,633	11,815	11,815	
64.0	00	15,318	13,976	25,791	
65.0	00	19,523	17,421	43,211	
Device	Routing	Invert	Outlet Devices	S	
#1	Discarded	62.00'	2.410 in/hr Ex	filtration over S	Surface area
#2	Primary	63.00'	6.0" Round C	Culvert	
			L= 23.5' CPF	P, projecting, no	headwall, Ke= 0.900
			Inlet / Outlet In	nvert= 63.00' / 6	2.50' S= 0.0213 '/' Cc= 0.900
			n= 0.013 Cor	rugated PE, smo	ooth interior, Flow Area= 0.20 sf
#3	Secondary	64.00'	20.0' long x 0).5' breadth Bro	ad-Crested Rectangular Weir

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Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.85 cfs @ 14.12 hrs HW=63.95' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.85 cfs)

Primary OutFlow Max=0.62 cfs @ 14.12 hrs HW=63.95' (Free Discharge) 2=Culvert (Inlet Controls 0.62 cfs @ 3.18 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=62.00' (Free Discharge) 3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 4: Infiltration Basin-4

Inflow Area =	14.295 ac,	0.30% Impervious, Inflov	v Depth > 1.48" for 100-Year	event
Inflow =	14.69 cfs @	12.28 hrs, Volume=	1.767 af	
Outflow =	1.90 cfs @	14.84 hrs, Volume=	1.489 af, Atten= 87%, Lag=	153.2 min
Discarded =	1.90 cfs @	14.84 hrs, Volume=	1.489 af	
Primary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af	

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 68.51' @ 14.84 hrs Surf.Area= 34,043 sf Storage= 32,120 cf

Plug-Flow detention time= 225.5 min calculated for 1.489 af (84% of inflow) Center-of-Mass det. time= 156.8 min (1,051.8 - 895.0)

Volume	Invert	Avail.Storage	Storage Description
#1	66.50'	35,427 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	67.00'	80,433 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

115,859 cf Total Available Storage

		,		J
Elevation		Surf.Area	Inc.Store	Cum.Store
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)
66.5	50	5,477	0	0
67.0	00	6,706	3,046	3,046
68.0	00	9,279	7,993	11,038
69.0	00	11,959	10,619	21,657
70.0	00	15,580	13,770	35,427
Elevation		Surf.Area	Inc.Store	Cum.Store
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)
67.0	00	1,364	0	0
68.0	00	12,524	6,944	6,944
69.0	00	34,019	23,272	30,216
70.0	00	66,415	50,217	80,433
Device	Routing	Invert	Outlet Devices	

Device	Routing	IIIVEIL	Odilet Devices
#1	Discarded	66.50'	2.410 in/hr Exfiltration over Surface area
#2	Primary	69.00'	20.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

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Discarded OutFlow Max=1.90 cfs @ 14.84 hrs HW=68.51' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 1.90 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=66.50' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 5: Infiltration Basin-5

Inflow Area =	0.850 ac,	0.00% Impervious, Inflow De	epth > 1.15" for 100-Year event
Inflow =	0.77 cfs @	12.14 hrs, Volume=	0.082 af
Outflow =	0.14 cfs @	13.27 hrs, Volume=	0.081 af, Atten= 82%, Lag= 67.6 min
Discarded =	0.14 cfs @	13.27 hrs, Volume=	0.081 af
Primary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 70.42' @ 13.27 hrs Surf.Area= 2,434 sf Storage= 932 cf

Plug-Flow detention time= 65.1 min calculated for 0.081 af (100% of inflow) Center-of-Mass det. time= 63.7 min (967.4 - 903.6)

Volume	Invert	Avail.Sto	rage Storage D	escription	
#1	70.00'	5,98	37 cf Custom S	Stage Data (Pri	smatic) Listed below (Recalc)
Elevatio		ırf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
70.0	00	2,045	0	0	
71.0	00	2,980	2,513	2,513	
72.0	00	3,968	3,474	5,987	
Device	Routing	Invert	Outlet Devices		
#1	Discarded	70.00'	2.410 in/hr Exf	iltration over S	Surface area
#2	Primary	71.00'	20.0' long x 0.	5' breadth Bro	ad-Crested Rectangular Weir
			Head (feet) 0.2	20 0.40 0.60 (0.80 1.00
			Coef. (English)	2.80 2.92 3.0	08 3.30 3.32

Discarded OutFlow Max=0.14 cfs @ 13.27 hrs HW=70.42' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.14 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=70.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 6: Infiltration Basin-6

Inflow Area =	13.219 ac,	0.48% Impervious, Inflow	Depth > 1.48" for 100-Year ever	nt
Inflow =	14.33 cfs @	12.25 hrs, Volume=	1.636 af	
Outflow =	1.55 cfs @	15.32 hrs, Volume=	1.236 af, Atten= 89%, Lag= 184	1.3 min
Discarded =	1.10 cfs @	15.32 hrs, Volume=	1.030 af	
Primary =	0.45 cfs @	15.32 hrs, Volume=	0.206 af	
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af	

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Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 58.95' @ 15.32 hrs Surf.Area= 19,640 sf Storage= 32,035 cf

Plug-Flow detention time= 257.2 min calculated for 1.233 af (75% of inflow)

Center-of-Mass det. time= 163.1 min (1,056.4 - 893.3)

Volume	Invert	Avail.Storage	Storage Description
#1	57.00'	53,030 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	57.00'	2,077 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

55,107 cf Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
57.00	13,283	0	0
58.00	16,082	14,683	14,683
59.00	18,971	17,527	32,209
60.00	22,670	20,821	53,030
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
57.00	150	0	0
58.00	398	274	274
59.00	851	625	899
60.00	1,506	1,179	2,077

Device	Routing	Invert	Outlet Devices
#1	Discarded	57.00'	2.410 in/hr Exfiltration over Surface area
#2	Primary	58.33'	6.0" Round Culvert
	•		L= 21.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 58.33' / 58.00' S= 0.0157 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Secondary	59.00'	20.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=1.10 cfs @ 15.32 hrs HW=58.95' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 1.10 cfs)

Primary OutFlow Max=0.45 cfs @ 15.32 hrs HW=58.95' (Free Discharge) 2=Culvert (Inlet Controls 0.45 cfs @ 2.30 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=57.00' (Free Discharge) 3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Attachment 4
Drawdown and Groundwater Recharge Calculations





Groundwater Recharge Volume Required:

Rv = F x Impervious Area, where:

Rv = Required Recharge Volume [Ac-ft]

F = Target Depth Factor associated with each Hydrologic Soil Group (HSG) [in]

Impervious Area = Total Pavement and Rooftop Area under Post-development Conditions [Ac]

			Impervious Area	Required Recharge	
_			[Acres]	Volume [Ac-ft]	_
HSG "A", use F =	0.6	in	0.136	0.117	
HSG "B", use F =	0.35	in	0.034	0.029	
HSG "C", use F =	0.25	in	0.000	0.000	
HSG "D", use F =	0.1	in	0.000	0.000	_
Total R	0.146	Ac-ft			

Capture Area Adjustment: (Ref: DEP Handbook V.3 Ch.1 P.27-28)

Adjusted Required Recharge Volume = Ca x Rv	0.146 Ac-ft
Capture Area Adjustment Factor = (Total)/(Infil) = Ca =	1.00
Percent Imp. Area Draining to Infiltrative BMPs =	100.0%
Impervious Area Draining to Infiltrative BMPs (infil) =	0.15 Acres
Total Site Impervious Area (Total)=	0.146 Acres

Groundwater Recharge Volume Provided:

ВМР	Provided Recharge Volume [Ac-ft]	
Infiltration Basin 1 =	0.141	_
Infiltration Basin 2 =	0.188	
Infiltration Basin 3 =	0.271	
Infiltration Basin 4 =	1.191	
Infiltration Basin 5 =	0.058	
Infiltration Basin 6 =	0.460	_
Total Provided Recharge Volume =	2.309	Ac-ft

PROVIDED GROUNDWATER RECHARGE VOLUME IS GREATER THAN OR EQUAL TO THE REQUIRED RECHARGE VOLUME, THEREFORE PROPOSED STORMWATER MANAGEMENT DESIGN IS IN COMPLIANCE WITH STANDARD 3.

JOB NO.	1833.112	COMPUTED BY:	NBB	CHECKED BY:	JRM
JOB: 150 T	Tihonet Pond Road PV+ES	DATE:	10/30/20	DATE:	11/3/2020

uudoum Timee –	Rv		Rv = Storage Volume Below Outlet [Ac-ft]		
wdown Time =	(K) (Bottom Area	where:	K= Infiltration Rate [in/hr]		
			Bottom Area= Bottom Area of Recharge System [Ac]		
Infiltration Basin-1					
	Rv =	0.141 Ac-ft			
	K =	2.410 in/hr			
Bottom	n Area =	0.129 Acres			
Drawdown		5.442 Hours	< 72 Hours, Design is in compliance with the standar		
Infiltration Basin-2					
	Rv =	0.188 Ac-ft			
	K =	2.410 in/hr			
Bottom	n Area =	0.144 Acres			
Drawdown	Time =	6.501 Hours	< 72 Hours, Design is in compliance with the standar		
Infiltration Basin-3					
	Rv =	0.271 Ac-ft			
	K =	2.410 in/hr			
Bottom	n Area =	0.252 Acres			
Drawdown	Time =	5.355 Hours	< 72 Hours, Design is in compliance with the standar		
Infiltration Basin-4					
	Rv =	1.191 Ac-ft			
	K =	2.410 in/hr			
Bottom	n Area =	0.126 Acres			
Drawdown	Time =	47.066 Hours	< 72 Hours, Design is in compliance with the standar		
Infiltration Basin-5					
	Rv =	0.058 Ac-ft			
	K =	2.410 in/hr			
	Area =	0.047 Acres	472 House Design is in security with the		
Drawdown	Time =	6.145 Hours	< 72 Hours, Design is in compliance with the standa		
Infiltration Basin-6					
	Rv =	0.460 Ac-ft			
	K =	2.410 in/hr			
Bottom	Area =	0.308 Acres			

Note:

2. Infiltration Rate based on Volume 3, Chapter 1, Table 2.3.3 *Rawls Rates* from the 2008 MA DEP Stormwater Management Handbook.

JOB NO. <u>1833.112</u>	COMPUTED BY:	NBB	CHECKED BY:	JRM
JOB: 150 Tihonet Pond Road PV+ES	DATE:	10/30/2020	DATE:	11/3/2020

^{1.} The infiltration BMPs have been designed to fully drain within 72 hours, therefore the proposed stormwater management design is in compliance with Standard 3.

Stage-Area-Storage for Pond 1: Infiltration Basin 1

			ı <u>-</u>		
Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(acres)	(acre-feet)	(feet)	(acres)	(acre-feet)
70.00	0.129	0.000	71.04 71.06	0.156	0.147
70.02	0.129 0.130	0.003	71.08	0.157 0.159	0.150 0.153
70.04 70.06	0.130	0.005 0.008	71.08	0.159	0.156
70.08	0.130	0.010	71.10	0.161	0.150
70.10	0.131	0.013	71.12	0.165	0.163
70.10	0.131	0.016	71.14	0.167	0.166
70.12	0.132	0.018	71.18	0.168	0.169
70.14	0.133	0.021	71.20	0.170	0.173
70.18	0.133	0.024	71.22	0.172	0.176
70.20	0.134	0.026	71.24	0.174	0.180
70.22	0.134	0.029	71.26	0.176	0.183
70.24	0.135	0.032	71.28	0.177	0.187
70.26	0.135	0.034	71.30	0.179	0.190
70.28	0.135	0.037	71.32	0.181	0.194
70.30	0.136	0.040	71.34	0.183	0.197
70.32	0.136	0.042	71.36	0.185	0.201
70.34	0.137	0.045	71.38	0.187	0.205
70.36	0.137	0.048	71.40	0.188	0.209
70.38	0.138	0.051	71.42	0.190	0.212
70.40	0.138	0.053	71.44	0.192	0.216
70.42	0.139	0.056	71.46	0.194	0.220
70.44	0.139	0.059	71.48	0.196	0.224
70.46	0.140	0.062	71.50	0.198	0.228
70.48	0.140	0.065	71.52	0.199	0.232
70.50	0.141	0.067	71.54	0.201	0.236
70.52	0.141 0.141	0.070	71.56 71.58	0.203 0.205	0.240 0.244
70.54 70.56	0.141	0.073 0.076	71.60	0.205	0.244 0.248
70.58	0.142	0.079	71.62	0.207	0.252
70.60	0.142	0.082	71.64	0.210	0.256
70.62	0.143	0.084	71.66	0.212	0.261
70.64	0.144	0.087	71.68	0.214	0.265
70.66	0.144	0.090	71.70	0.216	0.269
70.68	0.145	0.093	71.72	0.218	0.274
70.70	0.145	0.096	71.74	0.219	0.278
70.72	0.146	0.099	71.76	0.221	0.282
70.74	0.146	0.102	71.78	0.223	0.287
70.76	0.146	0.105	71.80	0.225	0.291
70.78	0.147	0.108	71.82	0.227	0.296
70.80	0.147	0.111	71.84	0.228	0.300
70.82	0.148	0.114	71.86	0.230	0.305
70.84	0.148	0.116	71.88	0.232	0.309
70.86	0.149	0.119	71.90	0.234	0.314
70.88	0.149	0.122	71.92	0.236	0.319
70.90	0.150	0.125	71.94 71.06	0.238	0.324
70.92	0.150	0.128 0.131	71.96	0.239	0.328
70.94 70.96	0.151 0.151	0.131	71.98 72.00	0.241 0.243	0.333 0.338
70.96 70.98	0.151	0.134	12.00	0.243	0.330
70.98 71.00	0.152 0.152	0.137 0.141			
71.02	0.154	0.144			
11.02	J. 10-	J. 177			

Storage (acre-feet)

0.507

0.524

0.541

0.559

0.578

0.596

0.616

0.635

0.655

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Stage-Area-Storage for Pond 2: Infiltration Basin 2

Surface

(acres)

0.338

0.346

0.355

0.363

0.372

0.380

0.389

0.397

0.406

		•	· ·
Elevation (feet)	Surface (acres)	Storage (acre-feet)	Elevation (feet)
64.00	0.114	0.000	66.60
64.05	0.116	0.006	66.65
64.10	0.118	0.012	66.70
64.15	0.121	0.018	66.75
64.20	0.123	0.024	66.80
64.25	0.125	0.030	66.85
64.30	0.127	0.036	66.90
64.35	0.130	0.043	66.95
64.40	0.132	0.049	67.00
64.45	0.134	0.056	
64.50	0.137	0.063	
64.55 64.60	0.139 0.141	0.069 0.076	
64.65	0.141	0.084	
64.70	0.146	0.004	
64.75	0.148	0.098	
64.80	0.150	0.106	
64.85	0.153	0.113	
64.90	0.155	0.121	
64.95	0.157	0.129	
65.00	0.159	0.137	
65.05	0.163	0.145	
65.10	0.167	0.153	
65.15	0.171	0.161	
65.20	0.175	0.170	
65.25	0.179	0.179	
<mark>65.30</mark> 65.35	0.182	0.188	
65.40	0.186 0.190	0.197 0.207	
65.45	0.190	0.216	
65.50	0.194	0.226	
65.55	0.202	0.236	
65.60	0.205	0.246	
65.65	0.209	0.256	
65.70	0.213	0.267	
65.75	0.217	0.278	
65.80	0.221	0.289	
65.85	0.225	0.300	
65.90	0.228	0.311	
65.95	0.232	0.323	
66.00	0.236	0.334	
66.05	0.245	0.346	
66.10 66.15	0.253 0.262	0.359 0.372	
66.20	0.202	0.385	
66.25	0.270	0.399	
66.30	0.273	0.413	
66.35	0.296	0.427	
66.40	0.304	0.442	
66.45	0.312	0.458	
66.50	0.321	0.474	
66.55	0.329	0.490	
			ı

Storage (acre-feet)

0.820

0.841

0.862

0.883

0.904

0.926

0.948

0.970

0.992

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Stage-Area-Storage for Pond 3: Infiltration Basin-3

Surface

(acres)

0.410

0.414

0.419

0.424

0.429

0.434

0.438

0.443

0.448

Elevation

(feet)

64.60

64.65

64.70

64.75

64.80

64.85

64.90

64.95

65.00

Elevation	Surface	Storage
(feet)	(acres)	(acre-feet)
62.00	0.252	0.000
62.05	0.254	0.013
62.10	0.256	0.025
62.15 62.20	0.258 0.260	0.038
62.25	0.260	0.051 0.064
62.30	0.262	0.004
62.35	0.265	0.091
62.40	0.267	0.104
62.45	0.269	0.117
62.50	0.271	0.131
62.55	0.273	0.144
62.60	0.275	0.158
62.65	0.277	0.172
62.70	0.279	0.186
62.75	0.280	0.200
62.80	0.282	0.214
62.85	0.284	0.228
62.90	0.286	0.242
62.95 <mark>63.00</mark>	0.288 <mark>0.290</mark>	0.257 <mark>0.271</mark>
63.05	0.290	0.286
63.10	0.296	0.300
63.15	0.299	0.315
63.20	0.302	0.330
63.25	0.305	0.345
63.30	0.309	0.361
63.35	0.312	0.376
63.40	0.315	0.392
63.45	0.318	0.408
63.50	0.321	0.424
63.55	0.324	0.440
63.60	0.327	0.456 0.473
63.65 63.70	0.330 0.333	0.489
63.75	0.336	0.469
63.80	0.340	0.523
63.85	0.343	0.540
63.90	0.346	0.557
63.95	0.349	0.574
64.00	0.352	0.592
64.05	0.357	0.610
64.10	0.362	0.628
64.15	0.366	0.646
64.20	0.371	0.664
64.25	0.376	0.683
64.30 64.35	0.381 0.386	0.702 0.721
64.40	0.386	0.721
64.45	0.395	0.760
64.50	0.400	0.780
64.55	0.405	0.800

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Stage-Area-Storage for Pond 4: Infiltration Basin-4

Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(acres)	(acre-feet)	(feet)	(acres)	(acre-feet)
66.50	0.126	0.000	69.10	1.138	1.301
66.55	0.129	0.006	69.15	1.180	1.358
66.60	0.131	0.013	69.20	1.221	1.418
66.65	0.134	0.019	69.25	1.262	1.481
66.70	0.137	0.026	69.30	1.304	1.545
66.75	0.140	0.033	69.35	1.345	1.611
66.80	0.143	0.040	69.40	1.386	1.679
66.85	0.145	0.047	69.45	1.428	1.750
66.90	0.148	0.055	69.50	1.469	1.822
66.95	0.151	0.062	69.55	1.510	1.896
67.00	0.185	0.070	69.60	1.552	1.973
67.05	0.201	0.080	69.65	1.593	2.052
67.10 67.15	0.217	0.090	69.70	1.634	2.132
67.15 67.20	0.233	0.101	69.75	1.676	2.215
67.20	0.248	0.113	69.80	1.717	2.300
67.25	0.264	0.126 0.140	69.85	1.758	2.387 2.476
67.30 67.35	0.280 0.296	0.140 0.154	69.90 69.95	1.800 1.841	2.567
67.40	0.290	0.169	70.00	1.882	2.660
67.45	0.311	0.185	70.00	1.002	2.000
67.50	0.343	0.202			
67.55	0.359	0.202			
67.60	0.374	0.238			
67.65	0.390	0.257			
67.70	0.406	0.277			
67.75	0.422	0.298			
67.80	0.437	0.319			
67.85	0.453	0.341			
67.90	0.469	0.364			
67.95	0.485	0.388			
68.00	0.501	0.413			
68.05	0.528	0.439			
68.10	0.556	0.466			
68.15	0.584	0.494			
68.20	0.612	0.524			
68.25	0.639	0.555			
68.30	0.667	0.588			
68.35	0.695	0.622			
68.40	0.723	0.657			
68.45	0.750	0.694			
68.50	0.778	0.732			
68.55	0.806	0.772			
68.60	0.834	0.813			
68.65	0.861	0.855			
68.70	0.889	0.899			
68.75	0.917	0.944			
68.80	0.945	0.991			
68.85	0.972	1.039			
68.90	1.000	1.088			
68.95	1.028	1.139			
69.00	1.056	1.191			
69.05	1.097	1.245			

Stage-Area-Storage for Pond 5: Infiltration Basin-5

	C. mf a a a	Ctanana	l ====================================	Conford	Ctonomo
Elevation (feet)	Surface (acres)	Storage (acre-feet)	Elevation (feet)	Surface (acres)	Storage (acre-feet)
70.00	0.047	0.000	71.04	0.069	0.060
70.02	0.047	0.001	71.04	0.009	0.062
70.02	0.047	0.001	71.00	0.070	0.063
70.04	0.048	0.002	71.00	0.070	0.065
70.08	0.049	0.004	71.10	0.071	0.066
70.10	0.049	0.005	71.12	0.072	0.067
70.12	0.050	0.006	71.16	0.072	0.069
70.12	0.050	0.007	71.18	0.072	0.070
70.16	0.050	0.008	71.20	0.073	0.072
70.18	0.051	0.009	71.22	0.073	0.073
70.20	0.051	0.010	71.24	0.074	0.075
70.22	0.052	0.011	71.26	0.074	0.076
70.24	0.052	0.012	71.28	0.075	0.078
70.26	0.053	0.013	71.30	0.075	0.079
70.28	0.053	0.014	71.32	0.076	0.081
70.30	0.053	0.015	71.34	0.076	0.082
70.32	0.054	0.016	71.36	0.077	0.084
70.34	0.054	0.017	71.38	0.077	0.085
70.36	0.055	0.018	71.40	0.077	0.087
70.38	0.055	0.019	71.42	0.078	0.088
70.40	0.056	0.020	71.44	0.078	0.090
70.42	0.056	0.022	71.46	0.079	0.092
70.44	0.056	0.023	71.48	0.079	0.093
70.46	0.057	0.024	71.50	0.080	0.095
70.48	0.057	0.025	71.52	0.080	0.096
70.50	0.058	0.026	71.54	0.081	0.098
70.52	0.058	0.027	71.56	0.081	0.100
70.54	0.059	0.028	71.58	0.082	0.101
70.56	0.059	0.030	71.60	0.082	0.103
70.58	0.059	0.031	71.62	0.082	0.104
70.60	0.060	0.032	71.64	0.083	0.106
70.62	0.060	0.033	71.66	0.083	0.108
70.64 70.66	0.061 0.061	0.034 0.036	71.68 71.70	0.084 0.084	0.109 0.111
70.68	0.061	0.037	71.70	0.084	0.111
70.70	0.062	0.038	71.72	0.085	0.115
70.72	0.062	0.039	71.76	0.086	0.116
70.74	0.063	0.041	71.78	0.086	0.118
70.76	0.063	0.042	71.80	0.087	0.120
70.78	0.064	0.043	71.82	0.087	0.121
70.80	0.064	0.044	71.84	0.087	0.123
70.82	0.065	0.046	71.86	0.088	0.125
70.84	0.065	0.047	71.88	0.088	0.127
70.86	0.065	0.048	71.90	0.089	0.128
70.88	0.066	0.050	71.92	0.089	0.130
70.90	0.066	0.051	71.94	0.090	0.132
70.92	0.067	0.052	71.96	0.090	0.134
70.94	0.067	0.054	71.98	0.091	0.136
70.96	0.068	0.055	72.00	0.091	0.137
70.98	0.068	0.056			
71.00	0.068	0.058			
71.02	0.069	0.059			

Storage (acre-feet)

1.051

1.077

1.103

1.130

1.156

1.183

1.210 1.238

1.265

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Stage-Area-Storage for Pond 6: Infiltration Basin-6

Surface

(acres)

0.515

0.520

0.525

0.530

0.535

0.540

0.545

0.550 **0.555**

Elevation

(feet)

59.60

59.65

59.70

59.75

59.80

59.85

59.90

59.95

60.00

		•
Elevation	Surface	Storage
(feet)	(acres)	(acre-feet)
57.00	0.308	0.000
57.05	0.311	0.015
57.10	0.315	0.031
57.15	0.318	0.047
57.20	0.322	0.063
57.25	0.326	0.079
57.30	0.329	0.096
57.35	0.333	0.112
57.40	0.336	0.129
57.45	0.340	0.146
57.50	0.343	0.163
57.55	0.346	0.180
57.60	0.350	0.197
57.65	0.353	0.215
57.70	0.357	0.233
57.75	0.361	0.251
57.80	0.364	0.269
57.85	0.368	0.287
57.90	0.371	0.306
57.95	0.375	0.324
58.00	0.378	0.343
58.05	0.382	0.362
58.10	0.386	0.381
58.15	0.390	0.401
58.20	0.394	0.420
58.25	0.398	0.440
<mark>58.30</mark>	<mark>0.401</mark>	<mark>0.460</mark>
58.35	0.405	0.480
58.40	0.409	0.500
58.45	0.413	0.521
58.50	0.417	0.542
58.55	0.421	0.563
58.60	0.425	0.584
58.65	0.429	0.605
58.70	0.433	0.627
58.75	0.436	0.648
58.80	0.440 0.444	0.670
58.85 58.90	0.448	0.692
58.95	0.452	0.737
59.00	0.456	0.760
59.05	0.461	0.783
59.10	0.466	0.806
59.15	0.471	0.830
59.20	0.476	0.853
59.25	0.481	0.877
59.30	0.486	0.901
59.35	0.491	0.926
59.40	0.496	0.950
59.45	0.501	0.975
59.50	0.506	1.000
59.55	0.510	1.026

Attachment 5
Site Owner's Manual

Site Owner's Manual

150 Tihonet Road PV+ES Project

150 Tihonet Road (aka 0 & 169 Tihonet Road) Wareham, Massachusetts

Prepared for:



Borrego Solar Systems, Inc. 55 Technology Drive, Suite 102 Lowell, MA 01851

Prepared by:



November 3, 2020

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1.0 INTRODUCTION

The Site Owner's Manual complies with the Long-Term Pollution Prevention Plan (Standard 4) and the Long-Term Operation and Maintenance Plan (Standard 9) requirements of the 2008 Massachusetts Department of Environmental Protection (MassDEP) Stormwater Handbook. The Manual outlines source control and pollution prevention measures and maintenance requirements of stormwater best management practices (BMPs) associated with the proposed development.



2.0 SITE OWNER'S AGREEMENT

2.1 Operation and Maintenance Compliance Statement

Site Owner: Borrego Solar Systems, Inc.

55 Technology Drive, Suite 102

Lowell, MA 01851

Responsible Party: Borrego Solar Systems, Inc.

Borrego Solar Systems, Inc. or their successors shall maintain ownership of the on-site stormwater management system as well as the responsibility for operation and maintenance during the post-development stages of the project. The site has been inspected for erosion and appropriate measures have been taken to permanently stabilize any eroded areas. All aspects of stormwater best management practices (BMPs) have been inspected for damage, wear and malfunction, and appropriate steps have been taken to repair or replace the system or portions of the system so that the stormwater at the site may be managed in accordance with the Stormwater Management Standards. Future responsible parties shall be notified of their continuing legal responsibility to operate and maintain the BMPs. The operation and maintenance plan for the stormwater BMPs is being implemented.

Responsible Party Signature	Date

2.2 Stormwater Maintenance Easements

The Site Owner will have access to all stormwater practices for inspection and maintenance, including direct maintenance access by heavy equipment to structures requiring regular maintenance.

2.3 Record Keeping

The Site Owner shall maintain a rolling log in which all inspections and maintenance activities for the past three years shall be recorded. The Operation and Maintenance Log includes information pertaining to inspections, repairs, and disposal relevant to the project's stormwater management system. The Log is located in Appendix A.

The Operation and Maintenance Log shall be made available to the Conservation Commission and the DEP upon request. The Conservation Commission and the DEP shall be allowed to enter and inspect the premises to evaluate and ensure that the responsible party complies with the maintenance requirements for each BMP.



2.4 Training

Employees involved in grounds maintenance and emergency response will be educated on the general concepts of stormwater management and groundwater protection. The Site Owner's Manual will be reviewed with the maintenance staff. The staff will be trained on the proper course of action for specific events expected to be incurred during routine maintenance or emergency situations.



3.0 LONG-TERM POLLUTION PREVENTION PLAN

In compliance with Standard 4 of the 2008 DEP Stormwater Management Handbook, this section outlines source control and pollution prevention measures to be employed on-site after construction.

3.1 Storage of Materials and Waste

The site shall be kept clear of trash and debris at all times. Certain materials and waste products shall be stored inside or outside upon an impervious surface and covered, as required by local and state regulations.

3.2 Vehicle Washing

No commercial vehicle washing shall take place on-site.

3.3 Routine Inspections and Maintenance of Stormwater BMPs

See Section 4.0 Long-Term Operation and Maintenance Plan, for routine inspection and maintenance requirements for all proposed stormwater BMPs.

3.4 Spill Prevention and Response

A contingency plan shall be implemented to address the spill or release of petroleum products and hazardous materials and will include the following measures:

- 1. Equipment necessary to quickly attend to inadvertent spills or leaks shall be stored on-site in a secure but accessible location. Such equipment shall include but not be limited to the following: safety goggles, chemically resistant gloves and overshoe boots, water and chemical fire extinguishers, sand and shovels, suitable absorbent materials, storage containers and first aid equipment (i.e. Indian Valley Industries, Inc. 55-gallon Spill Containment kit or approved equivalent).
- 2. Spills or leaks shall be treated properly according to material type, volume of spillage and location of spill. Mitigation shall include preventing further spillage, containing the spilled material in the smallest practical area, removing spilled material in a safe and environmentally-friendly manner, and remediation of any damage to the environment.
- 3. For large spills, MassDEP Hazardous Waste Incident Response Group shall be notified immediately at 888-304-1133 and an emergency response contractor shall be consulted.

3.5 Maintenance of Grassed Areas

Grassed areas shall be maintained regularly by the facility operator. Vegetated and landscaped BMPs will be maintained as outlined in Section 4.0.



3.6 Snow and Deicing Chemical Management

Snow removal at the site shall comply with the following requirements:

- Plowed snow shall not be placed in wetland resource areas or associated buffer zones. The following maintenance measures shall be undertaken at all snow disposal sites:
 - o Debris shall be cleared from an area prior to using it for snow disposal.
 - Debris and accumulated sediments shall be cleared from the site and properly disposed of at the end of the snow season and no later than May
 15
- The use of deicing materials and sand shall not be used at the proposed project site to protect off-site areas.



4.0 LONG-TERM OPERATION AND MAINTENANCE PLAN

This section outlines the general maintenance activities for the stormwater best management practices (BMPs) associated with the proposed stormwater management system and identifies the long-term inspection and maintenance requirements for each BMP.

4.1 Stormwater Management System Components

The following table outlines the type and quantity of the BMPs and their general location. Please reference the site plan(s) provided in the Figures section for exact location.

BMP Type	Quantity	Location
Infiltration	6	Throughout the site
Basin	О	Throughout the site

4.2 Inspection and Maintenance Schedules

4.2.1 Infiltration Basins

Infiltration basins shall be inspected and maintained after major storm events (rainfall totals greater than 2.5 inches in 24 hours) during the first three months of operation and twice a year and when there are discharges through the outlet control structure thereafter. Additionally, all pretreatment BMPs shall be inspected in accordance with the minimal requirements specified for those practices and after all major storm events. Inspections shall include the following measures:

- During and after major storm events, the length of time standing water remains in the basin shall be recorded.
 - o If the time is greater than 72 hours, thoroughly inspect the basin for signs of clogging.
 - o A corrective action plan shall be developed by a qualified professional to restore infiltrative function. The Site Owner shall take immediate action to implement these corrective measures.
- Examine the outlet structure for evidence of clogging or outflow release velocities that are greater than the design velocity.
- Identify areas of sediment accumulation, differential settlement, cracking, and erosion within the basin.
- Inspect embankments for leakage and tree growth.
- Examine the health of the vegetation within the basin and on the embankments.



Corrective measures shall be taken immediately as warranted by the inspections. If any evidence of hydrocarbons is found during inspection, the material shall be immediately removed using absorbent pads or other suitable measures and legally disposed.

Preventative maintenance shall include the following activities:

- Mow the buffer area and basin bottom and side slopes, if vegetated.
- Remove trash, debris, and accumulated organic matter.
- Remove clippings after mowing.

4.2.2 Stormwater Outfalls

Flared end sections and associated riprap aprons, and overflow spillways shall be inspected at least once per year and after major storm events (rainfall totals greater than 2.5 inches in 24 hours) to ensure that the stability of the outlet area is maintained. The outfall area shall be kept clear of debris such as trash, branches, and sediment. Repairs shall be made immediately if riprap displacement or downstream channel scour is observed.

4.3 Estimated Operation and Maintenance Budget

An operations and maintenance budget was prepared to approximate the annual cost of the inspections required in compliance with the DEP Stormwater Management Policy. The table below estimates the annual cost to inspect and maintain each proposed BMP, based on the requirements in Section 4.2.

BMP Type	# of BMPS	Annual O&M Cost (per BMP) ¹	Total Cost
Infiltration Basin	6	\$50-\$100	\$300-\$600
Riprap Spillway/Flared Ends	6	\$200-\$400	\$1200-\$2400
		Total	\$1,500 - \$3,000

4.4 Public Safety Features

The site is not open to the public. A locked vehicle gate will be located at the entrance to the gravel access driveway. In addition, a 7' chain-link-fence will surround the array. Operation and maintenance of the facility will be conducted in accordance with the safety requirements of the facility operator and applicable OSHA regulations.

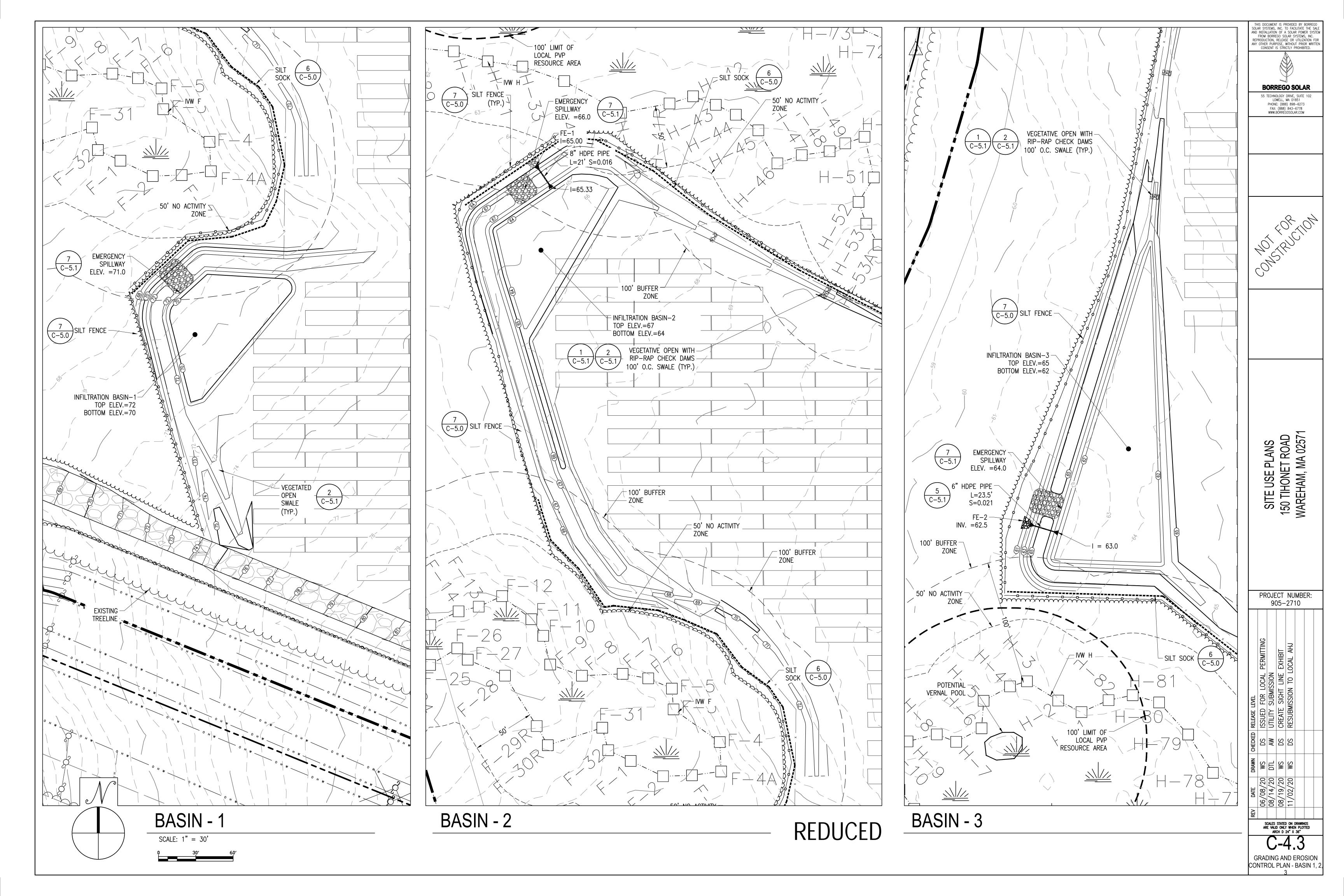
¹ Annual maintenance cost is based on estimate of the cost to complete all inspection and maintenance measures outlined in Section 4.2. For BMPs that require sediment removal at regular intervals (i.e. every 5 or 10 years), the annual cost includes the annual percentage of that cost.

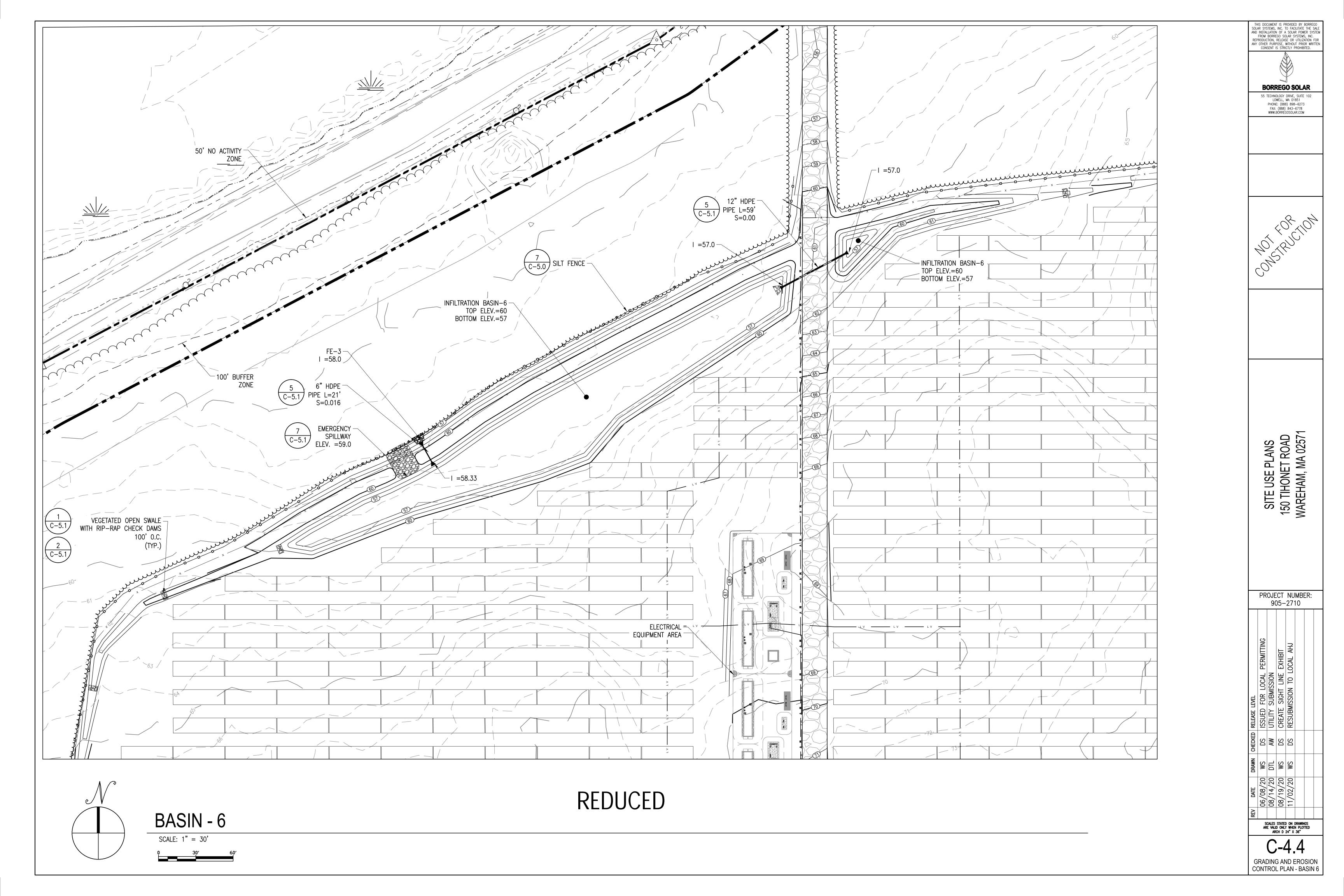


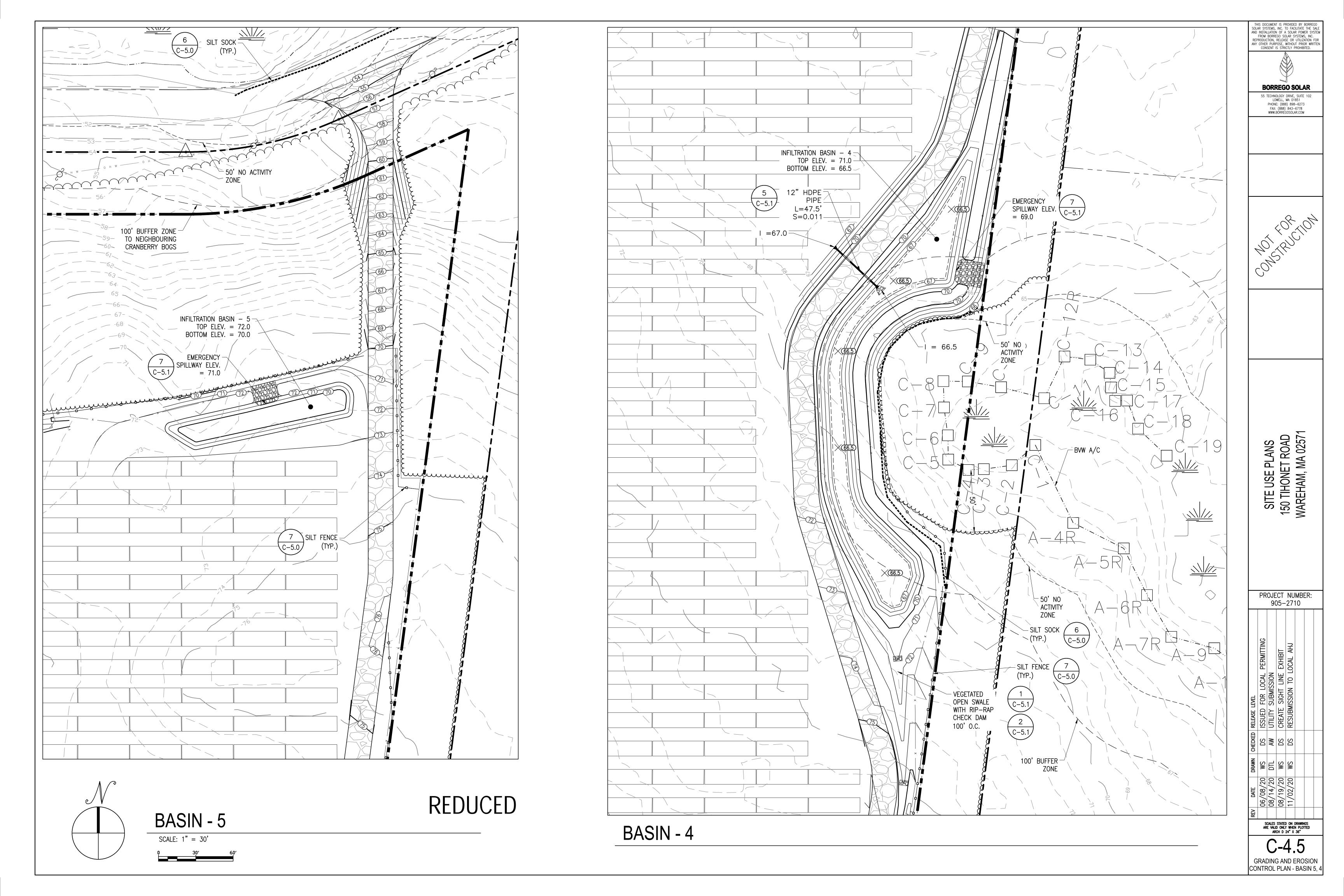
Figures

Figure 1: Site Plan









Appendices



Appendix A

Operation and Maintenance Log



Operation and Maintenance Log Wareham, Massachusetts 1833112RP005B

OPERATION AND MAINTENANCE LOG

This template is intended to comply with the operation and maintenance log requirements of the 2008 MassDEP Stormwater Management Handbook. Copies of this log should be made for all inspections and kept on file for three years from the inspection date.

Name/Company of Inspector:
Date/Time of Inspection:
Weather Conditions: (Note current weather and any recent precipitation events)

Stormwater BMP	Inspection Observations	Actions Required

Appendix B

List of Emergency Contacts



Operation and Maintenance Log Wareham, Massachusetts 1833112RP005B

<u>List of Emergency Contacts</u>

MassDEP Hazardous Waste Incident Response Group (617) 792-7653

Town of Wareham Municipal Maintenance 95 Charge Pond Road Wareham, MA 02571 (508) 295-5300

Town of Wareham Fire Department 20 Church Street Wareham, MA 02571 (508) 295-2973

Town of Wareham Police Department 2515 Cranberry Highway Wareham, MA 02571 (508) 295-1212



Attachment 6
Stormwater Pollution Prevention Plan



Draft Stormwater Pollution Prevention Plan

150 Tihonet Road PV+ES Project

150 Tihonet Road (aka 0 & 169 Tihonet Road) Wareham, Massachusetts





Borrego Solar Systems, Inc. 55 Technology Drive, Suite 102 Lowell, MA 01851

Prepared by:



November 3, 2020

Stormwater Pollution Prevention Plan (SWPPP)
Wareham, Massachusetts
1833112RP006B

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HISTORIC PRESERVATION DOCUMENTATION



APPENDIX M:

1.0 CONTACT INFORMATION/RESPONSIBLE PARTIES

1.1 OPERATOR(S)/ SUBCONTRACTORS

()				*
Operator(s)	T			
Company:	Borrego Solar Systems,	, Inc.		
Name:				
Address:			•	
City:		State:	ZIP Code:	
Telephone:		Email:		*
Company:	TBD			
Name:				
Address:				
City:		State:	ZIP Code:	
Telephone:		Email:		
Subcontrac	tor(s)			
Company:	TBD			
Name:				
Address:				
City:		State:	ZIP Code:	
Telephone:		Email:		
Area of Con	trol:	Site Work (Contractor	
24-Hour Em	ergency Contact			
Company:	TBD			
Name:				
Telephone:				



1.2 STORMWATER TEAM

SWPPP Preparer

•••••				
Company:	Beals and Thomas, Inc.			
Name:	Nathaniel Bautz, EIT			
Address:	144 Turnpike Road			
City:	Southborough	State:	MA ZIP Code:	01772
Telephone:	508-366-0560	Email:		

Personnel Responsible for Installation & Maintenance of Stormwater BMPs

Company:	TBD		
Name:			
Address:			
City:		State:	ZIP Code:
Telephone:		Email:	•

Inspection Personnel

Company:	TBD			
Name:				
Address:				
City:		State:	ZIP Code:	
Telephone:		Email:		



Personnel Responsible for Taking Corrective Actions

Company:	TBD		
Name:			
Address:			
City:		State:	ZIP Code:
Telephone:		Email:	





2.0 SITE EVALUATION, ASSESSMENT AND PLANNING

2.1 PROJECT/SITE INFORMATION

Project/Site	t Road P	V+ES P	roject			
Project Stre	t Road	t Road				
City:	Wareham		State:	MA	ZIP Code:	02571
County or S	Similar Subdivi	sion:	Plymou	th		
Latitude:	41°47'05" N		Longitu	de:	70°43'09"V	V
☐ US ☐ EP ☐ GF	Determining I GGS Topograph A Website PS her (please spec	ic Map (spec	cify scale			
□ N	AD 27 AD 83	□ WG □ Unk	S 84 known	or locate	ed on a prop	erty of religious of
	ificance to an I		y lands,	or rocati	☐ Yes	No No
(including th		ın reservatio	n if applic	cable), o		a of Indian country ian country, provide
Is this projec	et considered a	federal facili	ty?		☐ Yes	⊠ No
Are you app the 2017 CG		t coverage as	s a "feder	al opera	tor" as define	ed in Appendix A of
NPDES proj	ect or permit tr	acking numb	er: TBD			



	Is this project in response to a public emergency? Yes No
	If yes, document the cause of the public emergency (e.g., natural disaster, extreme flooding conditions), information substantiating its occurrence (e.g., state disaster declaration), and a description of the construction necessary to reestablish effective public services:
2.2 NAT	URE AND SEQUENCE OF CONSTRUCTION ACTIVITY
2.2.1	Function of the Construction Activity
	Function of the construction activity:
2.2.2	Single-Family Residential Multi-Family Residential Institutional Utility Building Demolition Will there be demolition of any structure built or renovated before January 1, 1980? If yes, do any of the structures being demolished have at least 10,000 square feet of floor space? Commercial Industrial Highway or Road Construction Other (please specify): Renew. Energy No
2.2.3	Agricultural Land
	Was the pre-development land use used for agriculture? ☐ Yes ☐ No
2.2.4	Estimated Project Dates
	Estimated Project Start Date: TBD Estimated Project Completion Date: TBD

2.1.1 Emergency-Related Projects



Estimated Timeline of Activity	Construction Activity and BMP Descriptions
TBD	 Before any site grading activities begin Stake Limit of Construction. Workers shall be informed that no construction activity is to occur beyond this limit at any time. Install sediment controls as shown on the plans. An adequate stockpile of erosion control materials shall be on site at all times for emergency or routine replacement and shall include materials to repair silt fences, compost mulch tubes, or any other devices planned for use during construction. Construct stabilized construction exits. Construct staging and materials storage area. Install temporary sanitary facilities and dumpsters.
TBD	 Site grading Begin overall site grading. Establish topsoil stockpile. Install silt fences around stockpile. Build stormwater basins and complete overall site grading. Disturbed areas where construction will cease for more than 14 days shall be stabilized with erosion controls.
TBD	Infrastructure (utilities, solar panels, etc.) 1. Construct temporary concrete washout area. 2. Install utilities, solar panels.
TBD	 Final stabilization and landscaping Finalize grading activities. Remove all temporary erosion control BMPs and stabilize any areas disturbed by their removal with erosion controls. Monitor stabilized areas until final stabilization is reached.

2.3 SOILS, SLOPES, VEGETATION, AND CURRENT DRAINAGE PATTERNS

Soil type(s): The Natural Resources Conservation Service (NRCS) lists the on-site soils types as predominantly hydrologic soil classes A, B and D. The soil groups include Carver coarse sand, Poquonock sand, Birchwood sand, Massasoit-complex, Canton fine sandy loam, Windsor loamy sand, and Udipsamments.

Carver sands are excessively drained soils formed in layers of coarse and very coarse sand that contain less than twenty percent rock fragments, most of which are fine gravel. Carver soils are level to steep soils on pitted and dissected outwash plains and moraines.



Poquonock sand is a very deep, well-drained soil formed in sandy eolian or glacifluvial material over loamy or sandy lodgement till on uplands. They are moderately deep to a densic contact and very deep bedrock.

Birchwood soils are very deep, moderately well drained soil formed in sandy eolian deposits underlain by loamy dense glacial till. Birchwood soils are on the lower side slopes and toe slopes and on gently sloping areas of ground moraines and drumlins.

Canton fine sandy loam are gently sloping to sloping, well drained soils are on side slopes of glacial upland hills and ridges. Boulders and stones cover 10 to 35 percent of the surface.

Windsor sands are gently sloping, excessively drained soil is on terraces, outwash plains, kames, and eskers. Areas are irregular in shape and range from 2 to 100 acres.

Slopes: 1-30%

Drainage Patterns: Runoff from the site drains to the south, east, and west.

Vegetation: The existing site is comprised of woodland area.

2.4 CONSTRUCTION SITE ESTIMATES

Total construction site area to be disturbed: 54 acres

Maximum area to be disturbed at one time: 54 acres

Percentage impervious area before construction: <1%

Runoff curve number before construction: 38

Percentage impervious area after construction: <1%

Runoff coefficient after construction: 45



2.5 DISCHARGE INFORMATION

2.5.1	Description of Receiving Storm Sewer Systems
	Does your project/site discharge stormwater into a Municipal Separate Storm Sewer System (MS4)?
2.5.2	Receiving Waters
	Runoff from the site drains to Tihonet Pond to the west, and to existing wetlands and potential vernal pools to the west which eventually flow to Tihonet Pond. Runoff flows east to an off-site wetland system. Runoff from the northern portion of the site drains to stream on the north side of the property.
2.5.3	Impaired Waters/ TMDLs
	Has the surface water been listed as "impaired?" ☐ Yes ☐ No
	If yes, list the pollutant(s) causing the impairment: N/A
	Describe the method(s) used to determine whether or not your project site discharges to an impaired water:
	Has a TMDL been completed? ☐ Yes ☐ No
	If yes, list the title of the TMDL document: N/A
Ť	List the pollutant(s) for which there is a TMDL: N/A
2.5.4	Tier 2, 2.5, or 3 Waters
	Is this surface water designated as a Tier 2, 2.5 or 3 water? ⊠ Yes ☐ No
	If yes specify which Tier the surface water is designated as: Tier 2

2.6 UNIQUE SITE FEATURES AND SENSITIVE AREAS

The site contain wetlands and a potential vernal pools; these features will not be impacted by the project. The hydrology of these areas is maintained by the proposed stormwater design. Additionally, they will be protected by sediment control barriers as needed to avoid potential sedimentation.



2.7 CONSTRUCTION SUPPORT ACTIVITIES

Construction support activities are not required for the project.

2.8 POTENTIAL SOURCES OF POLLUTION

2.8.1 Potential Sources of Sediment

- Grading and site excavation operations
- Vehicle tracking
- Soil stripping and stockpiling

2.8.2 Potential Sources of Non-Sediment Pollutants

- Combined Staging Area small fueling activities, minor equipment maintenance, sanitary facilities, and hazardous waste storage.
- Materials Storage Area general building materials, solvents, adhesives, paints, aggregates, trash, and so on.
- Construction Activity concrete pouring, and array construction
- Concrete Washout Area

Material/	Physical Description	Stormwater Pollutants	Location ^[1]
Chemical			
[2]Fertilizer	Liquid or solid grains	Nitrogen, phosphorous	Newly seeded areas
Cleaning solvents	Colorless, blue, or yellow-green liquid	Perchloroethylene, methylene chloride, trichloroethylene, petroleum distillates	No equipment cleaning allowed in project limits
Curing compounds	Creamy white liquid	Naphtha	Concrete Equipment Pads
Hydraulic oil/fluids	Brown oily petroleum hydrocarbon	Mineral oil	Leaks or broken hoses from equipment
Gasoline	Colorless, pale brown or pink petroleum hydrocarbon	Benzene, ethyl benzene, toluene, xylene, MTBE	Contractor staging area
Diesel Fuel	Clear, blue-green to yellow liquid	Petroleum distillate, oil & grease, naphthalene, xylenes	Contractor staging area
Kerosene	Pale yellow liquid petroleum hydrocarbon	Coal oil, petroleum distillates	Contractor staging area
Antifreeze/ coolant	Clear green/yellow liquid	Ethylene glycol, propylene glycol, heavy metals (copper, lead, zinc)	Leaks or broken hoses from equipment
Sanitary toilets	Various colored liquid	Bacteria, parasites, and viruses	Staging area

^[1] Area where material/chemical is used on-site.

^[2] Use of fertilizers containing nitrogen and/or phosphorus in ratios greater than recommended by the manufacture must be documented.



2.9 SITE PLANS

The Existing Conditions Plan shows the undeveloped site and its current features. The Site Plans show the developed site.

hese Site Plans include:	
☐ Delineation of construction phasing, if applicable	
Areas of soil disturbance and areas that will not be disturbed	
Direction(s) of stormwater flow and approximate slopes before and after magrading activities	ajor
Natural features to be preserved	
Locations of major structural and non-structural BMPs identified in the SW	PPP
Location(s) of sediment, soil or other construction materials will be stockpil	ed
Locations of stabilization measures	
Locations of off-site material, waste, borrow, or equipment storage areas	
□ Location of all waters of the U.S., including wetlands on or near the site. In if water bodies are listed as impaired, or are identified as Tier 2, 2.5 or 3 was a simple of the U.S., including wetlands on or near the site. Including wetlands on or near the site.	
Boundary lines of any natural buffers,	
□ Locations of stormwater discharges and/or locations where authorized non- stormwater will be discharged to surface water(s)	
□ Locations of storm drain inlets and stormwater control measures on the site the immediate vicinity of the site	and in
Locations of all pollutant-generating activities	
Locations where polymers, flocculants, or other treatment chemicals will be and stored	used
Areas of federally listed critical habitat for endangered or threatened species	\$
See Appendix B: Site Plans	



3.0 COMPLIANCE WITH APPLICABLE FEDERAL & STATE REQUIREMENTS

3.1 ENDANGERED SPECIES CERTIFICATION

Are endangered	d or threatened	species and	critical	habitats	on or	near	the project	area'
Yes	⊠No	-						

Describe how this determination was made:

The U.S. Fish & Wildlife Service listed the Northern Long-eared Bat (*Myotis septentrionalis*) as a Threatened species under the Endangered Species Act (ESA, 50 CFR 17.11) on April 2, 2015 and mapped the full state of Massachusetts as habitat. The Northern Long-Eared Bat is also listed as Endangered under the Massachusetts Endangered Species Act (MESA, M.G.L. c. 131 A).

Projects that result in tree removal activities shall comply with the 4(d) rule under the ESA, which states: "Incidental take resulting from tree removal is prohibited if: 1) Occurs within 0.25 mile radius of known northern long-eared bat hibernacula or 2) cuts or destroys known occupied maternity roost trees, or any other trees within a 150-foot radius from the known maternity tree during the pup season (June 1 through July 31)."

The NHESP Northern Long-eared Bat Locations in Massachusetts map, last updated June 6, 2019 was reviewed. It was determined that the Project does not occur within 0.25 miles of a known winter hibernacula or within a 150-foot radius of a known maternity roost tree. Therefore, no further review of potential impacts to Northern Long-eared Bat is required pursuant to the MESA.

PLACEHOLDER LANGUAGE PENDING SITE SPECIFIC REVIEW: A project review package was submitted to USFWS on DATE, addressing Northern Long-Eared Bat (NLEB) and Plymouth Red-Belly Turtle. In summary:

A habitat assessment for Northern Long-Eared Bat was performed on DATE by GZA GeoEnvironmental, Inc. (GZA) and concluded that the project site does not provide important habitat for NLEB, and hibernacula or maternity roosting tree habitat are not known within ¼ mile of the site. The assessment also indicates that the closest location of documented overwintering for this species is located ># miles from the site, and further, that summer forage habitat is not present within the proposed work area.

GZA also performed a Plymouth Red-Belly Turtle assessment of the site, dated DATE. The assessment found that the project site does not occur within mapped Critical Habitat for the turtle, and a general habitat assessment and limited site survey found that the project site has low to moderate potential to support this species and no individual turtles were



found. Accordingly, a "may affect, but is unlikely to adversely affect" concurrence letter was issued by USFWS on April 10, 2019.

If yes, describe the species and/or critical habitat:

If yes, describe or refer to documentation that determines the likelihood of an impact on the identified species and/or habitat and the steps taken to address that impact.

3.2 HISTORIC PRESERVATION

Stan 1			
Step 1 Will stormwater controls that require subsur	face earth disturbance	be installed or	the site?
		X Yes	□No
Step 2 If you answered yes in Step 1, have prior already determined that historic properties dehave precluded the existence of historic properties.	not exist, or that price		
		Yes	No
Step 3 If you answered no in Step 2, has it been of	letermined that the in	nstallation of s	ubsurface
earth-disturbing stormwater controls will ha			
		Yes	No
PLACEHOLDER LANGUAGE PENDING	SITE SPECIFIC RE	VIEW: Historio	sites are
not present. See Appendix M: Historic Prese	rvation Documentation	<mark>on</mark> .	
Step 4			
If you answered no in Step 3, did the State			7.1
Historic Preservation Office (THPO), or of			11 /
respond within 15 calendar days to indicate caused by the installation of stormwater con			urbances
caused by the installation of stormwater con	nois affect historic pr	Yes	No
		1 CS	
If no, no further documentation is required. I	f yes, describe the nat	ure of their resp	onse and
include documentation in the Appendix:		•	
Written indication that adverse effects	to historic properties	from the insta	llation of
stormwater controls can be mitigated by agre	eed upon actions.		



	No agreement has been reached regarding measures to mitigate effects to historic properties from the installation of stormwater controls.
	Other:
3.3	SAFE DRINKING WATER ACT UNDERGROUND INJECTION CONTROL REQUIREMENTS
	Do you plan to install any of the following controls?
	Infiltration trenches (if stormwater is directed to any bored, drilled, driven shaft or dug hole that is deeper than its widest surface dimension, or has a subsurface fluid distribution system)
	Commercially manufactured pre-cast or pre-built proprietary subsurface detention vaults, chambers, or other devices designed to capture and infiltrate stormwater flow
	Drywells, seepage pits, or improved sinkholes (if stormwater is directed to any bored, drilled, driven shaft or dug hole that is deeper than its widest surface dimension, or has a subsurface fluid distribution system)
	If yes, attach documentation of contact between you and the applicable state agency or EPA Regional Office responsible for implementing the requirements for underground injection wells in the Safe Drinking Water Act and EPA's implementing regulations at 40 CFR Parts 144-147.
3.4	APPLICABLE STATE OR LOCAL PROGRAMS
	This SWPPP complies with the requirements of Standard 8 of the Massachusetts Department of Environmental Protection Stormwater Handbook, which states:
	A plan to control construction-related impacts, including erosion, sedimentation, and other

pollutant sources during construction and land disturbance activities (construction period erosion, sedimentation, and pollution prevention plans) shall be developed and



implemented.

4.0 EROSION AND SEDIMENT CONTROL BMPS

This SWPPP contains a listing of the erosion and sediment control best management practices (BMPs) that will be implemented to control pollutants in stormwater discharges. The BMPs are categorized under one of the areas of BMP activity as described below:

- Natural Buffers or Equivalent Sediment Controls
- Phased construction activity
- Control stormwater flowing onto and through the project
- Stabilize soils
- Protect slopes
- Protect storm drain inlets
- Establish perimeter controls and sediment barriers
- Retain sediment on-site and control dewatering practices
- Establish stabilized construction exits

4.1 NATURAL BUFFERS OR EQUIVALENT SEDIMENT CONTROLS

Are there any surface waters lo	ocated within 50	feet of your	construction of	disturbances that
receive stormwater discharges	from the site?		Yes	⊠ No

4.2 PHASED CONSTRUCTION ACTIVITY

Phased construction is not proposed. To minimize erosion during grading activities, grading and site work shall be conducted after snowmelt and during periods of predicted dry weather. The areas of the site that will remain vegetated after construction shall be stabilized with hydromulch or seeding immediately after grading activities are completed. All other areas of the construction site shall be stabilized if site work is not planned for more than 14 days. Disturbed areas shall be stabilized immediately after construction but no later than 14 days after construction ceases. Areas graded shall be stabilized with hydromulch immediately after construction but no later than 14 days after construction ceases.



4.3 STABILIZE SOIL

4.3.1 Temporary Stabilization

Description:	Initiation of temporary vegetative cover shall occur immediately where construction will cease for more than 7 days. Temporary vegetative cover shall be established using hydroseeding for areas of exposed soil (including stockpiles).
Installation Schedule:	Temporary stabilization measures shall be initiated immediately where construction activities will temporarily cease for more than 14 days. Stabilization will be completed as soon as practicable, but no later than 7 calendar days after stabilization has been initiated.
Maintenance and Inspection:	Stabilized areas shall be inspected weekly and after storm events until a dense cover of vegetation has become established. If failure is noticed at the seeded area, the area shall be reseeded, fertilized, and mulched immediately.

4.3.2 Hydromulching

Description:	Hydromulching shall provide immediate protection to
	exposed soils during short periods of disturbance.
	Hydromulch shall also be applied in areas that have been
	seeded for temporary or permanent stabilization.
Installation Schedule:	Hydromulch shall be applied to soil exposed temporarily
	for >14 days during construction.
Maintenance and	Hydromulched areas shall be inspected weekly and after
Inspection:	storm events to check for movement of mulch or erosion.
	If washout, breakage, or erosion occurs, the surface shall
	be repaired, and new hydromulch shall be applied to the
	damaged area.



4.3.3 Permanent Stabilization

Description:	Initiation of permanent stabilization measures shall occur
	immediately after the final design grades are achieved and
	earth moving activities cease. Vegetative cover shall be
	established on exposed soils. Permanent stabilization shall
	be completed in accordance with the procedures outlined
	in Section 6.0 Final Stabilization.
Installation Schedule:	Portions of the site where construction activities have
	permanently ceased shall be stabilized as soon as possible,
	but no later than 7 calendar days after stabilization has
	been initiated.
Maintenance and	All seeded areas shall be inspected weekly during
Inspection:	construction activities and after storm events until a dense
	cover of vegetation has been established. If failure is
	noticed at the seeded area, the area shall be reseeded in
	accordance with the plans. Care shall be taken to avoid
	compacting newly placed topsoil. After construction is
	completed at the site, permanently stabilized areas shall be
	monitored until final stabilization is reached.

4.3.4 Dust Control

Description:	Dust from the site shall be controlled by using a mobile pressure-type distributor truck to apply water to disturbed areas. The mobile unit shall apply water at a maximum rate of 300 gallons per acre and minimized as necessary to prevent runoff and ponding.
Installation Schedule:	Dust control shall be implemented as needed once site grading has been initiated and during windy conditions (forecasted or actual wind conditions of 20 mph or greater) while site grading is occurring. Spraying of water shall be performed no more than three times a day during the months of May–September and once per day during the months of October–April or whenever the dryness of the soil warrants it.
Maintenance and Inspection:	At least one mobile unit shall be available at all times to distribute water to control dust on the site. Each mobile unit shall be equipped with a positive shutoff valve to prevent over watering of the disturbed area.

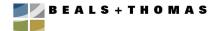


4.4 ESTABLISH PERIMETER CONTROLS AND SEDIMENT BARRIERS

4.4.1 Sediment Control Barrier

Permanent	▼ Temporary
Description:	A sediment control barrier, consisting of silt fence and
	compost mulch tube shall be installed along the down-
	gradient side of the proposed project to decrease the
	velocity of sheet flows and intercept and detain small
	amounts of sediment from disturbed areas.
Installation Schedule:	Sediment Control Barrier shall be installed prior to
	clearing and grubbing.
Maintenance and	Sediment Control Barrier shall be inspected weekly,
Inspection:	following storms, and daily during rainy periods.
	Damaged fencing or tubes shall be replaced. Concentrated
	flows shall be intercepted and rerouted. Sediment
	accumulations shall be removed when reaching a depth of
	6-inches, or one-half of the above ground height of the
	barrier, whichever is less. Deteriorated sediment control
	material shall be replaced. Used mulch tubes and fencing
	shall be properly disposed of.





4.5 ESTABLISH STABILIZED CONSTRUCTION ENTRANCE/EXIT

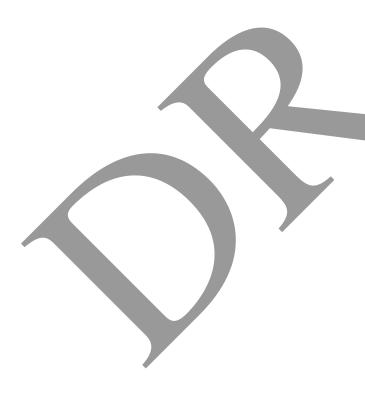
Permanent	Temporary
Description:	Temporary gravel or crushed stone construction
	entrance/exit or other means shall be used to minimize off-
	site movement of soil with vehicles. Construction access
	points shall be maintained to minimize tracking of soil onto
	public roads. If the rock entrance is not working to keep
	streets clean, then install wheel wash, sweep streets, or
	wash streets if wash water can be collected.
Installation Schedule:	Stabilized construction entrance shall be installed prior to
	earthmoving activities.
Maintenance and	Stabilized construction entrances shall be inspected daily.
Inspection:	Gravel or crushed stone shall be added if the pad is no
	longer in accordance with the specifications. If the rock
	entrance is not working to keep public streets clean, then
	install wheel wash, sweep streets, or wash streets if wash
	water can be collected. When sediment has been tracked
	off of the site onto public roads, it shall be removed by the
	end of the same working day, or by the end of the next
	working day if track-out occurs on a non-work day.
	Remove sediment by sweeping, shoveling or vacuuming
	public roadways were sediment has been tracked-out.

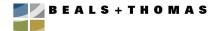
4.6 DEWATERING PRACTICES

Description:	All groundwater or stormwater discharged from	
	excavations, trenches, or other similar points shall be	
	treated by sediment basins, sediment traps, sediment socks,	
	dewatering tanks, tube settlers or filtration systems	
	specifically designed to remove sediment from the	
	excavations. All dewatering practices shall conform to the	
	following:	
	ionowing.	
	. V:-1.1. G4: C1. 11 41. 41. 1 1	
	 Visible floating solids or foam shall not be discharged; 	
	• An oil-water separator or suitable filtration device	
	(such as a cartridge filter) that is designed to remove	
	oil, grease, or other products if dewatering water is	
	found to contain these materials shall be used;	
	• To the extent feasible, utilize vegetated, upland areas	
	of the site to infiltrate dewatering water before	
	discharge. In no case will surface waters be considered	
	part of the treatment area;	
	part of the treatment area,	



	• Velocity dissipaters shall be installed at all points where dewatering activities are discharged to the surface.		
	 With backwash water, either haul it away for disposal or return it to the beginning of the treatment process; and 		
	• Replace and clean the filter media used in dewatering devices when the pressure differential equals or exceeds the manufacturer's specifications.		
Installation Schedule:	Install settling or filtration methods prior to commencing dewatering. Engineer is required to approve settling of filtration method design prior to installation.		
Maintenance and Inspection:	Settling of filtration controls shall be inspected weekly and following storms. Sediment shall be removed when it reaches a depth of one foot, or half the design capacity whichever is less.		





5.0 GOOD HOUSEKEEPING BMPS

This SWPPP contains a listing of the good housekeeping best management practices (BMPs) that shall be implemented to control pollutants in stormwater discharges during construction-related work. The BMPs are categorized below:

- Material Handling and Waste Management
- Establish Proper Building Material Staging Areas
- Designate Washout Areas
- Establish Proper Equipment/Vehicle Fueling and Maintenance Practices
- Allowable Non-Stormwater Discharges and Control Equipment/Vehicle Washing
- Spill Prevention and Control Plan

5.1 MATERIAL HANDLING AND WASTE MANAGEMENT

Several management procedures and practices are proposed to prevent and/or reduce the discharge of pollutants to stormwater from solid or liquid wastes that will be generated at the site. These measures are grouped into the following categories: (1) solid or construction waste disposal, (2) recycling, (3) sanitary and septic waste, and (4) hazardous materials.

5.1.1 Solid or Construction Waste Disposal

Description:	All waste materials shall be collected and disposed of into metal
	trash dumpsters or enclosed trash containers in the materials storage
	area. Dumpsters shall have a secure watertight lid, be placed away
	from stormwater conveyances and drains, and meet all federal, state,
	and municipal regulations. Only trash and construction debris from
	the site shall be deposited in the dumpster. No construction materials
	shall be buried on-site unless authorized by a program for
	recycling/beneficial use. All personnel shall be instructed regarding
	the correct disposal of trash and construction debris. Notices that
	state these practices shall be posted in the office trailer and the
	individual who manages day-to-day site operations shall be
	responsible for seeing that these practices are followed.
Installation	Trash dumpsters shall be installed once the materials storage area
Schedule:	has been established.
Maintenance	The dumpsters shall be inspected weekly and immediately after
and	storm events. The dumpsters shall be emptied weekly and taken to
Inspection:	an approved landfill or recycling facility. If trash and construction
	debris are exceeding the dumpsters' capacity, the dumpsters shall be
	emptied more frequently. Waste container lids shall be closed when
	not in use and at the end of the business day. For waste containers
	that do not have lids, provide cover or a similarly effective means to
	minimize the discharge of pollutants.



5.1.2 Recycling

Description:	Wood pallets, cardboard boxes, and other recyclable construction scraps shall be disposed of in a designated dumpster for recycling. The dumpster shall have a secure watertight lid, be placed away from stormwater conveyances and drains and meet all local and state solid-waste management regulations. Only solid recyclable construction scraps from the site shall be deposited in the dumpster. All personnel shall be instructed regarding the correct procedure for disposal of recyclable construction scraps. Notices that state these procedures shall be posted in the office trailer, and the individual who manages day-to-day site operations shall be responsible for seeing that these procedures are followed.			
Installation	Designated recycling dumpsters shall be installed when building			
Schedule:	materials arrive on-site.			
Maintenance	The recycling dumpster shall be inspected weekly and immediately			
and	after storm events. The recycling dumpster shall be emptied weekly			
Inspection:	and taken to an approved recycling center. If recyclable			
	construction wastes are exceeding the dumpsters' capacity, the			
	dumpsters shall be emptied more frequently.			

5.1.3 Sanitary and Septic Waste

Description:	Temporary sanitary facilities (portable toilets) shall be provided at			
	the site throughout the construction phase. The portable toilets shall			
	be located in the staging area, away from concentrated flow paths			
	and traffic flow.			
Installation	The portable toilets shall be brought to the site once the staging area			
Schedule:	has been established.			
Maintenance	All sanitary waste shall be collected from the portable facilities on			
and	a regular basis. The portable toilets shall be inspected weekly for			
Inspection:	evidence of leaking holding tanks. Toilets with leaking holding			
	tanks shall be removed from the site and replaced with new portable			
	toilets.			



5.1.4 Hazardous Materials and Waste

Description:	All hazardous waste materials such as oil filters, petroleum
Description.	products, paint, and equipment maintenance fluids shall be stored
	in structurally sound and sealed shipping containers, within the
	hazardous materials storage area. Hazardous waste materials shall
	be stored in appropriate and clearly marked containers and
	segregated from other non-waste materials. Secondary containment
	shall be provided for all waste materials in the hazardous materials
	storage area and shall consist of commercially available spill
	pallets. Additionally, all hazardous waste materials shall be
	disposed of in accordance with federal, state, and municipal
	regulations. Hazardous waste materials shall not be disposed of into
	the on-site dumpsters. All personnel shall be instructed regarding
	proper procedures for hazardous waste disposal. Notices that state
	these procedures shall be posted in the office trailer and the
	individual who manages day-to-day site operations shall be
	responsible for seeing that these procedures are followed.
Installation	Shipping containers used to store hazardous waste materials shall
Schedule:	be installed once such materials arrive on-site.
Maintenance	The hazardous waste material storage areas shall be inspected
and	weekly and after storm events. The storage areas shall be kept
Inspection:	clean, well-organized, and equipped with ample cleanup supplies
mspection.	
	as appropriate for the materials being stored. Material safety data
	sheets, material inventory, and emergency contact numbers shall be
	maintained in the office trailer.

5.2 ESTABLISH PROPER BUILDING MATERIAL STAGING AREAS

Description: Construction equipment and maintenance materials shall be stored at the combined staging area and materials storage areas. A watertight shipping container shall be used to store hand tools, small parts, and other construction materials. Nonhazardous building materials such as packaging material (wood, plastic, and glass), and construction scrap material (steel, metal scraps, and pipe cuttings) shall be stored in a separate covered storage facility adjacent to the shipping container.

All hazardous-waste materials such as oil filters, petroleum products, paint, and equipment maintenance fluids shall be stored in structurally sound and sealed containers under cover within the storage area.

Very large items, shall be stored in the open in the materials storage area. Such materials shall be elevated on blocks to minimize contact with runoff.



Installation	The materials storage area shall be installed after grading and before any			
Schedule:	infrastructure is constructed at the site.			
Maintenance	The storage area shall be inspected weekly and after storm events. The			
and	storage area shall be kept clean, well-organized, and equipped with ample			
Inspection:	cleanup supplies as appropriate for the materials being stored. Perimeter			
	controls, containment structures, covers, and liners shall be repaired or			
	replaced as needed to maintain proper function.			

5.3 DESIGNATE WASHOUT AREAS

5.3.1 Concrete Washout

Description:	A designated temporary, above-grade concrete washout area shall be constructed. The temporary concrete washout area shall be					
	constructed with a recommended minimum length and minimum					
	width of 10 feet, but with sufficient quantity and volume to contain					
	all liquid and concrete waste generated by washout operations. The					
	washout area shall be lined with plastic sheeting at least 10 mils					
	thick and free of any holes or tears. Signs shall be posted marking					
	the location of the washout area to ensure that concrete equipment					
	operators use the proper facility.					
	Concrete pours shall not be conducted during or before an anticipated storm event. Concrete mixer trucks and chutes shall be					
	washed in the designated area or concrete wastes shall be properly					
	disposed of off-site. When the temporary washout area is no longer					
	needed for the construction project, the hardened concrete and					
	materials used to construct the area shall be removed and disposed					
	of according to the maintenance section below, and the area shall					
	be stabilized.					
Installation	The washout area shall be constructed before concrete pours occur					
Schedule:	at the site.					
Maintenance and	The washout areas shall be inspected daily to ensure that all concrete washing is being discharged into the washout area, no					
Inspection:	leaks or tears are present, and to identify when concrete wastes need					
	to be removed. The washout areas shall be cleaned out once the area					
	is filled to 75 percent of the holding capacity. Once 75% of the					
	area's holding capacity has been reached, the concrete wastes shall					
	be allowed to harden; the concrete shall be broken up, removed,					
	and taken to an approved landfill for disposal or recycled on-site or					
	off-site in accordance with applicable laws. The plastic sheeting					
	shall be replaced if tears occur during removal of concrete wastes					
	from the washout area.					



Design Specifications:

- 1. Temporary concrete washout type Above Grade shall be constructed as detailed above.
- 2. The washout shall be a minimum of 50 feet from storm drain inlets.
- 3. Plastic lining shall be free of holes, tears, or other defects that compromise the impermeability of the material.

5.4 ESTABLISH PROPER EQUIPMENT/VEHICLE FUELING AND MAINTENANCE PRACTICES

Description:	Several types of vehicles and equipment will likely be used on-site
	throughout the project, including graders, scrapers, excavators, loaders,
	rollers, trucks and trailers, backhoes, and forklifts. All major
	equipment/vehicle fueling and maintenance shall be performed outside of
	wetland resource areas and associated buffer zones. When vehicle fueling
	must occur on-site, the fueling activity shall occur in the staging area. Only
	minor equipment maintenance shall occur on-site. All equipment fluids
	generated from maintenance activities shall be disposed of into designated
	drums stored on spill pallets in accordance with the Material Handling and
	Waste Management Section 5.1. Absorbent, spill-cleanup materials and
	spill kits shall be available at the combined staging and materials storage
	area. Drip pans shall be placed under all equipment receiving maintenance
	and vehicles and equipment parked overnight.
Installation	BMPs implemented for equipment and vehicle maintenance and fueling
Schedule:	activities shall begin at the start of the project.
Maintenance	Inspect equipment/vehicle storage areas weekly and after storm events.
and	Vehicles and equipment shall be inspected on each day of use. Leaks shall
Inspection:	be repaired immediately, using dry cleanup measures where possible and
	eliminating the source of the discharge. Problem vehicle(s) or equipment
	shall be removed from the project site. Keep ample supply of spill-cleanup
	materials on-site and immediately clean up spills and dispose of materials
	properly. Do not clean surfaces by hosing-down the area.



5.5 ALLOWABLE NON-STORMWATER DISCHARGES AND CONTROL EQUIPMENT / VEHICLE WASHING

Description:	All equipment and vehicle washing shall be performed off-site, except as				
	required for wheel washes and concrete washout areas.				
Installation	N/A				
Schedule:					
Maintenance	N/A				
and					
Inspection:					

5.6 SPILL PREVENTION AND CONTROL PLAN

Description:	i.	Employee Training: All employees shall be trained as detailed in			
Description.	1.				
		the Inspection and Maintenance Section 8.0 of this report.			
	ii.	Vehicle Maintenance: Vehicles and equipment shall be maintained			
	off-site, except for minor maintenance as needed. All vehicles and				
		equipment including subcontractor vehicles shall be checked for			
		leaking oil and fluids. Vehicles leaking fluids shall not be allowed on-site.			
	iii.	Hazardous Material Storage: Hazardous materials shall be stored in			
	111.	accordance with this report and applicable regulations.			
	iv.	Spill Kits: Spill kits shall be kept within the materials storage area.			
	Spills: All spills shall be cleaned up immediately upon discovery.				
	Spent absorbent materials and rags shall be hauled off-site				
immediately after the spill is cleaned up for disposal at an approved					
landfill. Spills shall be reported to the National Response Center at					
		1-800-424-8802 and MassDEP at 888-304-1133 as applicable in			
		accordance with state and federal requirements.			
		<u> </u>			
	V.	Material safety data sheets: A material inventory and emergency			
		contact information shall be maintained at the on-site project trailer.			
Installation	The s	pill prevention and control procedures shall be implemented once			
Schedule:	constr	uction begins on-site.			
Maintenance	All p	ersonnel shall be instructed on the correct procedures for spill			
and	prever	ntion and control. Notices that state these practices shall be posted in			
Inspection:	the of	fice trailer, and the individual who manages day-to-day site operations			
_	shall b	be responsible for seeing that these procedures are followed.			



5.7 FERTILIZER DISCHARGE RESTRICTIONS

Description:	Discharges from fertilizers containing nitrogen and phosphorus sha minimized. Fertilizers shall be applied at rates and amounts consistent the manufacture's specification, and shall at no time exceed local, sta					
	federal specifications. See project landscape specifications for acceptable fertilizers that can be used for the project.					
Installation	Fertilizers shall be applied at an appropriate time of year, timed to					
Schedule:	coincide as closely as possible to the period of maximum vegetation					
	uptake and growth. Avoid applying fertilizers before heavy rains. Do not					
	apply fertilizers to frozen ground or stormwater conveyance channels					
	flowing with water.					
Maintenance	N/A					
and						
Inspection:						

5.8 ALLOWABLE NON-STORMWATER DISCHARGE MANAGEMENT

Any changes in construction activities that produce other allowable non-stormwater discharges shall be identified, and the SWPPP shall be amended and the appropriate erosion and sediment control shall be implemented.

The following is a list of allowable non-stormwater discharges:

- Water Used to Control Dust
- Uncontaminated Excavation Dewatering
- Firefighting
- Non-Detergent Laden Vehicle Wash Water

Except for water used to control dust and irrigation water, the above discharges shall not be routed to areas of exposed soil.



6.0 FINAL STABILIZATION

In compliance with the Construction General Permit, soil stabilization measures must be implemented immediately whenever earth-disturbing activities are temporarily or permanently ceased on any portion of the site. Earth-disturbing activities are temporarily ceased when clearing, grading, and excavation within any area of a site that will not include a permanent structure will not resume for a period of 7 or more calendar days, but such activities will resume in the future.

In the context of this provision, "immediately" means as soon as practicable, but no later than the end of the next work day, following the day when the earth-disturbing activities have temporarily or permanently ceased. The following activities constitute the initiation of stabilization:

- Preparing the soil for vegetative or non-vegetative stabilization;
- Applying mulch or other non-vegetative product to the exposed area;
- Seeding or planting the exposed area;
- Starting any of the activities in listed above on a portion of the area to be stabilized, but not on the entire area; and
- Finalizing arrangements to have stabilization product fully installed in compliance with the applicable deadline for completing stabilization.

As soon as practicable, but no later than 7 calendar days after the initiation of soil stabilization measures the following activities are required to be completed:

- For vegetative stabilization, all activities necessary to initially seed or plant the area to be stabilized; and/or
- For non-vegetative stabilization, the installation or application of all such non-vegetative measures.

The following sections detail the management practices proposed to achieve final stabilization of the site.



6.1 PERMANENT SEEDING

Description:	Permanent seeding shall be applied immediately after the final design					
	grades are achieved on portions of the site but no later than 7 days after					
	construction activities have permanently ceased. After the entire site is					
	stabilized, any sediment that has accumulated shall be removed and					
	hauled off-site for disposal at an approved landfill. Construction debris,					
	trash and temporary BMPs (including sedimentation controls, material					
	storage areas, sanitary toilets, and inlet protection) shall also be removed					
	and any areas disturbed during removal shall be seeded immediately.					
	Seeding shall be performed in accordance to the Site Plans and Landscape					
	Specifications for the project.					
Installation	Seeding shall occur at portions of the site where construction activities					
Schedule:	have permanently ceased shall be stabilized, as soon as possible but no					
	later than 7 days after construction ceases.					
Maintenance	All seeded areas shall be inspected weekly during construction activities					
and	for failure and after storm events until a dense cover of vegetation has					
Inspection:	been established. If failure is noticed at the seeded area, the area shall be					
	reseeded in accordance with the plans. After construction is completed at					
	the site, permanently stabilized areas shall be monitored until final					
	stabilization is achieved.					





7.0 INSPECTIONS AND MAINTENANCE

7.1 INSPECTIONS

7.1.1 Inspection Schedule and Procedures

Inspections of the site will be performed once every 7 days and within 24 hours of the end of a storm event of 0.25-inch) or greater unless otherwise specified. The inspections will verify that all BMPs required are implemented, maintained, and effectively minimizing erosion and preventing stormwater contamination from construction materials.

To determine if a storm event of 0.25 inches or greater has occurred on the site, either a properly maintained rain gauge will be kept on the site or the storm event information will be obtained from a weather station that is representative of the location. If an inspection is conducted because of rainfall measuring 0.25 inches or greater, the applicable rain gauge or weather station readings that triggered the inspection will be noted in the inspection report.

Inspections shall include all areas of the site disturbed by construction activity and areas used for storage of materials that are exposed to precipitation. Inspectors shall look for evidence of, or the potential for, pollutants entering the storm water conveyance system. Sedimentation and erosion control measures identified in the SWPPP shall be observed to ensure proper operation. Discharge locations shall be inspected to ascertain whether sediment and erosion control measures are effective in preventing significant impacts to waters of the United States, where accessible. Where discharge locations are inaccessible, nearby downstream location shall be inspected to the extent that such inspections are practicable. Locations where vehicles enter or exit the site shall be inspected for evidence of off-site sediment tracking.

For detailed inspection procedures, see Sections 4 and 5.

All inspections shall be coordinated with a representative from Owner Company. An Owner Company representative shall accompany the Inspector, when possible, during inspections.

Inspection reports are required to be completed within 24-hours of an inspection. If corrective actions are identified by the Inspector during the inspection, he/she shall notify and submit a copy of the inspection report to the Operator(s). For corrective actions identified, the Site Owner/Site Operator shall be responsible for initiating the corrective action within 24 hours of the report and completing maintenance as soon as possible or before the next storm event. For any corrective actions requiring



a SWPPP amendment or change to a stormwater conveyance or control design, the Site Owner/Site Operator shall notify Owner, as soon as possible, before initiating the corrective action.

The business days for the project construction are 7:00 am to 5:00 pm, Monday through Friday.

For a copy of the inspection report template, see Appendix E.

7.2 REDUCTIONS IN INSPECTION FREQUENCY

Once an area is stabilized, inspections may be reduced to twice per month for the first month, no more than 14 calendar days apart, then once per month. If construction resumes at the stabilized area the inspection frequency shall increase as outlined in Section 7.1.

If earth-disturbing activities are suspended due to frozen conditions inspections can be temporarily suspended until a thaw occurs.

7.3 CORRECTIVE ACTION LOG

The corrective action log describes repairs, replacements, and maintenance of BMPs undertaken as a result of the inspections and maintenance procedures. Additionally, remedies of permit violations and clean and proper disposal of spills, releases other deposits should be recorded.

If it is determined the stormwater controls have not been installed as required, or that they are not functioning adequately corrective action is required within 7 calendar days.

The operator will document the completion of the corrective action within 24 hours.

See Appendix F – Corrective Action Log.



8.0 RECORDKEEPING AND TRAINING

8.1 RECORDKEEPING

A copy of the SWPPP, along with all inspection reports and corrective action logs are required to be stored at an accessible location at the site or other location easily accessible during normal business hours, and shall be made available upon request of the EPA, or state or local agency approving stormwater management plans.

The following records shall be kept at the project site and shall be available for inspectors to review. These records shall be retained for a minimum period of at least 3 years after the permit is terminated.

Date(s) when major grading activities occur:

See Appendix I – Grading and Stabilization Activities Log

Date(s) when construction activities temporarily or permanently cease on a portion of the site:

See Appendix I – Grading and Stabilization Activities Log

Date(s) when an area is either temporarily or permanently stabilized:

See Appendix I – Grading and Stabilization Activities Log

8.2 LOG OF CHANGES TO THE SWPPP

The log of changes to the SWPPP is maintained in Appendix G and includes additions of new BMPs, replacement of failed BMPs, significant changes in the activities or their timing on the project, changes in personnel, changes in inspection and maintenance procedures and updates to site plans.

8.3 TRAINING

Prior to the commencement of earth-disturbing activities or pollutant-generating activities, whichever occurs first, training on the pollution prevention measures outlined in this SWPPP shall be provided to staff and subcontractors.

8.3.1 Individual(s) Responsible for Training

Company/Organization: TBD

Name: TBD



8.3.2 Description of Training Conducted

Informal training shall be conducted for all staff, including subcontractors, on the site. The training shall be conducted primarily via tailgate sessions and shall focus on avoiding damage to stormwater BMPs and preventing illicit discharges. The tailgate sessions shall be conducted biweekly and shall address the following topics: Erosion Control BMPs, Sediment Control BMPs, Non-Stormwater BMPs, Waste Management and Materials Storage BMPs, and Emergency Procedures specific to the construction site. (See Appendix J – Training Log)

Formal training shall be provided to all staff and subcontractors with specific stormwater responsibilities, such as installing and maintaining BMPs. The formal training shall cover all design and construction specifications for installing the BMPs and proper procedures for maintaining each BMP. Training shall also cover inspection schedules and procedures for personnel whose job duties are related to inspections. Formal training shall occur before any BMPs are installed on the site. (See Appendix J – Training Log)



9.0 CERTIFICATION AND NOTIFICATION

9.1 SIGNATURE, PLAN REVIEW, AND MAKING PLANS AVAILABLE

A copy of the SWPPP (including a copy of the Construction General Permit, NOI, and acknowledgement letter from EPA) shall be retained at the construction site (or other location easily accessible during normal business hours to EPA, a state, tribal or local agency approving sediment and erosion plans, grading plans, or storm water management plans; local government officials; the operator of a municipal separate storm sewer receiving discharges from the site; and representatives of the U.S. Fish and Wildlife Service or the National Marine Fisheries Service) from the date of commencement of construction activities to the date of final stabilization. A copy of the SWPPP shall be available at a central location on-site for the use of all those identified as having responsibilities under the SWPPP. If an on-site location is unavailable to store the SWPPP when no personnel are present, notice of the plan's location shall be posted near the main entrance at the construction site.

9.2 NOTICE OF PERMIT COVERAGE

A sign must be posted at a safe, publicly accessible location in close proximity to the construction site detailing the permit coverage. The notice must be located so that it is visible from the public road that is nearest to the active part of the construction site, and it must use a font large enough to be readily viewed from a public right-of-way. At a minimum, the notice must include:

- The NPDES Permit Tracking Number,
- A contact name and phone number for obtaining additional construction site information,
- The Uniform Resource Locator (URL) for the SWPPP (if available), or the following statement: "If you would like to obtain a copy of the Stormwater Pollution Prevention Plan (SWPPP) for this site, contact the EPA Regional 1 Office at (617) 918-1038,
- The following statement "If you observe indicators of stormwater pollutants in the discharge or in the receiving waterbody, contact the EPA through the following website; https://www.epa.gov/enforcement/report-environmental-violations."



9.3 OWNER CERTIFICATION

I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Name:	Title:	
Signature:	Date:	

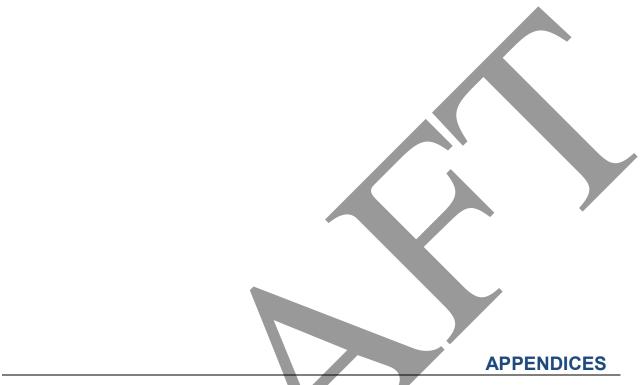


9.4 OPERATOR CERTIFICATION

I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Name:	Title:	
Signature:	Date:	

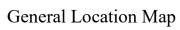










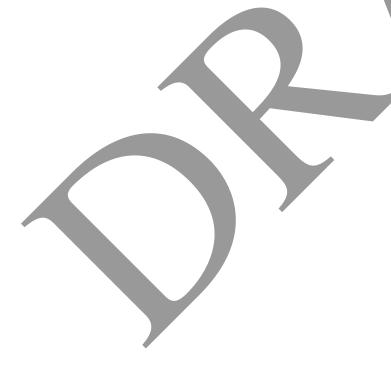
















Construction General Permit

https://www.epa.gov/sites/production/files/2017-02/documents/2017 cgp final permit 508.pdf





Appendix D NOI and Acknowledgement Letter from EPA





Appendix E

Inspection Reports

Inspections under this SWPPP shall be conducted in accordance with each installed BMPs recommended maintenance requirements. This inspection frequency may be reduced to at least once every month if: a) the entire site is temporarily stabilized, b) runoff is unlikely due to winter conditions (e.g. site is covered with snow, ice, or the ground is frozen), or c) construction is occurring during seasonal arid periods in arid areas and semi-arid areas. If an inspection report is filed according to this modified schedule it shall be noted at the end of the report under the "NOTES" section.

The following pages should be copied and completed for each inspection. All inspection forms should be compiled in a binder to prove compliance with this SWPPP.



Stormwater Pollution Prevention Plan: Inspection Checklist

	General Inf	ormation		
Project Name				
NPDES Tracking No.		Location		
Date of Inspection		Start/End Time		
Inspector's Name(s)				
Inspector's Title(s)				
Inspector's Contact Information				
Inspector's Qualifications				
Describe present phase of construction				
Type of Inspection: ☐ Regular ☐ Pre-store	rm event	storm event	Post-storm event	
	Weather In	formation		
Has there been a storm ev If yes, provide: Storm Start Date & Time: Approx. Amount of Precipi	tation (in):	ion? □Yes □N Storm Duration (hrs	-	
☐ Other:	Rain Sleet Fo	perature:	☐ High Winds	
Have any discharges occu If yes, describe:	rred since the last inspec	etion? □Yes □1	No	
Are there any discharges If yes, describe:	at the time of inspection	? □Yes □No		



E-1

Site-specific BMPs

- Number the structural and non-structural BMPs identified in your SWPPP on your site map and list them below (add as many BMPs as necessary). Carry a copy of the numbered site map with you during your inspections. This list will ensure that you are inspecting all required BMPs at your site.
- Describe corrective actions initiated, date completed, and note the person that completed the work in the Corrective Action Log.

ВМР	BMP Installed?	BMP Maintenance Required?	Corrective Action Needed and Notes
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	
	□Yes □No	□Yes □No	



Overall Site Issues

Below are some general site issues that should be assessed during inspections. Customize this list as needed for conditions at your site.

BMP/activity	Implemented?	Maintenance Required?	Corrective Action Needed and Notes
Are all slopes and disturbed areas not actively being worked properly stabilized?	□Yes □No	□Yes □No	
Are natural resource areas (e.g., streams, wetlands, mature trees, etc.) protected with barriers or similar BMPs?	□Yes □No	□Yes □No	
Are perimeter controls and sediment barriers adequately installed (keyed into substrate) and maintained?	□Yes □No	□Yes □No	
Are discharge points and receiving waters free of any sediment deposits?	□Yes □No	□Yes □No	
Are storm drain inlets properly protected?	□Yes □No	□Yes □No	
Is the construction exit preventing sediment from being tracked into the street?	□Yes □No	□Yes □No	
Is trash/litter from work areas collected and placed in covered dumpsters?	□Yes □No	□Yes □No	
Are washout facilities (e.g., paint, stucco, concrete) available, clearly marked, and maintained?	□Yes □No	□Yes □No	
Are vehicle and equipment fueling, cleaning, and maintenance areas free of spills, leaks, or any other deleterious material?	□Yes □No	□Yes □No	



BMP/activity	Implemented?	Maintenance Required?	Corrective Action Needed and Notes
Are materials that are potential stormwater contaminants stored inside or under cover?	□Yes □No	□Yes □No	
Are non-stormwater discharges (e.g., wash water, dewatering) properly controlled?	□Yes □No	□Yes □No	
(Other)	□Yes □No	□Yes □No	
	Non-C	ompliance	
supervision in accordance with a evaluated the information submit or those persons directly respons of my knowledge and belief, tru	system designed to ted. Based on my in tible for gathering to e, accurate, and co	nd all attachmen to assure that quanquiry of the per the information, to complete. I am aw	ts were prepared under my direction or alified personnel properly gathered and son or persons who manage the system, he information submitted is, to the best ware that there are significant penalties fine and imprisonment for knowing
Signature:			
Data			











Corrective Action Log

Use this form to note the date and activity for accurate record keeping (make additional copies as necessary). Examples include the restaking or reinforcement of the erosion control barrier, site watering to prevent dust erosion, street sweeping, equipment and machinery repair, etc.

Date	Activity Description	Additional Action Items
		4

Appendix G

SWPPP Amendment Log

The SWPPP, including the site plans, shall be amended whenever there is a change in design, construction, operation, or maintenance at the construction site that has or could have a significant effect on the discharge of pollutants to the waters of the United States that has not been previously addressed in the SWPPP.

The SWPPP shall be amended if during inspections or investigations by site staff, or by local, state, tribal or federal officials, it is determined that the SWPPP is ineffective in eliminating or significantly minimizing pollutants in storm water discharges from the construction site.

Based on the results of an inspection, the SWPPP shall be modified as necessary to include additional or modified BMPs designed to correct problems identified. Revisions to the SWPPP shall be completed within seven (7) calendar days following the inspection. Implementation of these additional or modified BMPs shall be accomplished as described in Subpart 3.6B of the Construction General Permit (located in Appendix C).



SWPPP Amendment Log

Amendment	Description of the Amendment	Date of	Amendment
No.		Amendment	Prepared by
			(Name(s) and Title)
			Z
		V	
	<u> </u>		

Appendix H Subcontractor Certifications/Agreements





Sample Subcontractor Certifications/Agreements

SUBCONTRACTOR CERTIFICATION STORMWATER POLLUTION PREVENTION PLAN

Project Number:		
Project Title:		
Operator(s):		
As a subcontractor, you are required to comply with for any work that you perform on-site. Any person or be subject to substantial penalties or loss of contract. working on this project of the requirements of the S review at the office trailer.	group who violates any condit You are encouraged to advise	tion of the SWPPP may each of your employees
Each subcontractor engaged in activities at the conidentified and sign the following certification statements		ct stormwater must be
I certify under the penalty of law that I have rea SWPPP for the above designated project and agree		
This certification is hereby signed in reference to the	above named project:	
Company: Address:		
Telephone Number:	<u></u>	
Type of construction service to be provided:		
Signature:		
Title: Date:		
Date.		





Grading and Stabilization Activities Log

Site Plans in Appendix B should be annotated to indicate areas where final stabilization has been accomplished and no further construction-phase permit requirements apply.



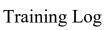
The following records are to be kept by each Site Operator throughout the construction period and maintained in the SWPPP. Insert additional documentation for record keeping as necessary.

Grading and Stabilization Activities Log

Date Location on Property Description	









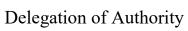


Training Log

Date	Training Topic	Attendee	Signature of Training Coordinator
		<u> </u>	







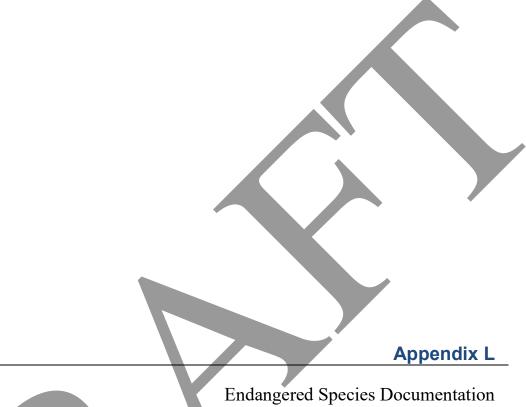


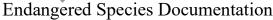
Sample Delegation of Authority Form

Delegation of Authority

I, (name), hereby designate the person or specifically described position
below to be a duly authorized representative for the purpose of overseeing compliance with environmenta
requirements, including the Construction General Permit, at the
construction site. The designee is authorized to sign any
reports, stormwater pollution prevention plans and all other documents required by the permit.
(name of person or position)
(company) (address)
(city, state, zip)
(phone)
(phone)
By signing this authorization, I confirm that I meet the requirements to make such a designation as set forth in Appendix I of EPA's Construction General Permit (CGP), and that the designee above meets the definition of a "duly authorized representative" as set forth in Appendix I. I certify under penalty of law that this document and all attachments were prepared under my direction of supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.
Name:
Company:
Title:
Signature:
Date:









Appendix M Historic Preservation Documentation



